# **APPENDIX M: ENGINEERING REPORTS**

## Volume 2 of 4

#### Contents:

Basis of Design Report -15% Design Appendices as follows:

- 1. A.1 Survey Data Collection Plan
- 2. A.2 Additional Survey Cross-Sections & Culvert Crossings
- 3. A.3 Survey Monument Sheets
- 4. A.4 Culvert Reports & Photos
- 5. A.5 Survey Data for Camp on Hope Canal S. of I-10
- 6. B.1 Geotechnical Data Collection Plan
- 7. B.2 Geotechnical Data Report

It should be noted that the Engineering Reports were provided by CPRA as standalone documents and in some cases the terminology within may not match the terminology used in the SEIS (e.g. MSP vs. MSA-2 for the selected alternative).





# SURVEY DATA COLLECTION PLAN

Topographic, Bathymetric,
Magnetometer and other
Professional Land Survey Services

# **Project:**

Maurepas Diversion and West Shore Lake Pontchartrain Reaches WSLP-111, -112, & -113

St. John the Baptist Parish, Louisiana

**Prepared for:** 



Mr. Clay Loyless, PE

Prepared by:
Ricardo Johnson, PLS
Fenstermaker

December 20, 2019 (Revised May 22, 2020, August 07, 2020)

December 20, 2019 (Rev. May 22, 2020, August 07,2020)

Clay Loyless, PE AECOM 1515 Poydras St. Suite 2700 New Orleans, LA 70112

RE: Maurepas Diversion and Reaches WSLP-111, WSLP-112, WSLP-113

Dear Mr, Loyless,

C.H. Fenstermaker & Associates, L.L.C. (Fenstermaker) appreciates the opportunity to submit this survey cost estimate to AECOM in support of the above referenced project. Fenstermaker proposes to provide the required survey services as specified in the Scope of Work dated December 12, 2019 (see Attachment A). Cost Breakdowns for each task of work are located on Attachment B and will be billed as Time and Materials, not to exceed, as per Fenstermaker's CPRA Contract Fee Schedule.

As specified in the Scope of Work, Fenstermaker will provide the required labor, equipment, and materials necessary to assure compliance with CPRA's and USACE New Orleans District requirements for each task performed. The Proposed Scope of Work and Survey Tasks (Attachment A) details Fenstermaker's approach and estimated time to perform each survey task once the Notice to Proceed is issued. A work schedule will be provided upon receipt of the Notice to Proceed.

The horizontal datum to be used for positioning will be NAD83 (2011), Louisiana State Plane (1702) South Zone. Fenstermaker will report water bottom elevations in NAVD88 using Geoid12B vertical datum and Local Mean Sea Level (LMSL).

Fenstermaker would like to thank you in advance for giving our staff the opportunity to meet your survey requirements. If you have any questions, please call my direct line at (337) 443-6475.

Sincerely,

C.H. FENSTERMAKER & ASSOCIATES, L.L.C.

Ricardo M. Johnson, PLS

**Director of Coastal Survey Services** 

# Attachment A Proposed Scope of Work and Survey Work Plan

Maurepas Freshwater Diversion and Reaches WSLP-111, WSLP-112, WSLP-113 Topographic, Bathymetric, Magnetometer and Other Surveying Services December 12, 2019

#### 1.0 Introduction

AECOM is currently designing the CPRA project "River Reintroduction into Maurepas Swamp" (PO-0029) (Maurepas Diversion) as well as Reaches WSLP-111, WSLP-112, and WSLP-113 of the USACE West Shore Lake Pontchartrain flood protection project (WSLP). Each project is briefly described below. Due to the co-location of the Maurepas Diversion and the WSLP project features in the same alignment corridor, the projects are being designed together.

#### 2.0 Maurepas Diversion Project

The Maurepas Diversion is a proposed 2,000 cubic foot per second (cfs) freshwater diversion from the Mississippi River into the Maurepas Swamp. The intake to the diversion is located at approximately River Mile 144 and the inland features will be located in St. John the Baptist Parish, Louisiana. The basic components are an intake channel in the batture; a gated structure in the Mississippi River levee; a sedimentation basin; a 5½ mile long open conveyance channel; crossings at the Canadian National (CN) and Kansas City Southern (KCS) railroads; crossings at River Road, Airline Highway, and Interstate 10; submerged weirs in Bayou Secret and Bourgeois Canal; embankment cuts in the existing ridge of an old cypress logging trail; and check valves on the northern side of culverts underneath I-10.

AECOM submitted the latest plans and specifications for the Maurepas Diversion to CPRA in September of 2013. Since that time, major changes to both the existing conditions as well as the overall Scope of Work have been made. The primary changes in conditions have been: 1) the development of the Marathon Petroleum Mt. Airy Terminal facilities on land adjacent to the proposed Maurepas Diversion alignment, and 2) the construction of a Marathon Petroleum marine docking facility upstream of the diversion intake. The permitting of a second marine dock facility which will include a pipe-bridge across the intake is currently in process. The primary change to the Scope of Work is the addition of the design of those components of the USACE WSLP flood protection project that parallel the Maurepas Diversion project from the south side of River Road to the north side of Airline Highway, which includes levees, floodwalls, and gates.

#### 3.0 West Shore Lake Pontchartrain Project

The West Shore Lake Pontchartrain (WSLP) project will provide hurricane and storm-damage risk reduction in St. Charles and St. John the Baptist Parishes. The recommended plan includes the construction of a levee system around the communities of Montz, Laplace, Reserve, and Garyville. The system will consist of approximately 18 miles of earthen levees and floodwalls, 4 floodgates, a drainage canal running parallel to the levee, 2 drainage structures, and 4 pump stations along the alignment. The flood protection features of final four reaches of the WSLP project (WSLP-111, WSLP-112, and WSLP-113) are to be constructed parallel to and immediately adjacent to the Maurepas Diversion. The main features of these reaches to be designed include an earthen tie-in to the Mississippi River levee, a gated crossing at River Road, a combination of levees and structural walls throughout the alignment, gated crossings at both the CN and KCS railroads, a raised crossing of Airline Highway, and a tie-in to the flood protection levee at Reach WSLP-110.

#### 4.0 Compilation of Existing Data

AECOM will gather the existing survey data from all sources relevant to the Maurepas Diversion and Reaches WSLP-111, WSLP-112, WSLP-113 and compile it into a merged electronic format. The compiled data will be submitted to C. H. Fenstermaker & Associates, LLC (Fenstermaker) for review and comment.

#### 5.0 Development of Survey Plan

Based on a review of the existing data and the comments received, Fenstermaker, in conjunction with AECOM, will develop a Draft Survey Data Collection Plan to acquire the survey information needed to perform engineering design on all aspects of the combined Maurepas Diversion and WSLP projects.

This will include surveying the area from the river to north of Airline Highway of the proposed Maurepas Diversion alignment, as the 2003 survey is long out of date and significant changes such as the construction of the Marathon Petroleum landside and the riverside facilities have been made. The area north of Airline Highway is swamp with no development as such a full re-surveying of this area may not be warranted, but the existing data needs to be spot checked and converted to the NAVD88, Epoch 2009.55, Geoid 12B datum. The area where the WSLP features will be constructed along the east side of the Maurepas Diversion has not been surveyed previously, so raw data will need to be collected for this area. In addition to collecting topographic data along the alignments of the two projects, additional data will be required at the road and railroad crossings.

Prior to conducting the planned surveying activities, the Survey Plan shall be submitted to both CPRA and the USACE Engineering Division Surveys Section for review and comment.

#### 6.0 Establishment of Survey Control

Fenstermaker will research NGS, CPRA, and USACE to locate any recently established and up-to-date survey control monuments that may exist with the project area. If survey control monuments cannot be located, Fenstermaker will coordinate with CPRA and USACE to establish first order survey control monuments for the combined Maurepas Diversion and WSLP projects. All surveys shall tie into a minimum of three Permanent Bench Marks, ideally located in the middle and at each end of the project, to ensure the reliability of the project's control. The vertical datum for both projects will be NAVD88, Epoch 2009.55, Geoid 12B. The project elevation data shall also be referenced to Local Mean Sea Level (LMSL). All geospatial data shall contain metadata which defines the relationship between NAVD88 and the LMSL local tidal datum. The horizontal datum for both projects will be NAD83, LA State Plane, South, U.S. Survey ft. Fenstermaker will coordinate the process of converting the existing data to the updated datums.

#### 7.0 Collection of Survey Data

Fenstermaker shall collect the survey data based on the Survey Plan using the control monumentation, to provide sufficient information for engineering design. All surveys shall be conducted in accordance with CEMVN-ED-SS-06-01, "USACE New Orleans District Guide for Minimum Survey Standards for Performing Hydrographic, Topographic, and Geodetic Surveys". GPS static networks shall follow the NGS Publication 58 guidelines for establishing vertical control. All RTK surveying shall be supported with documented ties to the project control for survey Quality Control.

Following the Survey Data Collection Plan, at AECOM's direction, Fenstermaker will collect the information needed for complete the design of the Maurepas Freshwater Diversion and WSLP Flood Protection Projects. Fenstermaker will provide the data collected under the plan, which will include a comprehensive survey (of one type or another) of all project areas from the Mississippi River to north of Interstate 10, including the area for the WSLP project adjacent to the diversion, to AECOM in electronic format. The survey shall be compliant with USACE guidelines, use proper survey monumentation, and be collected on the correct vertical and horizontal datums.

#### 8.0 General Description of Design Items

Includes all topographic surveying necessary for the design of:

- "River Reintroduction into Maurepas Swamp (PO-0029)" (Maurepas Diversion), which includes:
  - A headworks structure in the Mississippi River mainline Levee (MRL),
  - The relocation of River Road,
  - A sedimentation basin on the north side of River Road,
  - A 5½ mile long conveyance channel from the sedimentation basin to the Maurepas Swamp,
  - A culvert crossing at the CN RR,
  - A bridge crossing at the KCS RR,
  - A culvert crossing at U.S. Hwy 61, and
  - Restoration of the original cross-section of Hope Canal under Interstate 10.
- 2) "West Shore Lake Pontchartrain Flood Protection Project" (WSLP), which includes:
  - A levee tie-in to the MRL on the south side of River Road,
  - A roller gate crossing of River Road,
  - A levee section and I-Wall from River Road to the CN RR,
  - A gate crossing at the CN RR,
  - A levee section between the CN RR and the KCS RR.
  - A gate crossing at the KCS RR,
  - A levee section between the KCS RR and Airline Highway,
  - A 4,000-ft run along Airline Highway (which is to be raised), and
  - A levee section from Airline Highway to the tie-in point at WSLP Reach 15.

Baselines will be established for horizontal control and project benchmarks will be established for vertical control. This survey work will produce base maps for the engineering design of the elements required to create a complete set of construction plans. The topographic data, bathymetric data, cross-sections, and data collected in specific gridded areas will be used for both the design plans and for quantities calculations.

The specific Scope of Work - Survey Tasks, beginning at the Mississippi River and extending to the end of the project are described below along with the expected deliverables for each item.

A boundary survey has not been included in the scope of work stated below. AECOM will provide boundary information to be overlaid within the topographic data collected by Fenstermaker. This information will be for illustrative purposes only and has not been field verified for accuracy. As such, Fenstermaker can not be held liable for any issues that may come from the use of this data outside of its intended means.

#### 9.0 Scope of Work - Survey Tasks (Revised July 31, 2020)

#### **Survey Support for Clearing and Grubbing**

Prior to stake-out and magnetometer surveys of geotechnical boring locations, the Surveyor will provide survey support to the clearing and grubbing contractor along the clearing corridor detailed on maps provided by the geotechnical contractor. The survey crew will work in conjunction with clearing activities in an effort to minimize removal of trees and scrub brush beyond the required clearing limits.

#### **Stake and Magnetometer Survey of Geotechnical Boring Locations**

Once an area has been cleared of trees and scrub brush, the survey crew will perform pre-boring stakeout for the Geotechnical soils exploration program. The Surveyor will make a due diligence

effort to clear the boring locations of any hazards by performing a magnetometer survey. Upon completing the stakeout of the proposed boring locations, the results will be tabulated in an Excel spreadsheet with the following information in the required project datums: Boring Number; Latitude; Longitude; Northing; Easting; Elevation; Proposed Boring Depth. This spreadsheet will be provided to the geotechnical team for their reports and be included in the final Survey Report. Due to the limitations of survey equipment\*, please be advised that the surveyor cannot 100% ensure that no hazards exist within the boring areas.

#### Aerial LiDAR Survey using Unmanned Aerial Vehicle (UAV)

An Aerial LiDAR Survey will be performed over the entire project footprint to facilitate in providing the latest Aerial Orthoimagery for design plans and a Digital Surface Model (DSM) of existing infrastructure and ground features. Topographic surveys that will be performed on the ground will serve as ground truthing to validate elevations generated on the DSM. (Sub-consultant will be utilized for this service)

#### Mississippi River Bathymetric Surveys

Note: At the present time, Construction activities are currently underway along the riverfront of the Marathon Facility that may impact the bathymetric survey schedule. Communication with the Marathon Contractor Supervisor or Manager should be made to coordinate survey activities within and adjacent to the Mississippi River to maximize data collection.

#### Multibeam Survey:

Multibeam lines will be run parallel to the MRL B/L from the east bank of the river to 100-ft beyond the thalweg with a minimum of 50% overlap of the lines. Limits of the multi-beam survey are 1,500-ft upriver to 1,500-ft downriver of the Maurepas Diversion centerline. (Sub-consultant will be utilized for this service)

#### Single Beam Survey:

Single beam lines will be run parallel to the Maurepas Diversion centerline. Limits will be 1,600-ft upriver to 1,600-ft downriver from the diversion centerline. Run sections from edge of water into the shallow water out to a depth that allows at least a 100-foot overlap with the multibeam survey data. The transects shall be spaced at 200-ft intervals.

#### Magnetometer Survey:

Conduct a magnetometer survey within the Mississippi River to locate any hazards or anomalies, such as pipelines or debris. A minimum of three magnetometer survey lines will be run parallel to the bank-line to the outermost single beam survey transects. Due to the limitations of survey equipment\*, please be advised that the surveyor cannot 100% ensure that no hazards exist within the boring areas.

#### **Deliverables:**

- Survey results in AutoCAD Civil 3D, including a terrain surface of the combined multibeam and single beam data.
- ASCII files in a XYZ comma delimited format.
- · List of Anomalies with Point ID, Coordinates, Gammas, Durations, and Descriptions
- A survey report containing the methodology, field notes, tide information and a listing of survey points.

#### Mississippi River Levee & Batture

Recover USACE MRL B/L in the project area, from 1,000-ft upriver of the proposed diversion centerline to 2,500-ft below it. Take shots along the centerline of the levee at 50-foot intervals to create a profile.

Run transects from the River Road R/W, extending 100-ft beyond the MRL bank line to approximately 6-ft water depth such that they will tie into and overlap the multibeam and singlebeam bathymetric data collected in the river by at least 100-ft. Five of the transects shall be at 100-ft intervals centered on the Maurepas Diversion centerline, the remaining transects shall occur at 200-ft intervals. Take survey shots every 20-ft along the transects and at all grade breaks to include top of river bank, levee toes, levee crowns, slope paving, centerline of levee, water's edge, water surface shot, revetment, water bottom, etc. Survey break-lines along the centerline, tops of slope, and toes of slope along the existing MRL. Take break-line elevations at 50-ft intervals.

#### **Deliverables:**

- An AutoCAD Civil 3D drawing of the levee cross sections including a digital terrain model of the survey.
- Survey report, including survey methodology, field notes, coordinate listings and field data files.

#### LA Highway 44 (River Road)

Survey LA Highway 44 (River Road), for 1,500-ft in each direction from its intersection with the proposed Maurepas Diversion centerline. Run transects at **200-ft** intervals and centerline profiles of both east-bound and west-bound lanes at 50-ft intervals. Transects shall extend 50-feet beyond the R/W lines on each side of the highway. Locate all monumentation, above-ground utilities, culvert inverts, pipe sizes, manholes, power poles, road crossings, driveways, and other pertinent features.

#### **Deliverables:**

- Digital terrain model of survey area
- Utility Reports in DOTD format
- Structure reports in DOTD format
- Survey Report, with methodology, coordinate lists, field notes and survey field data file

#### Conveyance Channel and WSLP Levee - River Road to CN RR

Survey transects perpendicular to the Maurepas Diversion centerline alignment at 500-ft intervals. The transects shall extend from the eastern R/W to 30-ft beyond the western R/W. Shots along the transects shall be taken every 50-ft and at all grade changes that exceed 0.5-ft in elevation. Side shots, as necessary, shall be taken to pick up variations in topographic features such as ditches, meandering channels, broken marsh areas, or any existing features such as utility lines, power poles, etc. which may affect project design.

#### Deliverables:

- AutoCAD Civil 3D drawing of conveyance channel transects with a digital terrain model
- Survey report, containing a write up of survey methodology, coordinate files of the survey transect points, and coordinate files of the new survey data and survey field data files.

#### **Canadian National Railroad**

Survey **18 transects** perpendicular to the railroad at **200-ft** intervals. Transect limits will be 500-feet beyond Post Office Road/Daffodil Street to the east and 500-feet beyond the Marathon pipe

rack crossing to the west. Transects shall extend across the railroad R/W and 50-ft beyond, for a total width of 200-ft. Cross-sections will extend north to capture the shoo-fly area. Take survey shots every 20-feet along the transects and at all grade breaks. Pick-up all drainage features and ditches, as well as notable railroad features, including switches, signals, lights, frogs, ballast, etc.

#### Deliverables:

- AutoCAD Civil 3D drawing of Canadian National RR transects with a digital terrain model
- Survey report, containing a write up of survey methodology, coordinate files of the survey transect points, and coordinate files of the new survey data and survey field data files.

#### Conveyance Channel and WSLP Levee - CN RR to KCS RR

Survey transects perpendicular to the Maurepas Diversion centerline at 500-ft intervals. The transects shall extend across both the Maurepas Diversion and WSLP rights-of-way, including from the eastern internal railroad track of the Marathon Petroleum facility to 30-ft beyond the eastern R/W of the WSLP. Shots along the transects shall be taken every 50-ft and at all grade changes that exceed 0.5-ft in elevation. Side shots, as necessary, shall be taken to pick up variations in topographic features such as ditches, meandering channels, broken marsh areas, or any existing features such as utility lines, power poles, etc. which may affect project design.

#### Deliverables:

- AutoCAD Civil 3D drawing of conveyance channel transects with a digital terrain model
- Survey report, containing a write up of survey methodology, coordinate files of the survey transect points, and coordinate files of the new survey data and survey field data files.

#### Kansas City Southern Railroad

Survey **17 transects** perpendicular to the railroad at **200-ft** intervals. Transect limits will be 500-feet beyond LA 54 to the east and 2,500-feet beyond the LA 54 to the west. Transects shall extend across the railroad R/W and 25-ft beyond, for a total width of 150-ft. Cross-sections will extend north and south to capture the potential shoo-fly areas on each side. 5 cross-sections will be extended to cover the pipeline ROW to the north. Take survey shots every 20-feet along the transects and at all grade breaks. Pick-up all drainage features and ditches, as well as notable railroad features, including switches, signals, lights, frogs, ballast, etc.

#### Deliverables:

- AutoCAD Civil 3D drawing of Kansas City Southern RR transects with a digital terrain model
- Survey report, containing a write up of survey methodology, coordinate files of the survey transect points, and coordinate files of the new survey data and survey field data files.

#### LA Highway 54

Survey LA Highway 54, from 500-ft south of KCS RR to the U.S. 61 centerline. Run transects at 200-foot intervals with shots every 20-ft. Transects shall extend 25-feet beyond the R/W lines on each side of the roadway. Locate all above-ground utilities, culvert inverts, pipe sizes, manholes, power poles, road crossings, driveways, and other pertinent features. Also survey the roadway at 50-foot intervals along the centerline and create a profile.

#### **Deliverables:**

- Digital terrain model of survey area
- Utility Reports in DOTD format
- Structure reports in DOTD format
- Survey Report, with methodology, coordinate lists, field notes and survey field data files

#### Conveyance Channel and WSLP Levee – KCS RR to U.S. Highway 61

Survey transects perpendicular to the Maurepas Diversion centerline at 500-ft intervals. The transects shall extend across both the Maurepas Diversion and WSLP rights-of-way, including 25-ft beyond each R/W boundary line. Shots along the transects shall be taken every 50-ft and at all grade changes that exceed 0.5-ft in elevation. Side shots, as necessary, shall be taken to pick up variations in topographic features such as ditches, meandering channels, broken marsh areas, or any existing features such as utility lines, power poles, etc. which may affect project design. Pickup the pipeline and utility crossings that occur just south of U.S. Highway 61.

#### Deliverables:

- AutoCAD Civil 3D drawing of conveyance channel transects with a digital terrain model
- Survey report, containing a write up of survey methodology, coordinate files of the survey transect points, and coordinate files of the new survey data and survey field data files.

#### U.S. Highway 61 (Airline Highway)

Survey U.S. 61 (Airline Highway) for 2,500-ft on either side of the Maurepas Diversion centerline. Run transects at 200-ft intervals with shots every 20-ft. Transects shall extend 25-ft beyond the R/W lines on each side of the roadway. Locate all above-ground utilities, culvert inverts, pipe sizes, manholes, power poles, road crossings, driveways, and other pertinent features. Also survey the roadway at 50-ft intervals along the centerline and create a profile.

#### Deliverables:

- Digital terrain model of survey area
- Utility Reports in DOTD format
- Structure reports in DOTD format
- Survey Report, with methodology, coordinate lists, field notes and survey field data files

#### Conveyance Channel and WSLP Levee - U.S. Highway 61 to WSLP Levee End

Survey transects perpendicular to the Maurepas Diversion centerline at 500-ft intervals. The transects shall extend 25-ft beyond the diversion R/W on each side. Shots along the transects shall be taken every 50-ft and at all grade changes that exceed 0.5-ft in elevation. Side shots, as necessary, shall be taken to pick up variations in topographic features such as ditches, meandering channels, broken marsh areas, or any existing features such as utility lines, power poles, etc. which may affect project design.

#### Deliverables:

- AutoCAD Civil 3D drawing of conveyance channel transects with a digital terrain model
- Survey report, containing a write up of survey methodology, coordinate files of the survey transect points, and coordinate files of the new survey data and survey field data files.

#### Conveyance Channel – WSLP Levee End to Interstate 10

Survey transects perpendicular to the Maurepas Diversion centerline at 500-ft intervals. The transects shall extend 25-ft beyond the diversion R/W on each side. Shots along the transects shall be taken every 50-ft and at all grade changes that exceed 0.5-ft in elevation. Side shots, as necessary, shall be taken to pick up variations in topographic features such as ditches, meandering channels, broken marsh areas, or any existing features such as utility lines, power poles, etc. which may affect project design.

#### Deliverables:

- AutoCAD Civil 3D drawing of conveyance channel transects with a digital terrain model
- Survey report, containing a write up of survey methodology, coordinate files of the survey transect points, and coordinate files of the new survey data and survey field data files.

#### Interstate 10

Survey the existing Hope Canal underneath I-10 with hydrographic sections from 10-ft beyond topof-bank on both sides of the proposed Maurepas Diversion centerline (which is co-linear with the Hope Canal center line at the crossing). The sections shall extend 150-ft upstream of the eastbound lanes to 150-ft downstream of the westbound lanes. Shots shall be taken at 20-ft intervals along the transects. Pick-up all relevant features, including bridge bents, revetment, embankment tops and toes, etc.

#### **Deliverables:**

- Digital terrain model of survey area
- Utility Reports in DOTD format
- Structure reports in DOTD format
- Survey Report, with methodology, coordinate lists, field notes and survey field data files

#### Conveyance Channel - I-10 to Project End

Survey transects perpendicular to the Maurepas Diversion centerline at 500-ft intervals. The transects shall extend 25-ft beyond the diversion R/W on each side. Shots along the transects shall be taken every 50-ft and at all grade changes that exceed 0.5-ft in elevation. Side shots, as necessary, shall be taken to pick up variations in topographic features such as ditches, meandering channels, broken marsh areas, or any existing cadastral features, such as utility lines, power poles, etc. which may affect project design.

#### **Deliverables:**

- AutoCAD Civil 3D drawing of conveyance channel transects with a digital terrain model
- Survey report, containing a write up of survey methodology, coordinate files of the survey transect points, and coordinate files of the new survey data and survey field data files.

#### **Existing Pipelines**

Prior to locating existing pipelines, LA One Call (811) will be notified to facilitate with marking of existing pipelines. The survey team will perform a magnetometer survey to locate and mark all pipelines. Pipelines will be probed to determine depth of cover at 100-ft intervals within the Maurepas Diversion Corridor. Some existing pipelines have been identified utilizing various mapping database applications at the following locations:

- 1. Acadian Pipeline: LA Highway 44 (River Road) within the R/W and north of the roadway.
- 2. Acadian, Enterprise, and Gulf South Pipelines: North of and parallel with Canadian National Railroad
- 3. Bengal, Marathon, and Phillips 66 Pipelines: South of and parallel to U.S. Highway 61 (Airline Hwy)
- 4. Ascension Pipeline: North of and parallel to U.S. Highway 61 (Airline Hwy)
- 5. Semgroup, Phillips 66, and Enlink (Bridgeline) Pipelines: 1.45 miles North of and parallel to U.S. Highway 61 (Airline Hwy)

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#### Deliverables:

- AutoCAD Civil 3D drawing of located Pipelines
- Survey report, containing a write up of survey methodology, coordinate files of the survey points, field notes and depth of cover.

\*When requested, C. H. Fenstermaker & Associates, L.L.C. (Fenstermaker) uses pipeline locating equipment which may include magnetometers and/or gradiometers in an effort to locate underground and underwater pipelines in advance of drilling, construction, or abandonment activity. While reasonable efforts are made to locate all pipelines, the equipment used and the characteristics of pipelines themselves make it impossible to guarantee total success. Accordingly, it is incumbent upon the owners, operators and/or contractors conducting operations including dredging and excavation to conduct their operations with extreme caution and recognize that hazards in addition to those detected and marked by Fenstermaker may exist within the areas of operation in spite of Fenstermaker's most diligent efforts.

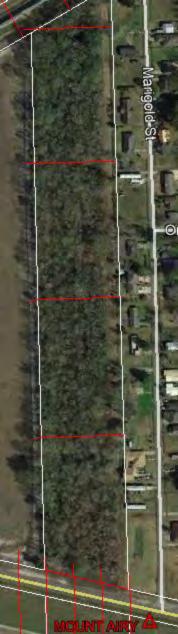
### Attachment B - Cost Estimate Breakdown

raurepas r Fask Number	reshwater Diversion and WSLP-111, WSLP-11	Subtotal	No. of Days Proposed	Potential Cost Reduction using UAV	Potential Time Savin using UAV	
ask Number		Subtotal	Froposed	using UAV	- Carxina	
1.0	Oversite and Management (Invoices, Progress Updates, etc.)	\$19,891.20		na	na	
2.0	Staking of Geotechnical Boring Locations	\$14,875.30	3	na	na	
2.1	Survey Support for Clearing (Addendum)	\$21,995.60	6	na	na	
3.0	Hydrographic Multibeam Survey	\$12,070.30	1	na	na	
4.0	Hydrographic Single Beam Survey	\$8,037.70	1	na	na	
5.0	Hydrographic Magnetometer Survey	\$7,640.85	1	na	na	
6.0	Mississippi River Levee & Batture Survey	\$23,023.00	5	\$4,604.60	1 Day Reduction	
7.0	LA 44 Survey (River Road)	\$24,257.20	5	\$4,851.44	1 Day Reduction	
8.0	Conveyance Channel and WSLP Levee Survey - River Road to CN RR	\$4,525.40	1	\$452.54	10% Reduction	
9.0	Canadian National Railroad Survey	\$20,015.60	4	\$10,007.80	2 Day Reduction	
10.0	Conveyance Channel and WSLP Survey- CN RR to KCS RR	\$21,406.00	5	\$4,281.20	1 Day Reduction	
11.0	Kansas City Southern Railroad Survey	\$18,342.50	4	\$9,171.25	2 Day Reduction	
12.0	Louisiana Highway 54 Survey	\$14,787.30	3	\$4,929.10	1 Day Reduction	
13.0	Conveyance Channel and WSLP Levee Survey - KCS to US 61	\$15,474.80	3	\$5,158.27	1 Day Reduction	
14.0	US 61 Survey (Airline Highway)	\$26,138.70	4	\$6,534.68	1 Day Reduction	
15.0	Conveyance Channel and WSLP Levee Survey - US 61 to WSLP Levee End	\$9,624.56	2	\$1,203.07	0.25 Day Reduction	
16.0	Conveyance Channel Survey - WSLP Levee End to I-10	\$38,498.24	8	\$8,421.49	1.75 Day Reduction	
17.0	Interstate 10 Survey	\$13,690.60	2	\$0.00	No Reduction	
18.0	Conveyance Channel Survey - I-10 to Project End	\$9,303.80	2	\$4,651.90	1 Day Reduction	
19.0	Aerial LiDAR Mapping and Orthoimagery	\$23,500.00	2			
	Total Project Cost Estimate:	\$347,098.65	62	\$64,267.33	13 Day Reduction	
	Potential Cost Savings using UAV:	\$64,267.33	13			
Res	vised Project Cost Estimate (Anticipated) :	\$282,831.32	49			

[[Contains Confidential, Proprietary, or Privileged Information Exempt from Public Disclosure.]]

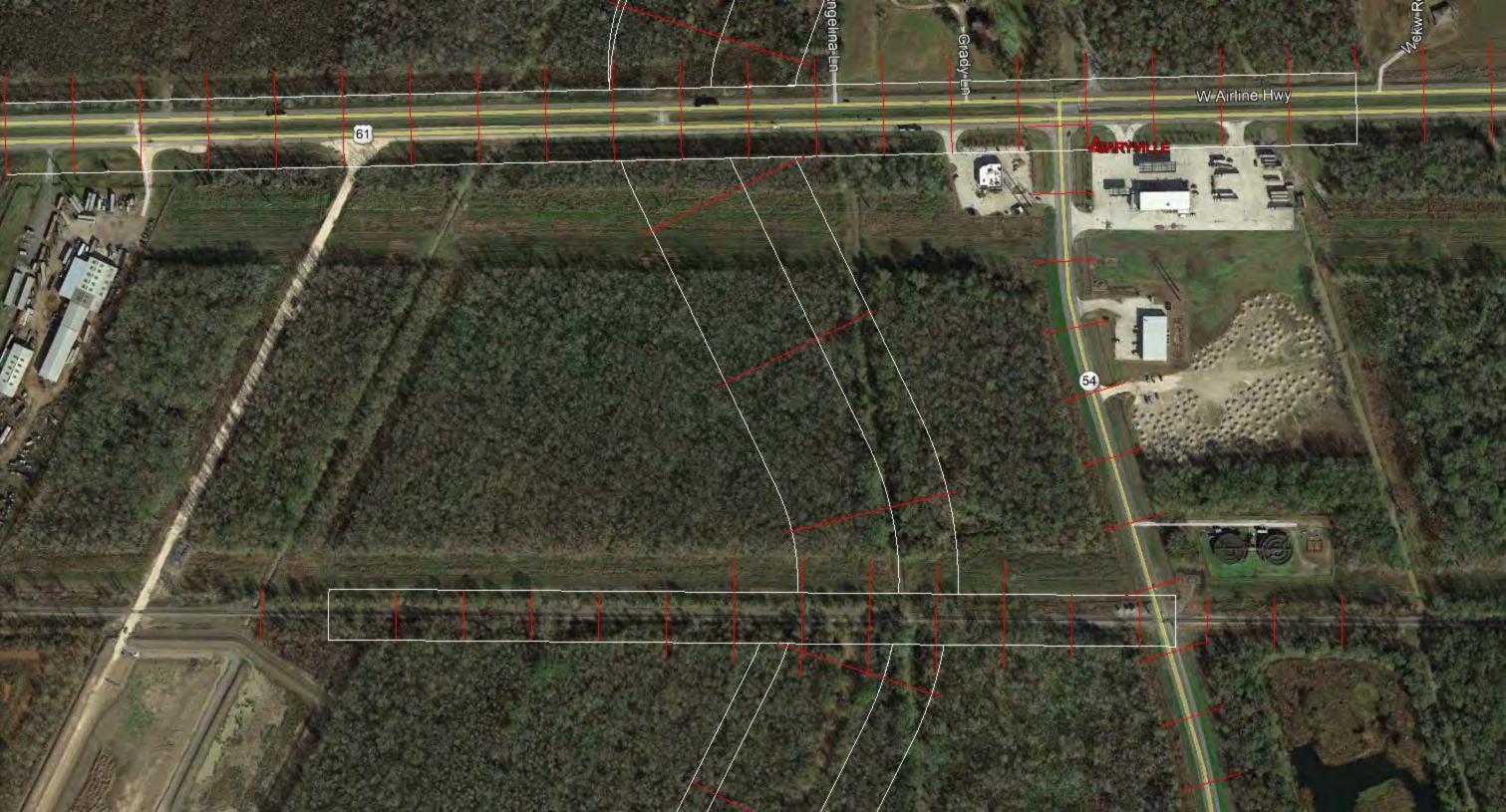
135 Regency Square | Lafayette, LA 70508 | 337.237.2200 phone | 337.232.3299 fax www.fenstermaker.com



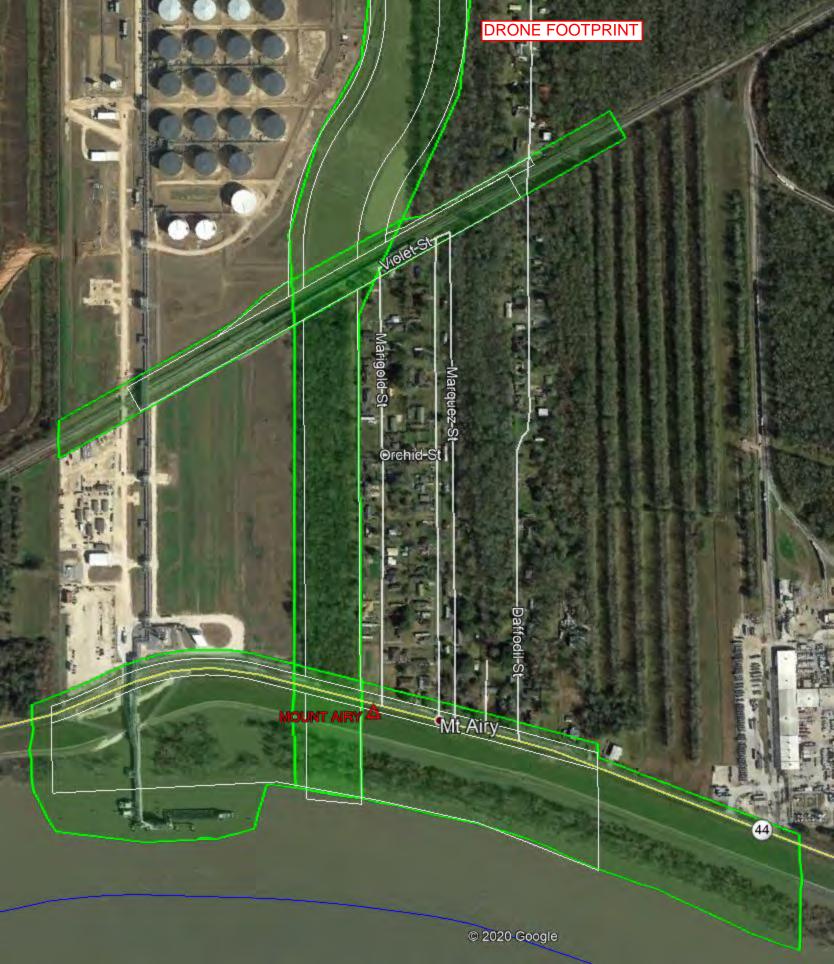




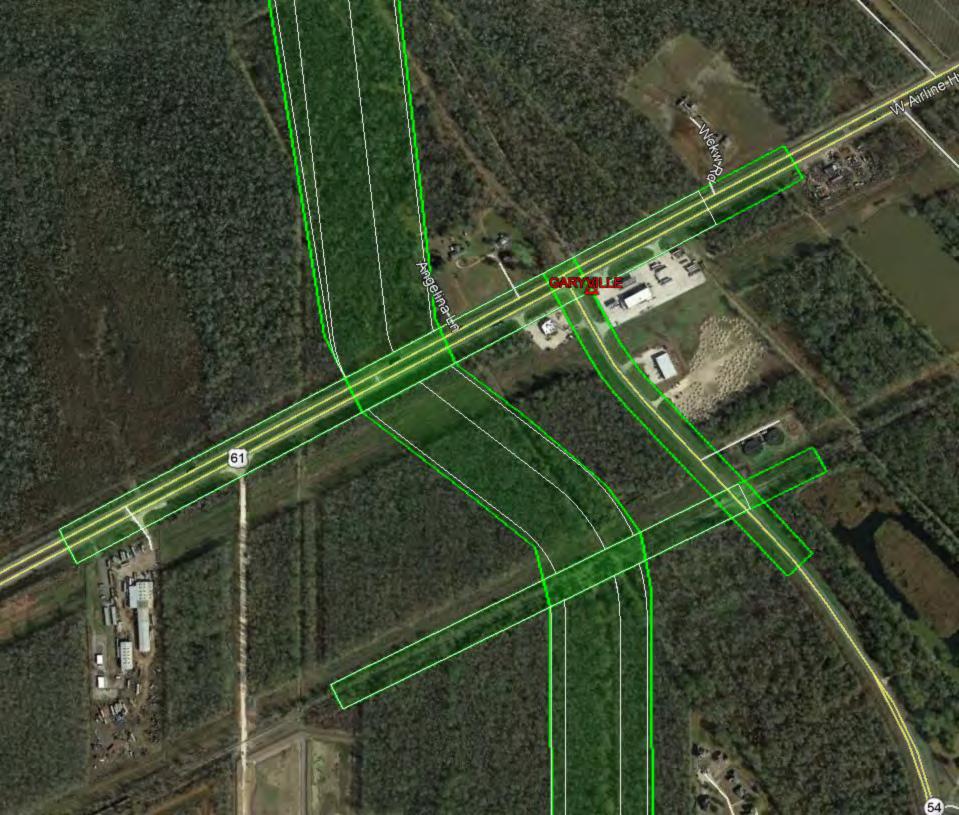




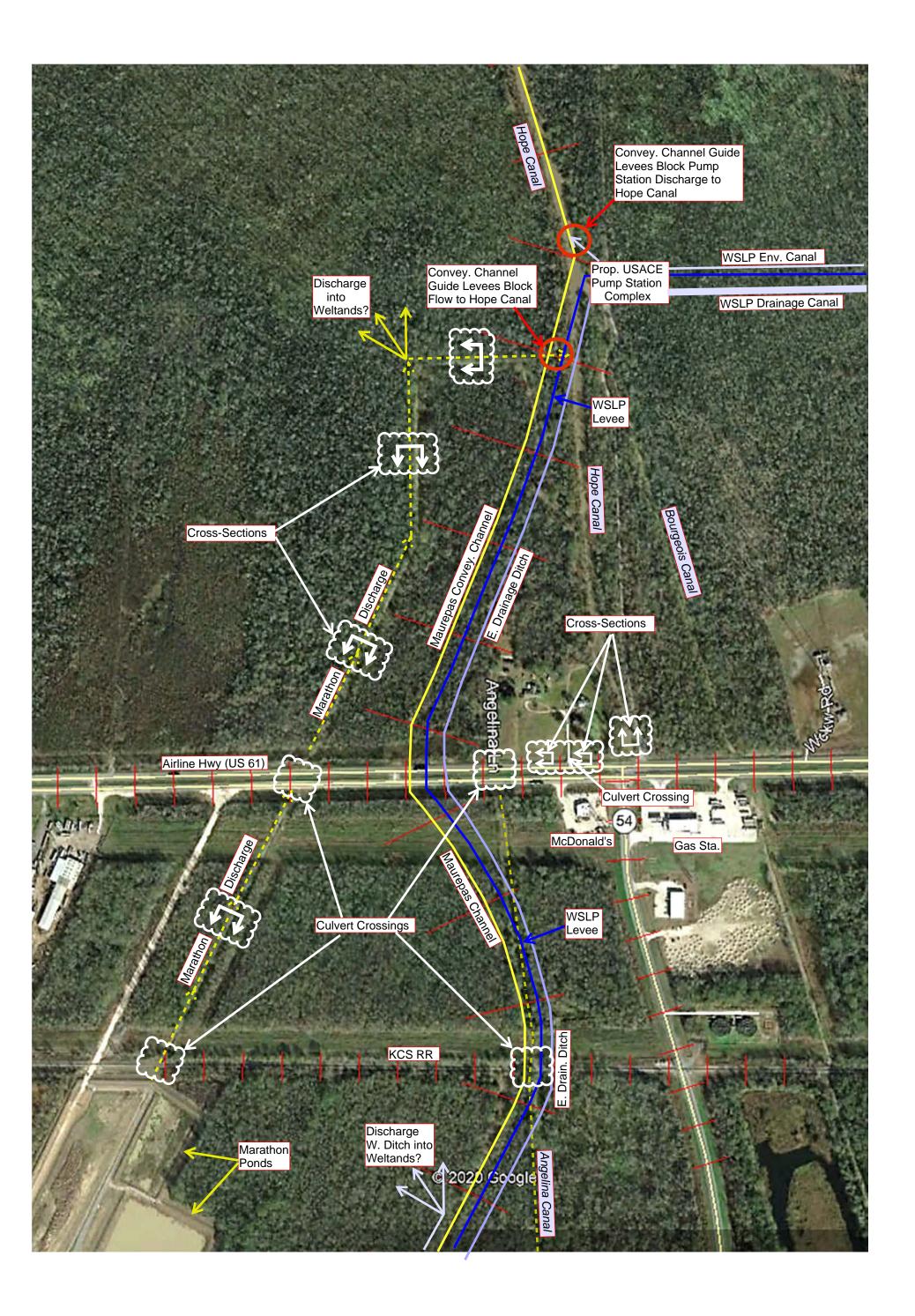














**AECOM** 

Maurepas Freashwater Diversion and Reaches WLSP-111, WSLP-112, WSLP-113 St. John the Baptist Parish, LA Page **11** of **12** 

## **Attachment A - Monument Datasheets**

## **Shared Solution**

PID: BBGG69

Designation: MOUNT AIRY

Stamping: MOUNT AIRY 2018

**Stability:** Most reliable; expected to hold position well **Setting:** Stainless steel rod in sleeve (10FT+ or 3.048M+)

Description: BEGINNING AT THE INTERSECTION OF HWY 54 AND HWY

44, HEAD NORTHWEST ALONG HWY 44 A DISTANCE OF A MILE MORE OR LESS TO A POINT. MARK IS 43.6 FT SOUTHWEST OF THE CENTER LINE OF HWY 44, 62 FT WEST OF THE CENTER LINE OF MARIGOLD ST, AND 15 FT OFF THE PROTECTED SIDE TOE OF LEVEE. MARK IS A HALF INCH STAINLESS STEEL ROD DRIVEN TO A DEPTH OF 44 FT ENCASED IN 6 INCH PVC SET IN CONCRETE WITH AN ALUMINUM ACCESS COVER STAMPED MOUNT

AIRY 2018.

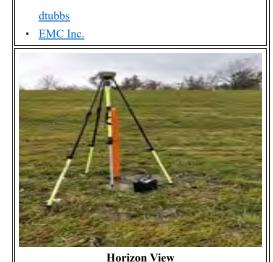
**CONTRIBUTED BY** 

**Observed:** 2018-12-13T12:57:00Z **Source:** OPUS - page5 1603.24



Close-up View

REF_FRAME: NAD_83(2011)	EPOCH: 2010.0000	SOURCE: NAVD88 (Computed using GEOID12B)	UNI	TS: SET PROFILE	DETAILS
LAT: 30° 3' 6.54		U	TM 15	<b>SPC</b> 1702(LA S)	
ELL HT: -24.188	$.27210" \pm 0.014 \text{ m}$ $\pm 0.007 \text{ m}$	NORTHING: 33		172212.961m 1066930.093m	
X: -61644.525 Y: -5525017.4		CONVERGENCE: 1.		0.34705556°	
<b>Z:</b> 3175335.0		POINT SCALE: 1.0 COMBINED FACTOR: 1.0		0.99992614 0.99992994	
<b>ORTHO HT:</b> 2.167	$\pm 0.021$ m				





The numerical values for this position solution have satisfied the quality control criteria of the National Geodetic Survey. The contributor has verified that the information submitted is accurate and complete.

## **Shared Solution**

PID: BBGG71

Designation: GARYVILLE

Stamping: GARYVILLE 2018

**Stability:** Most reliable; expected to hold position well **Setting:** Stainless steel rod in sleeve (10FT+ or 3.048M+)

**Description:** BEGINNING AT THE INTERSECTION OF HWY 54 AND THE

EASTBOUND LANE OF HWY 61, HEAD EAST ALONG HWY 61 A DISTANCE OF 108 FT TO A POINT. MARK IS 75 FT SOUTH OF THE CENTER LINE OF THE EASTBOUND LANE OF HWY 61 AND 26.3 FT SOUTHEAST OF A TRAFFIC SIGNAL POST. MARK IS A HALF INCH STAINLESS STEAL ROD DRIVEN TO A DEPTH OF 44 FT ENCASED IN 6 INCH PVC SET IN CONCRETE WITH AN ALUMINUM ACCESS

COVER STAMPED GARYVILLE 2018.

**Observed:** 2018-12-14T17:54:00Z **Source:** OPUS - page5 1603.24



Close-up View

IIII — I	EPOCH: 2010.0000	SOURCE: NAVD88 (Computed usin GEOID12B)	units m	S: SET PROFILE	DETAILS
LAT: 30° 4' 31.73693 LON: -90° 37' 29.468				SPC 1702(LAS)	
ELL HT: -24.878 X: -60242.582	± 0.012 m ± 0.008 m		<b>G:</b> 3329528.337m 1		

Y: -5523718.108 ± 0.012 m
Z: 3177604.947 ± 0.008 m
ORTHO HT: 1.524 ± 0.023 m

| CONVERGENCE: 1.19080000° 0.35425000° |
| POINT SCALE: 1.00024668 0.99992659 |
| COMBINED FACTOR: 1.00025059 0.99993050

#### CONTRIBUTED BY

dtubbs

• EMC Inc.





The numerical values for this position solution have satisfied the quality control criteria of the National Geodetic Survey. The contributor has verified that the information submitted is accurate and complete.



**VICINITY MAP** 

1 inch = 3,000 feet

Reproduced from NAIP Louisiana 2013 1m Aerial Imagery

### Station Name: PO29 SM 02

**Location:** Located approximately 4 miles northeasterly of Gramercy, Louisiana, in the median of Interstate 10, 2.5 miles east of La Highway 641 overpass in St. James Parish, Louisiana.

**Monument Description:** Monument is a 9/16 steel rod driven to 56 feet to refusal within a 6" PVC sleeve set in concrete with protective metal accesscove r stamped 'S2'

Stamping: S2

Installation Date: 20030929 Date of Survey: 2014

Monument Established By: 3001, Inc.

#### NAD83 (2011) Epoch 2010.00 Geodetic Position

Lat: 30°07'10.70180"N Long: 90°38'24.50682"W

#### NAD83 (2011) Epoch 2010.00 Datum LSZ (1702) Ft

N= 589,663.92' E= 3,499,986.35'

#### **Adjusted NAVD88 Height**

Elevation = 10.86 feet ( 3.311 mtrs )

Ellipsoid Height (2011.00) = -23.189mtrs.

Geoid12A Height = -26.500 mtrs.

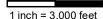
FOR REFERENCE ONLY
LCZ Adjusted NAVD88 Height
Elevation (Geoid09)= N/A
Ellipsoid Height = N/A
Elevation (Geoid03)= N/A
Ellipsoid Height = N/A
Elevation (Geoid99)= 11.35 feet (3.459mtrs.)
Ellipsoid Height = -23.169 mtrs.



 $Adjusted\ Position\ Established\ by\ John\ Chance\ Land\ Surveys,\ Inc.\ for\ the\ Coastal\ Protection\ \&\ Restoration\ Authority\ of\ Louisiana$ 



## **VICINITY MAP**



Reproduced from NAIP Louisiana 2013 1m Aerial Imagery

#### Station Name: 4437+49.06=PLMS 435

**Location:** Described by 3001, Inc 2003, the station is located in St. John the Baptist Parish at Garyville, La about 1.4 miles southeast of Mt. Airy, La and 4.15 miles east southeast of Gramercy, La ownership- USACE to reach the station from the intersection of State Highway 44 and Highway 3213 (Veterans Memorial Bridge) near Gramercy, La go 3.7 miles east along Highway 44 to Garyville, La and mark on right on top of levee. The station is a US CORP Engineers brass cap 0.6' below ground surface, stamped PLMS 435 1976-4437+49.06, 1.0' south of the f/s crown of levee, 6.6' south of centerline of levee and 125' south of centerline of hwy 44. The survey mark is an old type disk 4' round.

Monument Description: Survey Disk set in concrete monument

**Stamping:** 4437 49.06 PLMS 435 1976

Installation Date: UNK Date of Survey: 20100727

Monument Established By: USACE

#### NAD83 (2011) Epoch 2010.00 Geodetic Position

Lat: 30°02'14.06307"N Long: 90°37'17.74628"W

#### NAD83 (2011) Epoch 2010.00 Datum LSZ (1702) Ft

N= 559,734.48' E= 3,506,035.28'

#### **Adjusted NAVD88 Height**

Elevation = 31.54 feet ( 9.613 mtrs )

Ellipsoid Height (2011.00) = -16.702mtrs.

Geoid12A Height = -26.315 mtrs.

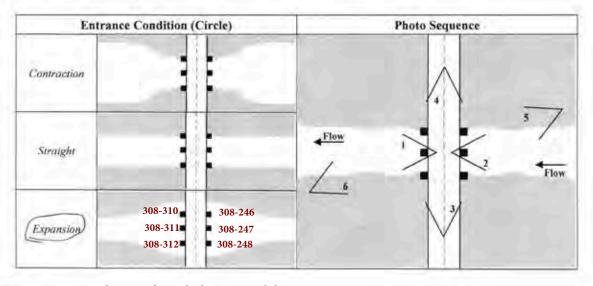
FOR REFERENCE ONLY
LCZ Adjusted NAVD88 Height
Elevation (Geoid09)= N/A
Ellipsoid Height = N/A
Elevation (Geoid03)= 31.40 feet ( 9.570mtrs.)
Ellipsoid Height = -16.753 mtrs.
Elevation (Geoid99)= N/A
Ellipsoid Height = N/A



Adjusted Position Established by John Chance Land Surveys, Inc. for the Coastal Protection & Restoration Authority of Louisiana

# Culvert Hydraulic Survey

Project Number: 2302328,000	Structure Number:
Date: 1/- 3-20	Road Name: And I Hay 61
Cross Sections: Upstream of Structure Downstream of Structure **Note- Cross sections should be located far enough	a away from road and side ditches to represent natural channel section.
Culvert Invert Elevation	
Channel Invert Elevation at Culvert	
Embankment Side Slope Rate	
Road Shots: Three shots (minimum) along cent	erline and Edge of Pavement
Roadway/Deck	
Type: Concrete Asphalt Timber Metal	Other:
Roadway/Deck Width (ft): 36	
Culvert/Pipe	
Number of Pipes: 1 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other: 48 RCP
Material: Concrete Corrugated Metal Smoot	th Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft):3	Height (ft): 3 Length (ft): 140 - 00
Entrance Type: Projecting Flat Headw	rall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 25 <10

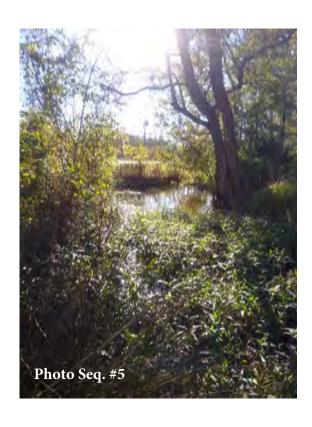


NOTES: See corresponding geo-referenced culvert positions below

# STRUCTURE #1





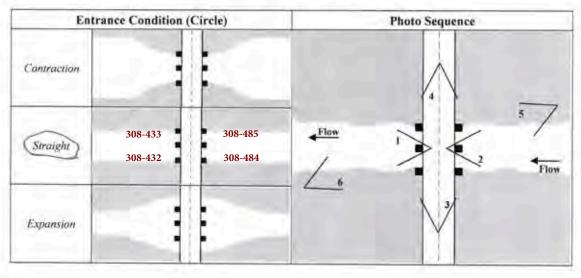




	(2011) Loui 8 GEOID 12		th Zone	1702,			= = :
POINT	x	Y	INVERT	LAT	LONG	ELLIPSOID	DESCRIPTION
308-246	3503156.44	573617.53	-1.09	N30°04'31.66792"	W90°37'49.53073"	-87.715'	PCUL1 ST
308-247	3503151.58	573617.30	-1.03	N30°04'31.66589"	W90°37'49.58607"	-87.653'	PCUL2 ST
308-248	3503141.78	573616.76	-2.02	N30°04'31,66114"	W90°37'49.69761"	-88.649	PCUL3 ST
308-310	3503151,75	573756.82	-1.17	N30°04'33.04695"	W90°37'49.574443"	-87.800'	PCUL1
308-311	3503147.04	573756.98	-1.28	N30°04'33.04891"	W90°37'49.62802"	-87.905'	PCUL2
308-312	3503138.14	573756.43	-1.93	N30°04'33.04397"	W90°37'49.72935"	-88.558'	PCUL3

# Culvert Hydraulic Survey

Project Number: 2302328.00C	Structure Number:	2
Date: 1/-3-20	Road Name: Han wil	
Cross Sections: Upstream of Structure Downstream of Structure **Note- Cross sections should be located far enough	th away from road and side ditches to	o represent natural channel section
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along cer		
Roadway/Deck		
Type: Concrete Asphalt Timber Metal	Other:	-2
Roadway/Deck Width (ft): 36		
Culvert/Pipe		
Number of Pipes: 1 2 3	Other:	
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:	48 ec7
Material: Concrete Corrugated Metal Smoo	oth Metal Corrugated Plastic Sn	nooth Plastic Other:
nside Diameter/Width (ft): 3	Height (ft): _ 3	Length (ft): 140,00
Entrance Type: Projecting Flat Head	wall Flat Headwall w/ Wingwalls	Other:
Culvert Opening Clogged (%): 100 75	50 25 <10	



NOTES: See corresponding geo-referenced culvert positions below

# STRUCTURE #2



Photo Seq. #2



Photo Seq. #5



Photo Seq. #4



Photo Seq. #1



Photo Seq. #6



Photo Seq. #3

	A Probability of the Control of the	1.01 - 0.1			-		
VAD83	(2011) Los	uisiana So	uth Zon	e 1702,		-	
NAVD8	8 GEOID 1	2B					
POINT	X	Y	INVERT	LAT	LONG	ELLIPSOID HEIGHT	DESCRIPTION
308-432	3504182.36	573786.66	-1.14	N30°04'33.27969"	W90°37'37,84220"	-87.768'	PCUL1 ST
308-433	3504190.89	573786.92	-1.27	N30°04'33.28174"	W90°37'37.74509"	-87.897 <sup>t</sup>	PCUL2 ST
308-484	3504186.23	573646.81	-0.98	N30°04'31.89500"	W90°37'37.80800"	-87.604	PCUL2
308-485	3504195.79	573647	-0.92	N30°04'31.89633"	W90°37'37.69918"	-87,544'	PCUL1

	Structure Number: # 3 Road Name: Railroad Tracks
Cross Sections: Upstream of Structure Downstream of Structure	away from road and side ditches to represent natural channel section.
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along center	
Roadway/Deck  Type: Concrete Asphalt Timber Metal (	Other: Genel
Roadway/Deck Width (ft):	Other: Genel
Culvert/Pipe	
Number of Pipes: 1 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Meta Smooth	Metal Corrugated Plastic Smooth Plastic Other
Inside Diameter/Width (ft): _ *\%	leight (ft): Length (ft): 57.5
Entrance Type: Projecting Flat Headwall	Flat Headwall w/ Wingwalls Other:
C-1 0	0 25 <10

Entra	ance Condition	(Circle)	Pho	to Sequence
Contraction				1
Straight	310-150	310-146	dFlow 1	5 / 2 File
Expansion				3

NOTES: See corresponding geo-referenced culvert positions below

#### **STRUCTURE #3**

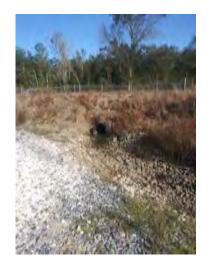


Photo Seq. #5

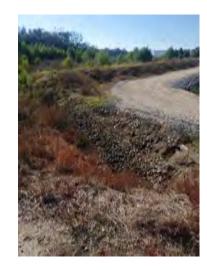


Photo Seq. #2



Photo Seq. #4



Photo Seq. #1



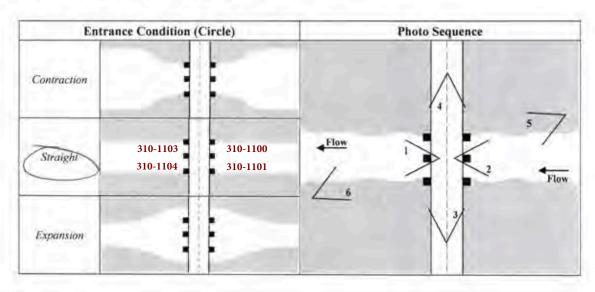
Photo Seq. #6



Photo Seq. #3

	(2011) Loi 8 GEOID 1:	A Comment	uth Zon	ie 1702,			
POINT	Х	Y	INVERT	LAT	LONG	<b>ELLIPSOID HEIGHT</b>	DESCRIPTION
310-146	3502421.15	572218.93	0.19	N30°04'17.86769"	W90°37'57.99694"	-86.412'	PCUL1 ST
310-150	3502433.37	572253.73	0.48	N30°04'18.21147"	W90°37'57.85541"	-86.125'	PCUL1

Date: 11-6-20 Structure Number: # 7 Road Name: Radrond Tracks
Cross Sections: Upstream of Structure Downstream of Structure
**Note- Cross sections should be located far enough away from road and side ditches to represent natural channel section.
Culvert Invert Elevation
Channel Invert Elevation at Culvert
Embankment Side Slope Rate
Road Shots: Three shots (minimum) along centerline and Edge of Pavement
Roadway/Deck
Type: Concrete Asphalt Timber Metal Other: Great
Roadway/Deck Width (ft):
Culvert/Pipe
Number of Pipes: 1 2 3 Other:
Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): 48 Height (ft): Length (ft): 58.0
Entrance Type: Projecting Flat Headwall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75 50 25 <10



NOTES: See corresponding geo-referenced culvert positions below

### STRUCTURE #4



Photo Seq. #1



Photo Seq. #2

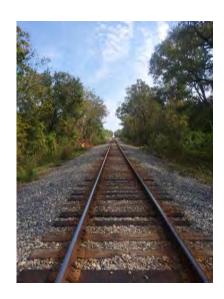


Photo Seq. #3



Photo Seq. #4



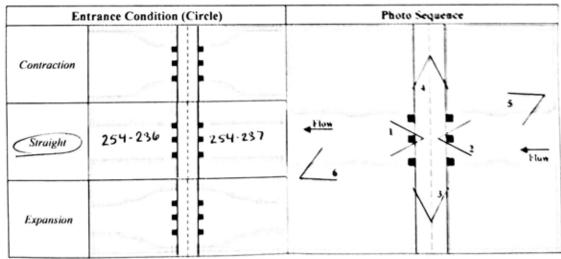
Photo Seq. #5



Photo Seq. #6

NIADOS	(2011) Lou	islana Cou	th Zon	. 1702			
	3 GEOID 12		ith Zon	e 1702,			
MANDO			-				
POINT	х	Y	INVERT	LAT	LONG	<b>ELLIPSOID HEIGHT</b>	DESCRIPTIO
	X 3504369.53	Y .572205.66	-2.02	LAT 30d04'17.61793"	LONG -90d37'35.82294"	-88.616	PCUL1 ST
POINT			0.100				PCUL1 ST
POINT 310-1100	3504369.53	572205.66	-2.02	30d04'17.61793"	-90d37'35.82294"	-88.616	

•	•
Project Number: 2202328.00 Struct	ure Number: #5
Project Number: 2202328.00 Struct Date: 10 September 2020 Road	Name: US 61 /Areins Har-1 /10, Warran
Cross Sections: Upstream of Structure  Downstream of Structure  **Note- Cross sections should be located far enough away from	
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerline and	
Roadway/Deck	
Type: Concrete (Asphala Timber Metal Other:	Gener Devening
Roadway/Deck Width (ft): 20	,
Culvert/Pipe	
Number of Pipes: 1 2 3 Other:	
Shape/Type of Pipe: Circular Box Arch Ellips	e Irregular Other
Material: Concrete Corrugated Metal Smooth Metal	Corrugated Plastic Smooth Plastic Other 307
Inside Diameter/Width (ft): _ 36 " Height	(ft): Length (ft): 32.75
Entrance Type: Projecting Flat Headwall Fla	t Headwall w/ Wingwalls Other
Culvert Opening Clogged (%): 100 75 50	25) <10
Entrance Condition (Circle)	Photo Sequence



NOTES: See corresponding geo-referenced culvert positions below

## STRUCTURE #5



	(2011) Lou 8 GEOID 12		th Zone	1702,			
POINT	X	Y	INVERT	LAT	LONG	<b>ELLIPSOID HEIGHT</b>	DESCRIPTION
254-236	3504516.72	573801.72	-2.26	N30°04'33.40829"	W90°37'34.03558"	-88.886'	PCUL1
254-237	3504549.47	573801.73	-3,25	N30°04'33.40638"	W90°37'33.66282"	-89.868'	PCUL1

Project Number: ZZ02528,00C	Structure Number: pt 10-11
Date: 12-15-20	Road Name: Name:
Cross Sections: Upstream of Structure Downstream of Structure **Note- Cross sections should be located far enough	h away from road and side ditches to represent natural channel section
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along cen	nterline and Edge of Pavement
Roadway/Deck	
Type: Concrete Asphalt Timber Metal	Other: Dirt
Roadway/Deck Width (ft):	
Culvert/Pipe	
Number of Pipes: 1 2 3	Other:
Shape/Type of Pipe: Circula Box Arch	n Ellipse Irregular Other:
	oth Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft):	Height (ft): 40.3
Entrance Type: Projecting Flat Head	wall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	(50) 25 <10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		3

NOTES:		

	Road Name:
Cross Sections: Upstream of Structure Downstream of Structure	
*Note- Cross sections should be located far enough	away from road and side ditches to represent natural channel section.
Culvert Invert Elevation	
Channel Invert Elevation at Culvert	
Embankment Side Slope Rate	Ten A (120 - 500) - 77777
Road Shots: Three shots (minimum) along center	rline and Edge of Pavement
Roadway/Deck	
Type: Concrete Asphalt Timber Metal	Other: LinesLand
Roadway/Deck Width (ft): / 0. 0	
Culvert/Pipe	
Number of Pipes: 1 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smooth	n Metal Corrugated Plastic Smooth Plastic Other;
Inside Diameter/Width (R):/	Height (ft): / Length (ft): 40.7
Entrance Type: Projecting Flat Headwa	all Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 25 <10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		

NOTES:		

Project Number: 2202328.00 C Structure Number: p+ 14-15 Date: 12-15-20 Road Name:	
Cross Sections: Upstream of Structure	
Downstream of Structure  *Note- Cross sections should be located far enough away from road and side ditches to represent natural channel s	section
Culvert Invert Elevation	
Channel Invert Elevation at Culvert	
Embankment Side Slope Rate	
Road Shots: Three shots (minimum) along centerline and Edge of Pavement	
Roadway/Deck	
Type: Concrete Asphalt Timber Metal Other: Lines Line	
Roadway/Deck Width (ft): /O, o	
Culvert/Pipe	
Number of Pipes: 1 2 3 Other:	
miner or their	
Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:	
Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:	
Inside Diameter/Width (ft): 1.5 Height (ft): 1.5 Length (ft): 34.5	
Entrance Type: Projecting Flat Headwall Flat Headwall w/ Wingwalls Other:	
Culvert Opening Closeed (%): 100 75 50 (5) <10	

Entrance (	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		2 6

NOTES:			

Project Number: 22 0 23 28.000 Date: 12-13-20	Structure Number: + 16-17 Road Name:
Cross Sections: Upstream of Structure Downstream of Structure	
**Note- Cross sections should be located far enough	away from road and side ditches to represent natural channel section.
Culvert Invert Elevation	
Channel Invert Elevation at Culvert	
Embankment Side Slope Rate	Alexander and the second second
Road Shots: Three shots (minimum) along center	erline and Edge of Pavement
Roadway/Deck	
Type: Concrete Asphalt Timber Metal	Other: Linestore
Roadway/Deck Width (ft):	
Culvert/Pipe	
Number of Pipes: 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smooth	h Metal Corrugated Plastic Smooth Plastic Other
Inside Diameter/Width (ft):/. \$	Height (ft): 1. 5 Length (ft): 3 %, 0
Entrance Type: Projecting Flat Headwa	all Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 25 <10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow Flor
Expansion		3

NOTES:		

Project Number: 22 > 2328.000	Structure Number: 24 18-19
Date: /2-/5-20	Road Name:
Cross Sections: Upstream of Structure Downstream of Structure **Note- Cross sections should be located far enough	away from road and side ditches to represent natural channel section
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along center	erline and Edge of Pavement
Roadway/Deck	
Type: Concrete Asphalt Timber Metal	Other: LinesLac
Roadway/Deck Width (ft): 10 0	
Culvert/Pipe	
Number of Pipes: (1) 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smoot	th Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft):/ . 5	Height (ft): 1, 5 Length (ft): 34. 4
Entrance Type: Projecting Flat Headwi	all Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 (25) <10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		3

NOTES:	

Project Number: 2202324.00C	Road Name:
Cross Sections: Upstream of Structure  Downstream of Structure  *Note: Cross sections should be located for enough	away from road and side ditches to represent natural channel section
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along cent	
Roadway/Deck	
Type: Concrete Asphalt Timber Metal Roadway/Deck Width (ft): 10.0	Other: Linestone
Culvert/Pipe	
Number of Pipes: 1 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smoo	
nside Diameter/Width (ft):/. 5	Height (ft): 1.5 Length (ft): 3 4.0
Entrance Type: Projecting Flat Headw	vall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 25 <10

Entrance (	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow Flow
Expansion		3

DTES:			

Project Number: 2202328,000 Structure Number: 22-23  Date: 12-15-20 Road Name:
Cross Sections: Upstream of Structure  Downstream of Structure  **Note- Cross sections should be located far enough away from road and side ditches to represent natural channel sections.
Culvert Invert Elevation
Channel Invert Elevation at Culvert
Embankment Side Slope Rate  Road Shots: Three shots (minimum) along centerline and Edge of Pavement
and any any and any any and any and any any any and any any and any and any and any and any any and any
Roadway/Deck
Type: Concrete Asphalt Timber Metal Other: Line star
Roadway/Deck Width (ft): /O , D
Culvert/Pipe
Number of Pipes: 1 2 3 Other:
Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): /.3 Height (ft): /.3 Length (ft): 52.0
Entrance Type: Projecting Flat Headwall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75 50 25 <10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		26

OTES:			

Project Number: 2202325, 000 S Date: (2-15-20 R	oad Name:
Cross Sections: Upstream of Structure Downstream of Structure **Note- Cross sections should be located far enough aw	yay from road and side ditches to represent natural channel section.
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerli	ne and Edge of Pavement
Roadway/Deck  Type: Concrete Asphalt Timber Metal O	ther: Longstone
Roadway/Deck Width (ft):	
Culvert/Pipe	
Number of Pipes: 2 3 0	ther:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smooth M	Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft):/. 5	eight (ft): 1,5 Length (ft): 34,5
Entrance Type: Projecting Flat Headwall	Flat Headwall w/ Wingwalls Other;
Culvert Opening Clogged (%): 100 75 (5)	25 <10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow Flow
Expansion		3

NOTES:			
			_

Project Number: 2202328.00 C	Road Name: Ry E Tracks
Cross Sections: Upstream of Structure Downstream of Structure	
*Note- Cross sections should be located far enoug	th away from road and side ditches to represent natural channel section.
Culvert Invert Elevation	
Channel Invert Elevation at Culvert	
Embankment Side Slope Rate	
Road Shots: Three shots (minimum) along cen	sterline and Edge of Pavement
Roadway/Deck	
	Other: Tracks
Type: Concrete Asphalt Timber Metal	Other: Traces
Roadway/Deck Width (ft): 27.0	
Culvert/Pipe	
Number of Pipes: 2 3	Other:
Tunnet vi tipes	57701
Shape/Type of Pipe: Circular Box Arch	the Ellipse Irregular Other:
Material Concrete Corrugated Metal Smoo	oth Metal Corrugated Plastic Smooth Plastic Other:
raterian Concietes Configured Means Since	
Inside Diameter/Width (ft): 3	Height (ft): 3 Length (ft): 55.0
Entrance Type: Projecting Flat Head	wall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 (25) <10
Culvert Opening Clogged (%): 100 75	50 25 10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 2 Flor
Expansion		3

	-	-			1
NOTES					
1-2-					

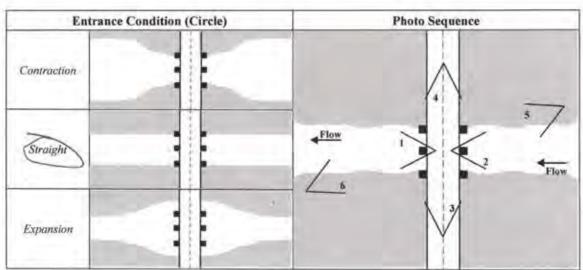
#### sivert Hydraulic Survey

Project Number: 220232 8.00 C	Structure Number: 3+ 68-65
Date: /2-15-20	Structure Number: 3+ 68-69 Road Name. Poge live See
Cross Sections: Upstream of Structure Downstream of Structure	
and the state of the state of	way from road and side ditches to represent natural channel section.
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along center	line and Edge of Pavement
Roadway/Deck	
Type: Concrete Asphalt Timber Metal	Other:
Roadway/Deck Width (ft): 15.0	
Culvert/Pipe	
Number of Pipes: 1 2 3	Other
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smooth	Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): 3,5	Height (ft): 31.5 Length (ft): 31.0
Entrance Type: Projecting Flat Headwal	Il Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 25 <10

Entrance (	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 1 Flow
Expansion		3

NOTES:			

Project Number: 2702328 00° Structure Number: 3+ 55-56  Date: 12-15-20 Road Name: Marigold 5±
Cross Sections: Upstream of Structure Downstream of Structure
**Note- Cross sections should be located far enough away from road and side ditches to represent natural channel section.
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerline and Edge of Pavement
Roadway/Deck
Type: Concrete Asphalt Timber Metal Other:
Roadway/Deck Width (ft): 23,0
Culvert/Pipe
Number of Pipes: 1 2 3 Other:
Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): 2.5 Height (ft): 2.5 Length (ft): 40.1
Entrance Type: Projecting Flat Headwall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75 50 25 <10



TES.			

Project Number: 2202324,000	Structure Number: p+ 75-80 Road Name: RxR Tracks
Date: 12-15-20	Road Name: RAR Tracks
Cross Sections: Upstream of Structure Downstream of Structure	
**Note- Cross sections should be located far enoug	h away from road and side ditches to represent natural channel section
Culvert Invert Elevation	
Channel Invert Elevation at Culvert Embankment Side Slope Rate	
Road Shots: Three shots (minimum) along cen	sterline and Edge of Pavement
The most financially mong cen	and and and an
Roadway/Deck	
Type: Concrete Asphalt Timber Metal	Other: Tracks
Roadway/Deck Width (ft): 27,0	
Culvert/Pipe	
Number of Pipes: (1) 2 3	Other:
	must be added
Shape/Type of Pipe: (Circular) Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal (Smooth	oth Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): 3 - 5	Height (ft): 3, 5 Length (ft): 6 5 - 3
Entrance Type: Projecting Flat Heady	wall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 25 <10
CHIPCIES AMERICAN ADDIVICULT OF THE CO.	

Entrance C	ondition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		

NOTES:			

Project Number: 2202324 000 Structure Number: 5+ 41-82	
Date: 12-15-20 Road Name: Page link St	
Cross Sections: Upstream of Structure  Downstream of Structure  **Note- Cross sections should be located far enough away from road and side ditches to represent natural cha	annel secti
Culvert Invert Elevation	
Channel Invert Elevation at Culvert	
Embankment Side Slope Rate  Road Shots: Three shots (minimum) along centerline and Edge of Pavement	
Thee store (minimum) dong semestine and mage of Carendan	
Roadway/Deck	
Type: Concrete Asphalt Timber Metal Other:	
Roadway/Deck Width (ft): 20.0	
Culvert/Pipe	
Number of Pipes: 1 2 3 Other:	
Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:	
Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other	
Material: Concrete Cortugated victal Shooth victal Cortugated riashe Shooth riashe Orner	,
Inside Diameter/Width (ft): /. S Height (ft): /. S Length (ft): 6	1.3
Entrance Type: Projecting Flat Headwall Flat Headwall w/ Wingwalls Other:	_
Culvert Opening Clogged (%): 100 75 50 (25) <10	

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion	1	

OTES:		

Culvert Hydraulic Survey Project Number: 220232 6.000 Structure Number: Road Name: Date: 12-15-20 Cross Sections: Upstream of Structure Downstream of Structure \*\*Note- Cross sections should be located far enough away from road and side ditches to represent natural channel section. Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Three shots (minimum) along centerline and Edge of Pavement Road Shots: Roadway/Deck Type: Concrete (Asphalt) Timber Metal Other: Roadway/Deck Width (ft): 20.0 Culvert/Pipe Number of Pipes: Other! (Circular) Box Ellipse Irregular Other: Shape/Type of Pipe: Arch

Height (ft):

Flat Headwall w/ Wingwalls

Length (ft):

Other:

Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic

Flat Headwall

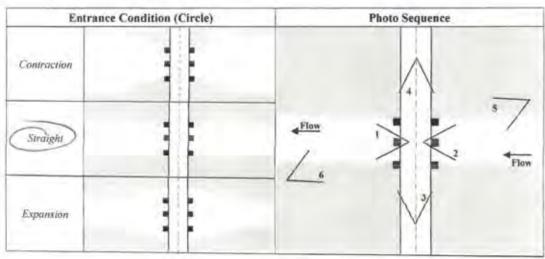
75

Projecting

Inside Diameter/Width (ft):

Culvert Opening Clogged (%): 100

Entrance Type:



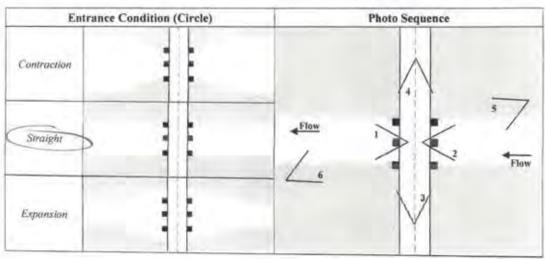
	1.1		
DTES:			

Project Number: 2202324, occ Structure Number: p + 97-94  Date: 12-15-20 Road Name: Description Structure  Downstream of Structure  Downstream of Structure  **Note- Cross sections should be located far enough away from road and side ditches to represent natural channel structure the section of Structure  Culvert Invert Elevation  Channel Invert Elevation at Culvert  Embankment Side Slope Rate  Road Shots: Three shots (minimum) along centerline and Edge of Pavement  Roadway/Deck  Type: Concrete Asphall Timber Metal Other:  Roadway/Deck Width (ft): Zo. O  Culvert/Pipe  Number of Pipes: 1 2 3 Other:  Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:	ct Number: 2.2 02324, 00C	Structure Number	: p+	97-98
Downstream of Structure  **Note- Cross sections should be located far enough away from road and side ditches to represent natural channel sections.  Culvert Invert Elevation  Channel Invert Elevation at Culvert  Embankment Side Slope Rate  Road Shots: Three shots (minimum) along centerline and Edge of Pavement  Roadway/Deck  Type: Concrete Asphall Timber Metal Other:  Roadway/Deck Width (ft): Zo.o  Culvert/Pipe  Number of Pipes: 1 2 3 Other:	12-15-20	Road Name:	rage hora	5+
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerline and Edge of Pavement  Roadway/Deck Type: Concrete Asphal) Timber Metal Other:  Roadway/Deck Width (ft): Z 0. 0  Culvert/Pipe Number of Pipes: 1 2 3 Other:				
Channel Invert Elevation at Culvert  Embankment Side Slope Rate  Road Shots: Three shots (minimum) along centerline and Edge of Pavement  Roadway/Deck  Type: Concrete Asphall Timber Metal Other:  Roadway/Deck Width (ft): Z 9. 0  Culvert/Pipe  Number of Pipes: 1 2 3 Other:	- Cross sections should be located far enough	away from road and side	e ditches to repr	resent natural channel sec
Road Shots: Three shots (minimum) along centerline and Edge of Pavement  Roadway/Deck Type: Concrete Asphal) Timber Metal Other:  Roadway/Deck Width (ft): Z 5. 5  Culvert/Pipe Number of Pipes: 1 2 3 Other:	el Invert Elevation at Culvert			
Type: Concrete Asphal) Timber Metal Other:  Roadway/Deck Width (ft): Z 5, 5  Culvert/Pipe  Number of Pipes: 1 2 3 Other:		rline and Edge of Paver	ment	
Type: Concrete Asphal) Timber Metal Other:  Roadway/Deck Width (ft): Z 5. 5  Culvert/Pipe  Number of Pipes: 1 2 3 Other:	vay/Dark			
Roadway/Deck Width (ft): Z 5. 5  Culvert/Pipe Number of Pipes: 1 2 3 Other:				
Culvert/Pipe Number of Pipes: 2 3 Other:	Concrete (Asphal) Timber Metal	Other;		
Number of Pipes: 2 3 Other:	vay/Deck Width (ft): 20,0			
	rt/Pipe			
Shape/Type of Pipe: Circular Box Arch Ellipse irregular Other:	er of Pipes: 1 2 3	Other:		
	Type of Pipe: Circular Box Arch	Ellipse irregular	Other:	
Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:	ial: Concrete Corrugated Metal Smoot	Metal Corrugated P	lastic Smooth	n Plastic Other:
Inside Diameter/Width (ft): 1.5 Height (ft): 1.5 Length (ft): 54.3	Diameter/Width (ft):/, 5	Height (ft): /, \$	Le	ength (ft): 54.3
Entrance Type: Projecting Flat Headwall Flat Headwall w/ Wingwalls Other:	nce Type: Projecting Flat Headwa	dl Flat Headwall w/	Wingwalls (	Other:
Culvert Opening Clogged (%): 100 75 50 25 <10	rt Opening Clogged (%): 100 75	50 25 <10		

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		3

1-1	111		0 -	
NOTES:				

Culvert Hydraulic Survey Project Number: 22 02328.00C Structure Number: Road Name: Date: 12-15-20 Cross Sections: Upstream of Structure Downstream of Structure \*\*Note- Cross sections should be located far enough away from road and side ditches to represent natural channel section. Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Three shots (minimum) along centerline and Edge of Pavement Road Shots: Roadway/Deck Type: Concrete (Asphalt Timber Metal Other: 20.0 Roadway/Deck Width (ft): Culvert/Pipe Number of Pipes: Other: Circular Box (Arch) Ellipse Irregular Other: Shape/Type of Pipe: Material: (Concrete) Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other: Length (ft): 70.5 Inside Diameter/Width (ft):\_\_\_ Height (ft):



Flat Headwall w/ Wingwalls

Entrance Type:

Projecting

Culvert Opening Clogged (%): 100 75

Flat Headwall

Project Number: 2202324,000 Str	ructure Number: by # 110 -117
Date: 12-15-2 Ro	ad Name: Rv R Tracks
Cross Sections: Upstream of Structure  Downstream of Structure  **Note- Cross sections should be located far enough awa	y from road and side ditches to represent natural channel section.
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerline	e and Edge of Pavement
Roadway/Deck	Taraba .
Type: Concrete Asphalt Timber Metal Oth Roadway/Deck Width (ft): 27, 5	ner: Tracks
Culvert/Pipe	
Number of Pipes: 2 3 Oil	her
Shape/Type of Pipe: Circular Box Arch E	llipse Irregular Other:
Material: Concrete Corrugated Metal Smooth Me	etal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): 3 He	ight (ft): 3 Length (ft): 85.5
Entrance Type: Projecting Flat Headwall	Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75 50	25 <10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 1
Expansion		3

OTES:			

	Structure Number: pt 111 -116 Road Name: Ryll Tracks
Cross Sections: Upstream of Structure Downstream of Structure	away from road and side ditches to represent natural channel section.
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along center	rline and Edge of Pavement
Roadway/Deck  Type: Concrete Asphalt Timber Metal	Other: Tracks
Roadway/Deck Width (ft): 27.0	
Culvert/Pipe	
Number of Pipes: 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal mooth	Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): 3.5	Height (ft): 3,5 Length (ft): 80.7
Entrance Type: Projecting Flat Headwa	Il Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 25 <10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		3

NOTES:		
2.0 10.0		

Project Number: 2202324,006 Structure Number:	+ 112-115
Date: 12-15-20 Road Name: Tre	c ks
Cross Sections: Upstream of Structure  Downstream of Structure  **Note- Cross sections should be located far enough away from road and side d	itches to represent natural channel section.
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerline and Edge of Pavement	it
Roadway/Deck  Type: Concrete Asphalt Timber Metal Other: Tracks	
Roadway/Deck Width (ft): 27,0	
Culvert/Pipe	
Number of Pipes: 1 2 3 Other:	
Shape/Type of Pipe: Circular Box Arch Ellipse Irregular	Other:
Material: Concrete Corrugated Metal Smooth Metal Corrugated Plas	tic Smooth Plastic Other;
Inside Diameter/Width (ft): 2.5 Height (ft): 2.5	Length (ft): 80.5
Entrance Type: Projecting Flat Headwall Flat Headwall w/ Wi	ngwalls Other:
Culvert Opening Clogged (%): 100 75 50 25 <10	

Entrance (	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		2 6

TES:			

			202328	200	Struct	ure Nun	Ree :	113	- 1/2/	
Date:	12-	5-20			Road	Name:	REE .	Track		-
	]	Downstrea	of Structure am of Struct uld be locate	ure	h away fro	m road an	d side ditch	es to repre	esent natur	ral channel se
-mei er	t Invert El		To be a very cold							
	el Invert E									
	kment Sid		ts (minimun	alone cer	terline and	Edge of I	Pavement			
remau .	inois.	i ince sno	rs (muninan	if mong cen	iteriffic mis	cuge of t	avenien			
Roady	vay/Deck									
	Concrete	Aspha	di Timba	Metal	Other.	Tra	65			
Type.	Contrete	rispini	ut timber	(freedom)	Connect	11.00	-			
Roady	vay/Deck V	Vidth (ft)	2	7.0						
Culver	rt/Pipe									
73.335		0	1		40.00					
Numb	er of Pipes	, ,	2	3	Other:					
Shape	Type of Pi	ре: (	Circular 1	Box Arch	Ellips	e Irregu	ılar Othe	r:		
Mater	ial: Conc	rete Co	orrugated Me	etal Smoo	oth Metal		ted Plastic	Smooth	Plastic	Other:
Inside	Diameter/	Width (fr	1):_ 3		Height	(ft):3		Lei	ngth (ft):	80.5
Entra	nce Type:	(	Projecting	Flat Head	wall Fla	it Headwal	l w/ Wingw	ralls O	ther	
			_	/ -						

Entrance C	ondition (Circle)	Photo Sequence
Contraction		
Straight		Flow
Expansion		3

NOTES:	

Project Number: 2202328.005	Structure Number: >+ 118-119
Date: (2-15-20	Road Name: _ No No
Cross Sections: Upstream of Structure Downstream of Structure **Note- Cross sections should be located far enough	away from road and side ditches to represent natural channel section
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along cent	erline and Edge of Pavement
Roadway/Deck	
Type: Concrete Asphalt Timber Metal	Other: D:
Roadway/Deck Width (ft):	
Culvert/Pipe	
Number of Pipes: (1) 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smoot	th Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): 2,5	Height (ft): 2. 5 Length (ft): 41. 7
Entrance Type: Projecting Flat Headw	vall Flat Headwall w/ Wingwalls Other:
Calvert Opening Closerd (%): 100 75	(50) 35 (10)

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		2

CEC.			
E2:			

Project Number: 2202324.00 Structure Number: 2 120-121  Date: 12-15-20 Road Name: Project Number: 2012
Date: 12-15-20 Road Name: Prive way
Cross Sections: Upstream of Structure  Downstream of Structure  **Note- Cross sections should be located far enough away from road and side ditches to represent natural channel section.
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerline and Edge of Pavement
Roadway/Deck
Type: Concrete Asphalt Timber Metal Other: Dec+  Roadway/Deck Width (ft): 10.5
Culvert/Pipe Number of Pipes: 2 3 Other:
Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:
Material: Concrete Corrugated Molal Smooth Metal Corrugated Plastic Smooth Plastic Other
Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 40-7
Entrance Type: Projecting Flat Headwall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75 50 25 <10

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 1 Flo
Expansion		7

EEC:	
TES:	

Cross Sections: Upstream of Structure  Downstream of Structure  **Note- Cross sections should be located far enough away from road and side ditches to represent natural channel sec  Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerline and Edge of Pavement  Roadway/Deck Type: Concrete (Asphalt) Timber Metal Other: College Sac  Roadway/Deck Width (ft):  Culvert/Pipe Number of Pipes: 1 2 3 Other:  Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:  Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:  Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	Project Number: 22 02 32 \ \ .000		Structure Number: p+ 13/-13Z Road Name: P. s+ Office Ld
Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerline and Edge of Pavement  Roadway/Deck Type: Concrete Asphalt Timber Metal Other: < \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	Cross Sections:	Upstream of Structure Downstream of Structure	h away from road and side ditches to represent natural channel sect
Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerline and Edge of Pavement  Roadway/Deck Type: Concrete Asphalt Timber Metal Other: <	Culvert Invert E	levation	
Road Shots: Three shots (minimum) along centerline and Edge of Pavement  Roadway/Deck  Type: Concrete Asphalt Timber Metal Other: < \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	Section of the sectio		
Roadway/Deck  Type: Concrete Asphalt Timber Metal Other: < \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \			A - A - A - A - A - A - A - A - A - A -
Type: Concrete Asphalt Timber Metal Other: Collective SAC  Roadway/Deck Width (ft):  Culvert/Pipe Number of Pipes: 1 2 3 Other:  Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:  Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:  Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	Road Shots:	Three shots (minimum) along cen	iterline and Edge of Pavement
Type: Concrete Asphalt Timber Metal Other: Collective SAC  Roadway/Deck Width (ft):  Culvert/Pipe Number of Pipes: 1 2 3 Other:  Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:  Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:  Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	Roadway/Deeb		
Roadway/Deck Width (ft):  Culvert/Pipe Number of Pipes: 1 2 3 Other:  Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:  Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:  Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	ROAD WAY/ DECK		Action to the second se
Culvert/Pipe Number of Pipes: 1 2 3 Other:  Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:  Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:  Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	Type: Concret	(Asphali ) Timber Metal	Other: Culte SAC
Culvert/Pipe Number of Pipes: 1 2 3 Other: Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other: Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other: Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	Danders (Dank )	Vitable (DA)	
Number of Pipes: 1 2 3 Other:  Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:  Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:  Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	Roadway/Deck	vidin (it):	
Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:  Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:  Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	Culvert/Pipe		
Shape/Type of Pipe: Circular Box Arch Ellipse Irregular Other:  Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:  Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	Number of Pine	. (1) 2 3	Other
Material: Concrete Corrugated Metal Smooth Metal Corrugated Plastic Smooth Plastic Other:  Inside Diameter/Width (ft): 15 Height (ft): 15 Length (ft): 7	Number of tipe	. 0	Omer:
Inside Diameter/Width (ft):/5	Shape/Type of F	ipe: Circular Box Arch	Ellipse Irregular Other:
Inside Diameter/Width (ft): 15" Height (ft): 15" Length (ft): 7		\	THE RESERVE AND ADDRESS OF THE PARTY OF THE
	Material: (Con	crete Corrugated Metal Smoo	oth Metal Corrugated Plastic Smooth Plastic Other
		Width (ft): 15	Height (ff): 15 Length (ff): 7
Patronica Turias (Pariastin Flat Handwall Flat Handwall w/ Winnesselle Ottom	Inside Diameter		the state of the s
Entrance Type: Projecting Flat Headwall Flat Headwall w/ Wingwalls Other:	Inside Diameter		

Entrance	Condition (Circle)	Photo Sequence
Contraction		
Straight		Flow 5
Expansion		3

NOTES:			

	2202328.000	Structure Number: pt 158-159 Road Name: RxR +calls
Date: (2-15-2) Cross Sections: Upstre		Road Name: KXK TO TO
	stream of Structure	
Mr. 10, 711190		gh away from road and side ditches to represent natural channel section
Culvert Invert Elevation	in .	
Channel Invert Elevati	40,000 - 41,000	
Embankment Side Slop		The total Age Roots Av
Road Shots: Three	shots (minimum) along cen	enterline and Edge of Pavement
Roadway/Deck		
		- V
Type: Concrete As	phalt Timber Metal	Other Track
Roadway/Deck Width	(n): 27.0	
Roadwily/Deck Width	21,0	
Culvert/Pipe		
	1	(61)
Number of Pipes: (	2 3	Other:
Shape/Type of Pipe:	Circular Box Arch	ch Ellipse Irregular Other:
Snape/Type of Pipe:	Circular Box Arch	in Empse irregular Other.
Material: Concrete	Corrugated Metal Smoo	ooth Metal Corrugated Plastic Smooth Plastic Other:
THE COUNTY OF	1	, and the same of
Inside Diameter/Width	(ft): Z.5	Height (ft): 2.5 Length (ft): 86.3
Entrance Type:	(Projecting Flat Heady	dwall Flat Headwall w/ Wingwalls Other;
		^
Calvact Onanina Class	and (9/4) 100 75	50 (25) -10

Entrance Condition (Circle)		Photo Sequence
Contraction		
Straight		Flow 5
Expansion		

ES:		

Project Number: 2202328.00C	Structure Number: p+ 160-161
Date: 12-15-20	Road Name: RR Tracks
Cross Sections: Upstream of Structure Downstream of Structure **Note- Cross sections should be located far end	ough away from road and side ditches to represent natural channel section
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along	centerline and Edge of Pavement
Roadway/Deck	
Type: Concrete Asphalt Timber Meta	al Other Tracks
Roadway/Deck Width (ft): 27.0	
Culvert/Pipe	
Number of Pipes: (1) 2 3	Other.
Shape/Type of Pipe: Circular Box A	arch Ellipse Irregular Other:
Material: Concrete Cornugated Metal (Sr	mooth Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): _ 2. 5	Height (ft): 2.5 Length (ft): 64.5
Entrance Type: Projecting Nat He	eadwall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 (25) <10

Entrance Condition (Circle)		Photo Sequence
Contraction		
Straight		Flow 5
Expansion		3

NOTES:	
HOTES.	

	Road Name: Now 2
Cross Sections: Upstream of Structure  Downstream of Structure	non-from and and side dischar to represent natural channel section
**Note- Cross sections should be located far enough av	way from road and side ditches to represent natural channel section.
Culvert Invert Elevation	
Channel Invert Elevation at Culvert	
Embankment Side Slope Rate Road Shots: Three shots (minimum) along centerl	ing and Edge of Payament
Road Shots: Three shots (minimum) along cemen	the and dage of ravenien
Roadway/Deck	
Type: Concrete Asphalt Timber Metal C	Other: Dir+
Roadway/Deck Width (ft):	
Culvert/Pipe	
Number of Pipes: 1 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smooth	Metal Corrugated Plastic Smooth Plastic Other:
Inside Diameter/Width (ft): 1.5	feight (ft): /. 5 Length (ft): 16,2
Entrance Type: Projecting Flat Headwall	Flat Headwall w/ Wingwalls Qther:
Culvert Opening Clogged (%): 100 (75)	0 25 <10

Entrance Condition (Circle)		Photo Sequence
Contraction		
Straight		Flow 5
Expansion		

inc.			
14 16.7			

Project Number: 2202329.00C	Structure Number: pt 324-525 Road Name: RsQ Tracks
Date: 12-15-20	Road Name: Rik Tracks
Cross Sections: Upstream of Structure  Downstream of Structure  **Note- Cross sections should be located far enough	away from road and side ditches to represent natural channel section.
Culvert Invert Elevation Channel Invert Elevation at Culvert Embankment Side Slope Rate Road Shots: Three shots (minimum) along center	erline and Edge of Pavement
Roadway/Deck	
Type: Concrete Asphalt Timber Metal	Other: Tracks
Roadway/Deck Width (ft): 27.0	
Culvert/Pipe	
Number of Pipes: 1 2 3	Other:
Shape/Type of Pipe: Circular Box Arch	Ellipse Irregular Other:
Material: Concrete Corrugated Metal Smoot	th Metal Corrugated Plastic Smooth Plastic Other
Inside Diameter/Width (ft): 4.5	Height (ft): 4.5 Length (ft): 65.2
Entrance Type: Projecting Nat Headw	rall Flat Headwall w/ Wingwalls Other:
Culvert Opening Clogged (%): 100 75	50 25 <10

Entrance Condition (Circle)		Photo Sequence
Contraction		
Straight		Flow 5
Expansion		3

	1.1		
ES:			
April 1			























































































































































































































































































## **APPENDIX - CAMP ON HOPE CANAL**

1600-FT SOUTH OF I-10

OWNERS: JEREMIAH JOHNSON & SHAWN GRADY

PARCEL NO. 9660442737

12/4/2020 Print

# St. John Parish Assessor 2020 Assessment Listing

#### Parcel#

9660442737

View on Map (http://atlas.geoportalmaps.com/stjohn\_public/q/Parcel?PARCEL\_ID=9660442737)

Primary Owner

JOHNSON, JEREMIAH &

**Mailing Address** 

SHAUN GRADY 97 BIG TREET BLVD GARYVILLE LA 70051-0000

Ward

WARD 6

Type

Real Estate

Legal

LOT 1.005 ACRES

**Physical Address** 

#### **Parcel Items**

Property Class	Assessed Value	Market Value	Units	Homestead
COUNTRY LOT	700	7,000	1.00	0
TOTAL	700	7,000	1.00	0

#### **Deeds**

Deed#	Туре	Date	Amount	Book	Page
354986	01-Regular Transfer	11/2/2017	12,000		
353970	04-Succession	9/7/2017	0		

### **Ownership History**

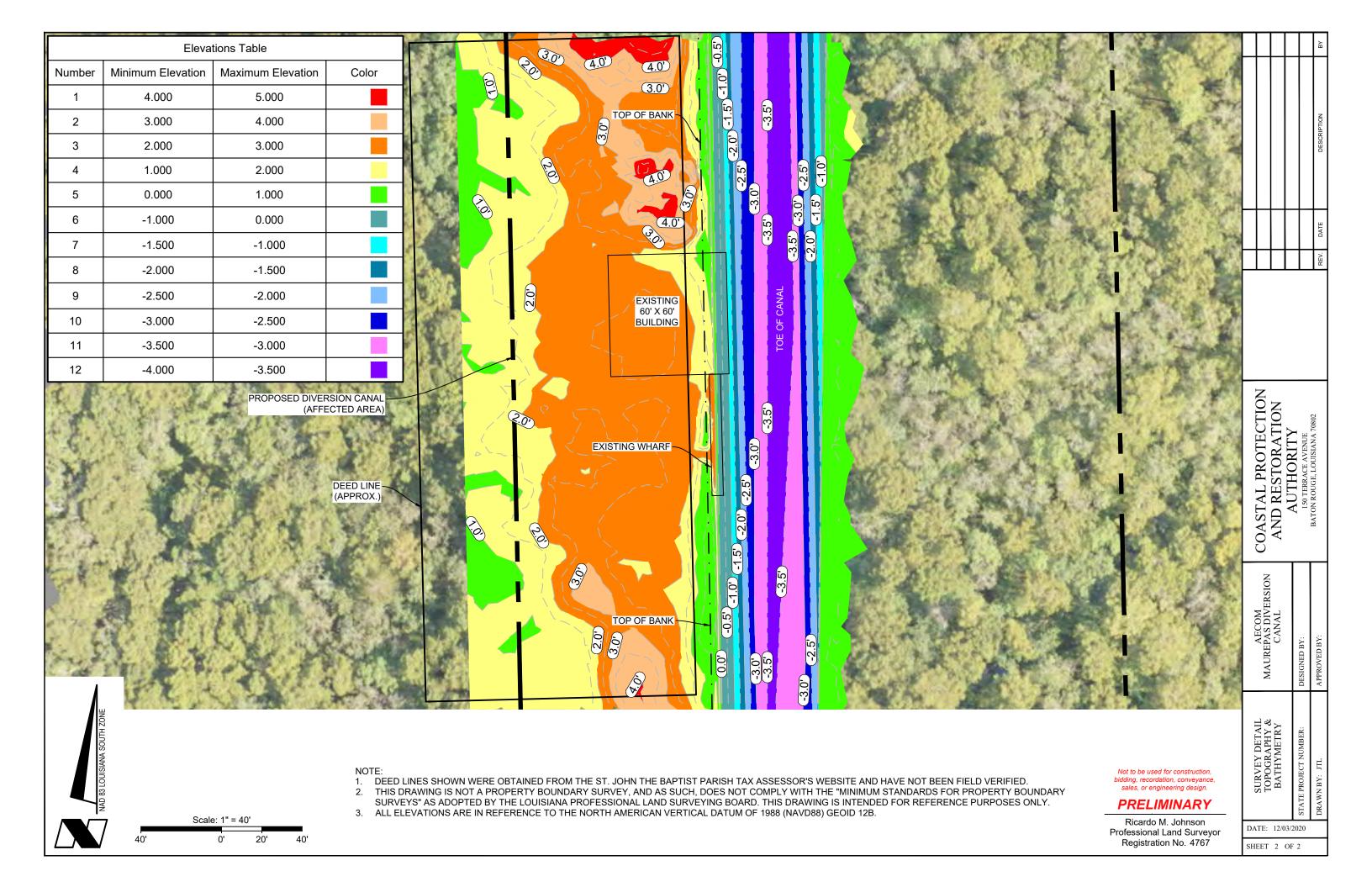
Homestead?	Name	Primary?	% Ownership	% Tax	From	То	Address
NO	JOHNSON, JEREMIAH &	YES	100.0000	100.0000	11/2/2017		
NO	BOGEN, LACIE LIN MICHEL	YES	100.0000	100.0000	9/7/2017	11/2/2017	
NO	MICHEL, FLOYD JR & LINDA	YES	100.0000	100.0000	1/17/2012	9/7/2017	

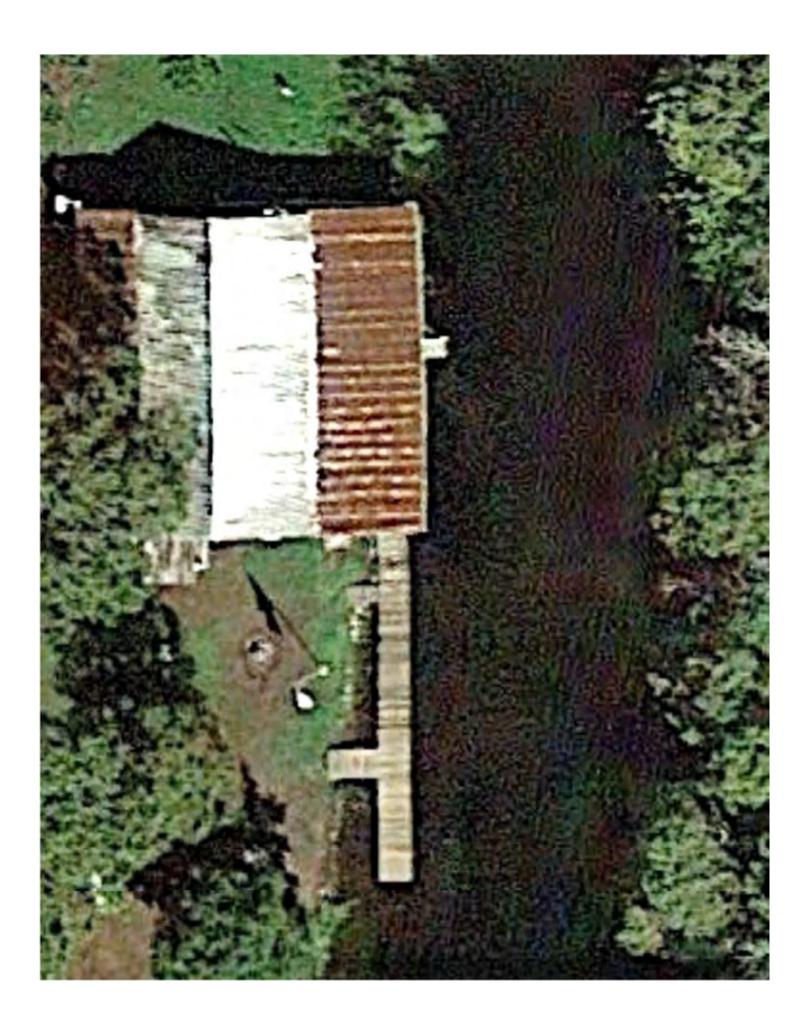
#### **PARISH**

Millage	Mills	Taxpayer Tax	Homestead Tax
PARISH WIDE 89.11	89.1100	62.38	0.00
LAW ENFORCE 16.99	16.9900	11.89	0.00
P LEVY 3.53	3.5300	2.47	0.00
LAW ENFORCE 16.00	16.0000	11.20	0.00
TOTALS	125.6300	87.94	0.00









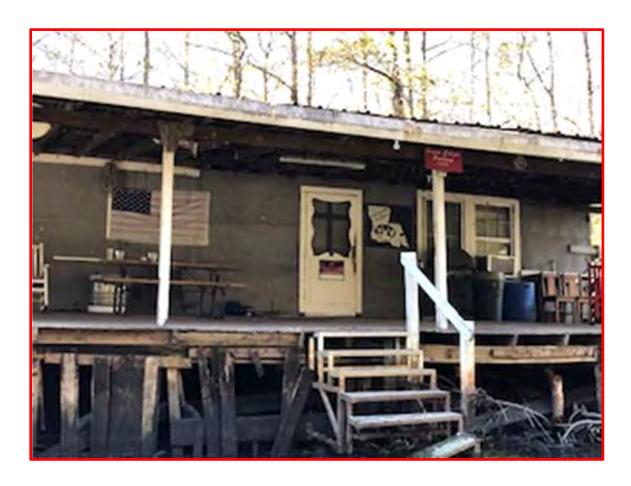
















## STATE OF LOUISIANA COASTAL PROTECTION AND RESTORATION AUTHORITY

## RIVER REINTRODUCTION INTO MAUREPAS SWAMP WEST SHORE OF LAKE PONTCHARTRAIN FLOOD PROTECTION STATE PROJECT No. PO-0029

## **Geotechnical Data Collection Plan**

for



And



AECOM 1515 Poydras Street New Orleans, LA 70806

July 16, 2020

Rev.	Date	Description	Checked	Approved						
			Eustis							
0	7/8/2020 Initial Draft Pending Adjustments from Site Visit	Initial Draft Pending Adjustments from Site Visit	Engineering							
			(JMW)							
1			Eustis							
	7/16/2020	Revised Plan Based on Agency Review Comments	Engineering	gineering						
			(JMW/SGW/GPS)							
	Revisions after Rev. O are denoted by a vertical line in the right-hand margin against the revised text									

## **Acronyms and Abbreviations**

CPRA Coastal Protection and Restoration Authority

CPTs Cone Penetration Tests

CUP Coastal Use Permit

DOTD Louisiana Department of Transportation and Development

EL Elevation

GPS Global Positioning System

HSDRRSDG Hurricane and Storm Damage Risk Reduction System Design

Guidelines

I.D. Inside Diameter

in. Inch

JSA Job Safety Analysis

LFPDG Louisiana Flood Protection Design Guidelines

LONO Letter of No Objection

MRL Mississippi River Levee

O.D. Outside Diameter

QA/QC Quality Assurance/Quality Control

ROE Right of Entry

SPTs Standard Penetration Tests

USACE U.S. Army Corps of Engineers

WSLP West Shore of Lake Pontchartrain

## **Table of Contents**

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## **Appendix**

Appendix A – Existing Data Locations

Appendix B – Proposed Exploration Locations

## 1 PURPOSE

This Geotechnical Data Collection Plan (Revision 1) presents the policy and specific actions that will be implemented for the proposed field exploration associated with the Soil Borings and Cone Penetration Tests (CPTs) for the Maurepas Diversion and West Shore of Lake Pontchartrain (WSLP) Protection project. The goal of the exploration program is to provide additional geotechnical data to support our design. This document presents plans and procedures that Eustis Engineering L.L.C. will implement for access, quality, safety, and consistency of work to be performed by field crews as well as any subcontractors required for the work. This plan will address the means and methods to obtain the samples, perform field testing, and perform laboratory testing. This plan will address requirements for clearing of vegetation and impacts to properties and wetlands where the field crews need access.

## 2 OVERVIEW

Exploration methods will include 5-in. diameter fixed piston Shelby tube sampling in addition to in-situ testing using CPTs. The scope of the field exploration is itemized below and includes the exploration types, quantities, and depths. The locations of the existing borings and CPTs provided by CPRA and AECOM are provided in Appendix A. Based on the review of this existing data, the design team identified geotechnical data gaps that are required to be filled to meet technical design requirements in addition to the requirements of the USACE's Hurricane and Storm Damage Risk Reduction System Design Guidelines (HSDRRSDG) and CPRA's Louisiana Flood Protection Design Guidelines (LFPDG). The proposed additional borings and CPTs are shown in Appendix B. The locations, types and depths of the exploration points have been carefully considered to meet technical design requirements and various agency design guidelines. The scope of the field exploration is itemized in a summary table included in Appendix B. We note the locations we present in Appendix B have been agreed upon verbally through discussion between Eustis Engineering, AECOM, and CPRA. The proposed plan includes drilling 5-in. diameter borings at approximate 1000-ft intervals along the WSLP alignment to support the design of the WSLP flood protection features with intermediate CPTs. Boring and CPT depths along the levee alignment will be 50 to 60 feet below the existing grade. At structure locations the borings and CPTs will be extended to depths of up to 125 feet. Currently, structural project features include gates, and/or T-wall monoliths at River Road, the KCS railroad, the CN railroad and at the tie-in near Hope Canal (i.e., Four Locations). This Revision of our Geotechnical Plan is based around comments received from CPRA to reduce our exploration plan. We present the revisions herein.

# 3 PERMITS AND COMMUNICATION WITH LANDOWNERS

In order to perform the borings and CPTs, a permit is required from the Pontchartrain Levee District (PLD), including Letters of No Objection from the Completed Works Branch at the USACE and CPRA. Additional permits include a Coastal Use Permit (CUP) and Section 10/404 permit from the USACE. If a boring in moved into the right of way of U.S. Highway 61, a permit from Louisiana Department of Transportation and Development (DOTD) will also be required, but at this time we do not anticipate this being necessary for the project. We understand right-of-entry (ROE) and permits for this site will be obtained by CPRA

and/or the Pontchartrain Levee District. We assume all other permits, ROE documents, and points of contact will be provided with the notice to proceed (NTP) in time to begin operations on 16 July 2020. Recommendations and restrictions mandated by permitting will be added to this execution plan when they become available. In addition, we will contact LA One Call for location of underground utilities prior to mobilizing to the site to perform the borings/CPTs.

## 4 ACCESS AND LOCATING EXPLORATION POINTS

A licensed surveyor will need to mark the levee alignment to facilitate the clearing operations. We understand AECOM retained C.H. Fenstermaker & Associates, L.L.C. to perform these surveying operations and provide the limits in conjunction with our clearing efforts. After clearing, a surveyor should also locate and stake all boring and CPT locations. Alternatively, the boring and cone locations can be located using a handheld GPS unit and the proposed coordinates. In the event the boring is offset due to access, safety, or other considerations, we will coordinate approval of the adjustment with AECOM and CPRA. After the borings and CPTs are completed, we will leave a labeled stake at the exploration location for a professional land surveyor to obtain latitude, longitude, and ground surface elevation of the "asdrilled" boring and CPT locations.

In order to gain access to the exploration locations south of the KCS railroad, we have assumed a bulldozer and/or excavator will be used to clear access paths (approximately 20'-25' wide) to provide access to boring/CPT locations. Locations cleared by dozer may be accessed by our track rig. For locations north of the KCS railroad, we assume Marsh equipment such as a Marsh Excavator will be necessary to clear an approximate 50-ft wide access path and complete our field exploration. We intend to use existing clearings/pathways/roads to access the cleared path and boring/CPT locations whenever possible. Potential Access routes are described in Section 6 of this plan. We may elect to use a truck rig when site access allows this alternative such as locations within the Marathon facility.

Tress will be cut at the existing ground surface or waterline. Trees will be felled in place and not removed, only pushed aside. Stump removal to the mudline or ground surface may involve some chipping operations. Permit restrictions or other clearing requirements may require adjustments to our plans. We will not haul away vegetation cleared at the site.

We anticipate having four work areas and sequencing our work in these areas in the following order:

- Work Area 1: Between River Road (HWY-44) and Canadian National Railroad (CN Rail),
- 2. Work Area 2: Between CN Rail and Kansas City Southern Railroad,
- 3. Work Area 3: Between the North side of Airline Highway and the northern extents of the project; and
- 4. Work Area 4: Between Kansas City Southern Railroad and south side of Airline Highway (US-61).

Based on discussions had between Eustis Engineering, AECOM, and CPRA, the generalized approach for clearing operations is to begin at River Road and clear a path generally along the future alignment centerline with a track bulldozer until reaching the CN Railroad (Work Area 1). This will provide access to SB-01, SCPT-02, SCPT-03, SB-02, and SB-03. The clearing equipment will backtrack to River Road and be moved to access project Station 1047+00 via Post Office Road. From there the track equipment will clear

a path moving north until reaching the Kansas City Southern Railroad (Work Area 2). This effort will provide access to SCPT-07, SB-05, SB-06, SB-07, SCPT-10, SB-08, SB-09, and SCPT-11. We will backtrack our track equipment to exit via Post Office Road as this allows us to avoid passing heavy equipment overtop known pipeline right of ways, or to require a new railroad crossing.

At this stage, our operations will transition to a cargo buggy mounted excavator system for clearing of Work Areas 3 and 4. We propose using an existing cleared area on the northside of US-61 for use with setting up geotechnical equipment associated with work adjacent to Hope Canal. We will begin north of US-61 by floating equipment north along Hope Canal and then begin clearing for access to SB-13 and then clearing an access path south along the project centerline (towards US-61). This will allow for clearing and access to SB-11, SB-12, SB-13, SCPT-13 and SCPT-14. We will then move all equipment to LA-54 on the south side of US-61 to gain access for clearing operations north of the Kansas City Railroad and to provide access to SCPT-12. This approach and sequence provide the least number of "local moves" of our equipment.

The sequence of work for our exploration plan is to generally follow the clearing plan in which we begin our work in Work Area 1, proceed to Work Area 2, then proceed to Work Area 3, and finish at Work Area 4. All track mounted equipment will be delivered to Work Areas 1 and 2 via a haul truck pulling a lowboy for the project. Notes on depth, clearing equipment, exploration equipment, and ground conditions are provided in the following table.

### PROPOSED BORING AND CONE EQUIPMENT AND CLEARING SUMMARY

Boring Location	Depth	Exploration Equipment	Clearing Equipment	Ground Conditions
SB-01	125	Track	Dozer	Land
SB-02	50	Track	Dozer	Land
SB-03	125	Track	Dozer	Land
SB-04	50	Track	None	Land
SB-05	50	Track Dozei		Land
SB-06	50	Track	Dozer	Land
SB-07	50	Track	Dozer	Land
SB-08	50	Track	Dozer	Land
SB-09	125	Track	Dozer	Land
<del>SB-10</del>	<del>50</del>	M. Buggy	Cargo Ex	
SB-11	125	M. Buggy	Cargo Exc.	Swamp
SB-12	50	M. Buggy	Cargo Exc.	Swamp
SB-13	50	M. Buggy	Cargo Exc.	Swamp

Cone Location	Depth	Exploration Clearing Equipment Equipment		
SCPT-01	<del>125</del>	Track	None	_
SCPT-02	125	Track	Dozer	Land
SCPT-03	50	Track	Dozer	Land
SCPT-04	<del>50</del>	<del>Track</del>	<del>Dozer</del>	
SCPT-05	<del>100</del>	<del>Track</del>	None	
SCPT-06	<del>50</del>	<del>Track</del>	None	
SCPT-07	50	Track	Dozer	Land
SCPT-08	<del>50</del>	Track Dozer		
SCPT-09	<del>50</del>	<del>Track</del>	<del>Track</del> <del>Dozer</del>	
SCPT-10	50	Track	Dozer	Land
SCPT-11	100	Track	Dozer	Land
SCPT-12	50	M. Buggy	Cargo Ex	Swamp
SCPT-13	50	M. Buggy	Cargo Ex	Swamp
SCPT-14	50	M. Buggy	Cargo Ex	Swamp

Note: Strikethroughs for line items in above tables have been discussed between all parties.

## **5 EQUIPMENT**

Eustis Engineering will mobilize its equipment from its Metairie, Louisiana office to execute and support the scope of work. The following equipment will be utilized for borings on land:

- CME-850 Track Mounted Drill Rigs
- 15-Ton Vertek Cone Penetrometer Rigs
- D5 Bulldozer or equivalent excavator
- Bobcat Skid Steer
- Four-Wheel Drive Utility Mules
- Miscellaneous Support Trucks and Trailers

For borings over soft, marsh deposits Eustis Engineering will employ the services of a marine equipment contractor to provide the following additional equipment:

- Marsh Excavators for Clearing
- Cargo Buggies or Swamp Buggies for Drilling and Cone Operations
- Airboats/Support Boats

## **6 OVERALL SCHEDULE & SEQUENCE**

We estimate approximately 5 weeks of clearing operations to provide access to the exploration locations along the entire alignment based on the use of two excavators in the swamp areas, one bulldozer in land-based location, and windrowing of trees for the alignment. Mobilization of equipment will be based on availability but may generally be assumed to occur within two weeks of authorization, surveying, and/or permits, as applicable. Our clearing operations assumes no time necessary for hauling of any cleared trees/vegetation. The field operations for the soil borings are estimated to require 44 crew days, and CPT operations will require 5 crew days. This estimate does not include mobilization/demobilization, local moves, or standby time due to weather or other delays as well as holidays. Our crews will generally operate on five or six-day work weeks. We anticipate finalizing our sequence of work upon receiving all permitting and landowner agreements.

## 7 CREW DAILY MOBILIZATION AND DEMOBILIZATION

Eustis Engineering will mobilize its crew daily from its Metairie, Louisiana office/geotechnical laboratory in various support trucks with trailers. Once at a given staging area, the crew will access exploration locations on site using a four-wheel drive utility mule or support boat. Soil samples will be transported vertically in specialized racks for 5-in. diameter undisturbed soil samples and daily from the site to our geotechnical laboratory.

## 8 FIVE-IN. DIAMETER SHELBY TUBE SAMPLING

Five-inch undisturbed samples will be obtained with a 5-in. outside diameter, thin-walled, Shelby tube sampler, continuously pushed with a fixed-piston type sampler (Hvorslev fixed-piston sampler or equivalent) for a penetration of 48 inches. The overall length of the sampling tube is 54 inches. Undisturbed samples are to be taken in accordance with ASTM D1587, "Thin-Walled Tube Sampling of Soils."

Five-inch diameter borings will be obtained for all structures and levee features. This sampling minimizes disturbance and will be supplemented by cone penetrometer tests and existing geotechnical data made available by AECOM and CPRA. Samples will be collected at 4 foot continuous intervals for the duration of the soil boring.

The sampling tube will be advanced in a single-motion push, if possible. The piston rod (inner rod) will be firmly fixed while the tube is advanced. A good mechanical connection, with adequate shear and moment capacity, will be maintained between the drill rig and the outer rod of the sampling head.

Proper mechanical operation of the sampling head will be ensured prior to obtaining a sample. The leather piston rings (seals or packing cups) will be in good condition. The vacuum release system will be functional and the sampling head will be thoroughly cleaned after obtaining a sample.

The piston will be firmly fixed at the bottom of the tube while the tube is being advanced to the next sampling depth by locking the piston rod (inner rod).

The vacuum release mechanism of the sampling head will be activated prior to removing a tube from the sampling head fixture after sampling.

Tubes will be reused if thoroughly cleaned, but dented sampling tubes will be discarded.

Bentonite-based or other approved drilling mud will be used throughout the drilling and sampling process to improve sample recovery and to minimize sample disturbance. The unit weight of the drilling mud will be controlled. The drilling fluid will have a density that maintains a compressive stress at the bottom of

#### Rev 1

the borehole, to offset the reduction in stress from the excavated soil, and the density of the drilling fluid will be great enough to apply a horizontal stress within the hole to prevent the hole from collapsing.

A 5-7/8" diameter three-winged baffled bit will be utilized during advancement of the boring at the end of the rod string and will divert the bentonite slurry back up the borehole to limit sample disturbance for the next sampling event. Bottom discharge bits will be prohibited.

The sampler for SM- and SP- type soils will be a standard Split Spoon sampler (1½-inch I.D., 2-inch O.D.).

Excessive rotation speeds of the cutting tool during drilling will be avoided. A rotation rate of 70 to 80 rpm is appropriate for area soils.

Once samples have been removed from the bore holes, they will be immediately be sealed within the sampling tube prior to shipment to the laboratory for extrusion, classification, and testing.

The interior ends of the Shelby tube samples will be trimmed, with a tool constructed for that purpose, to a minimum 1.5-inch recess. The tube ends will then be secured with expandable packers. The wing nut of the packer will be tightened with a tool designed for that purpose, so that the O-ring of the packer expands against the interior of the tube. The end will be tested with a pocket penetrometer as a rough indication of consistency. The tube will then be sealed with plastic end caps, and wrapped with three wraps of tape (electrical tape or duct tape) to prevent loss of moisture.

Sample tubes will be shipped to the testing laboratory in an upright position (e.g., in the same orientation as removed from the borehole) and secured and/or padded for protection against jarring or vibration. Rough handling will be avoided. Samples will be protected from extreme temperatures and exposure to moisture by providing shade or cover where necessary to protect the sample tubes. Due to the weight and bulk of the 54-in. tube sample, movement by manual means requires two people.



Figure 1: Typical land-based set up during drilling and sampling (track-mounted rig)



Figure 2. Marsh buggy with drill rig



Figure 3: Five-in. diameter Shelby tube vertical transportation rack

## 9 CONE PENETRATION TEST OPERATIONS

For the CPT, we will utilize our 2007 Vertek track mounted CPT rig. Two crewmen will be on the CPT rig which includes and operator and a helper. CPT test will be performed with an electronic piezocone and electronic readout. The rig will be positioned over the location of the sounding and leveling jacks are lowered to raise the machines mass off the rigs suspension system. The hydraulic rams of the penetrometer thrust system are set to as near vertical as possible by adjusting the leveling jacks. Once the rig is set level the following will be performed.

Soundings will be in accordance with ASTM D5778. We will use our 2.5 and 5.0-ton tension and subtraction cones depending on observed sensitivity and application. From our experience, these cone capacity limits offer the greatest accuracy and sensitivity to lower strength Holocene soils. CPTs will be correlated to undisturbed borings in some locations and will also be used to correlate existing geotechnical data made available by AECOM and CPRA.

The CPT probe will be saturated with silicon fluid in the field prior to each use. Soundings will be done in the following sequence:

Power up the penetrometer tip and data acquisition system according to the manufacturers recommendations, typically 15 to 30 minutes prior to use.

Obtain initial zero readings for the cone in an unloaded condition at a temperature as close to ground conditions as possible.

Record pertinent information such as project number, date, time, cone id, and starting point of each test.

The cone will be advanced into the soil at a rate of 2 centimeters per second plus or minus 0.5 centimeters. The depth, tip resistance, sleeve friction, pore pressure, and probe inclination are recorded at 5-centimeter intervals.

During the progress of the sounding, tip pressure, sleeve friction, and pore pressure will be continuously monitored for signs of proper operation. Other indicators such as rod rebound and probe inclination will be monitored to ensure damage will not occur if highly resistant layers or obstructions are encountered.

At the end of a sounding the penetrometer will be extracted. A final set of zero readings of the unloaded cone will be obtained and checked against the initial zero readings. Initial and final baselines will be documented.

The cone assembly will be inspected after each sounding for damage. Damaged components will be replaced as needed. In general, we expect CPT soundings to reach planned depths. If CPT soundings do not reach planned depths, alternatives will be discussed.

### Rev 1



Figure 4: CPT Operations



Figure 5: CPT rig mounted to buggy

## 10 WATER MEASUREMENTS

During the course of a given boring, ground water measurements will be made for borings on land. The initial depth and time at which the water table is encountered will be noted on the field log. The crew will let the borehole sit for approximately 15 minutes prior to recording the water level. Crews will measure water level inside borehole at the start of each day before the commencement of drilling, noting depth to the nearest inch, date, and time. Any sudden changes in water level during drilling, such as loss of drilling mud, will be noted. For boings completed in inundated areas, water depth measurements to the mudline will be made.

## 11 FIELD LOGS

Field logs will be filled out with a scaled log for each 50-ft of soil boring as well as an additional boring information sheet to provide supplemental information. Field Logs will include the following:

- Depth, interval, sample recovery and sample number. All samples will be numbered according to the designated boring number and consecutive numbering sequence for each sample.
- Type of SPT hammer (height, weight, type of drop hammer).
- Number of blows for each 6-in increment of the SPT.
- Visual description, estimate of dilatancy, and preliminary classification of the soil encountered using the Unified Soil Classification System:
- Pocket Penetrometer readings for applicable samples.
- Individual sample collection date and time.
- Depth of groundwater first encountered.
- Daily water level readings.
- Any unusual drilling conditions encountered (e.g. loss of drilling fluids, occurrence of gas, heave of the bottom of the hole).
- Rig, driller, logger
- Activities report of crew personnel, drilling progress, and operational observations.

## 12 GROUTING PROCEDURES

Upon completion of drilling, all bore holes and CPT soundings will be tremie-backfilled to the ground surface in accordance with the CPRA, USACE, and permit requirements. The grouting procedures are as follows:

A grout slurry with minimum density of 13.3 pounds per gallon (lbs/gal) will be batched. The design grout mix consists of: a) one sack (94 lbs) of Portland Type I/II Cement; b) ten gallons of water; and c) 7 pounds

#### Rev 1

of bentonite. The batched grout will be weighed and recorded on a backfill record. The maximum grout slurry density is 14.1 lbs/gal.

A tremie pipe (approximately 1-in. diameter) or drill rod will be placed to the bottom of the bore hole or CPT sounding.

The grout slurry will then be pumped using a positive displacement or piston pump to the bottom of the bore hole through the tremie pipe.

Grout placement will be continued until the return at the surface is within ±0.2 lbs/gal of the initial grout density. The return grout density, along with the backfill quantities, will be recorded on the backfill record.

## **13 QUALITY CONTROL AND SAFETY**

Prior to mobilization, the scope of field work to be accomplished along with safety will be reviewed in a project meeting held with the project manager, field supervisor, drill crew and subcontractors. This meeting will familiarize the field crews with the required work and safety requirements of the project. Quality Control and safety go hand in hand and have a direct relationship to the success of this scope of work. The Quality Control Plan is included in Appendix C – Quality Control Plan for Soil Borings, Laboratory Testing, and Cone Penetration Tests, attached to this document. A separate safety plan has also been submitted.

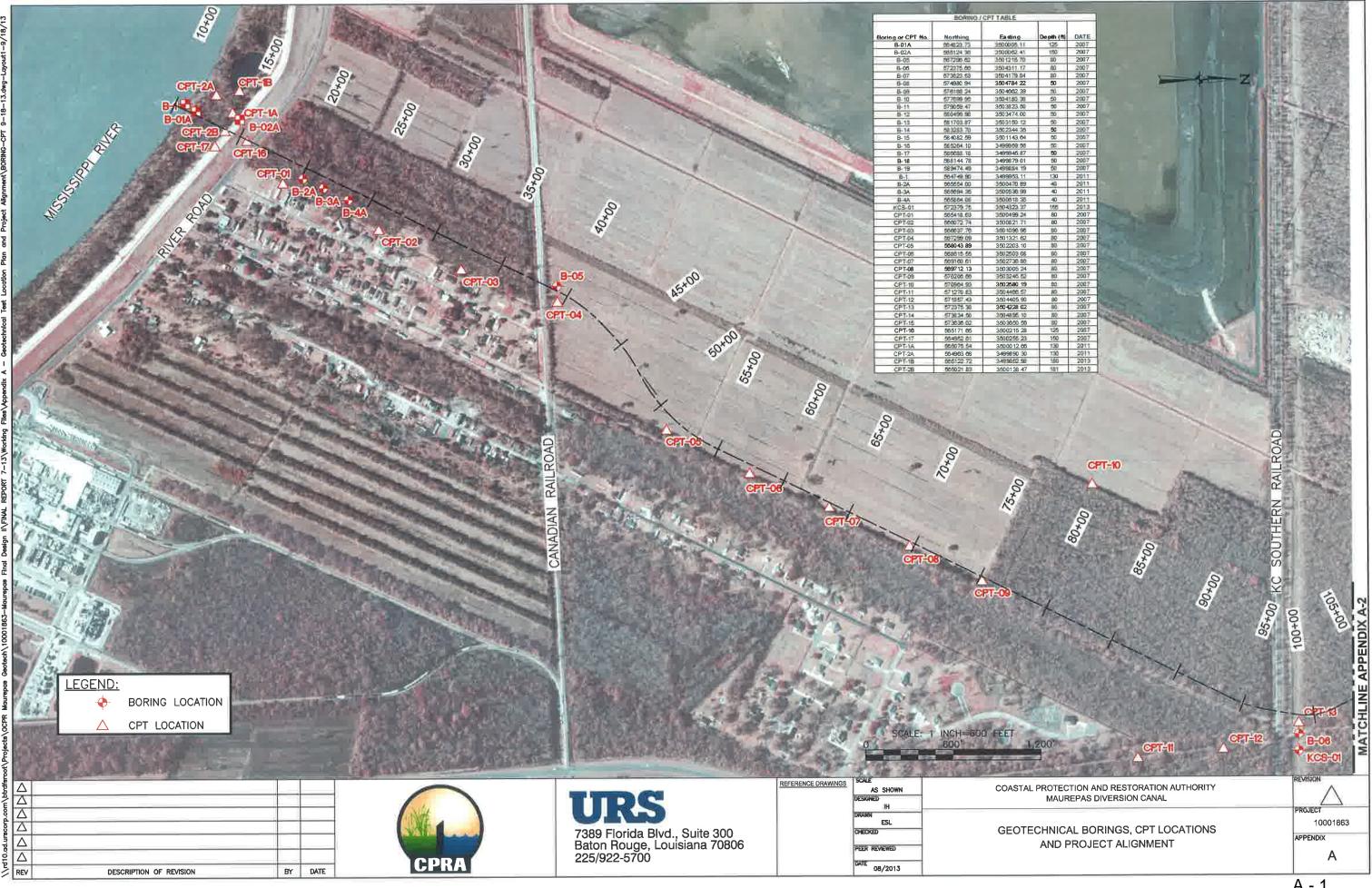
We will have an onsite supervisor on the barge and land that will be coordinating the scope of work with the drill crew. He or she will oversee quality and safety during the course of this scope of work. He or she will provide daily updates to the project manager pertaining to progress, and quality control supervision of the crews.

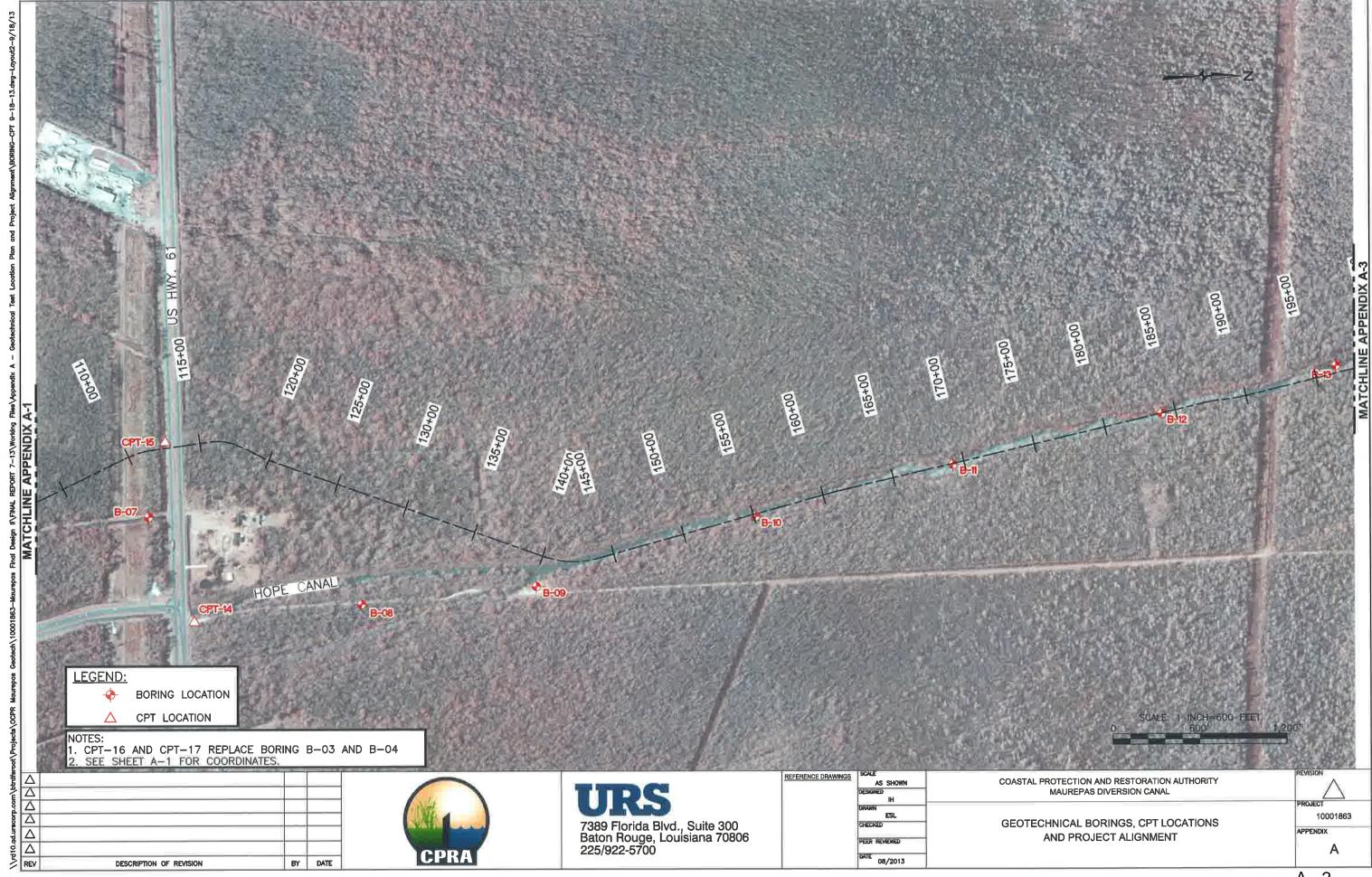
In addition, the field supervisor will oversee the overall safety of the crews. He will be monitoring any changed conditions as the work progresses.

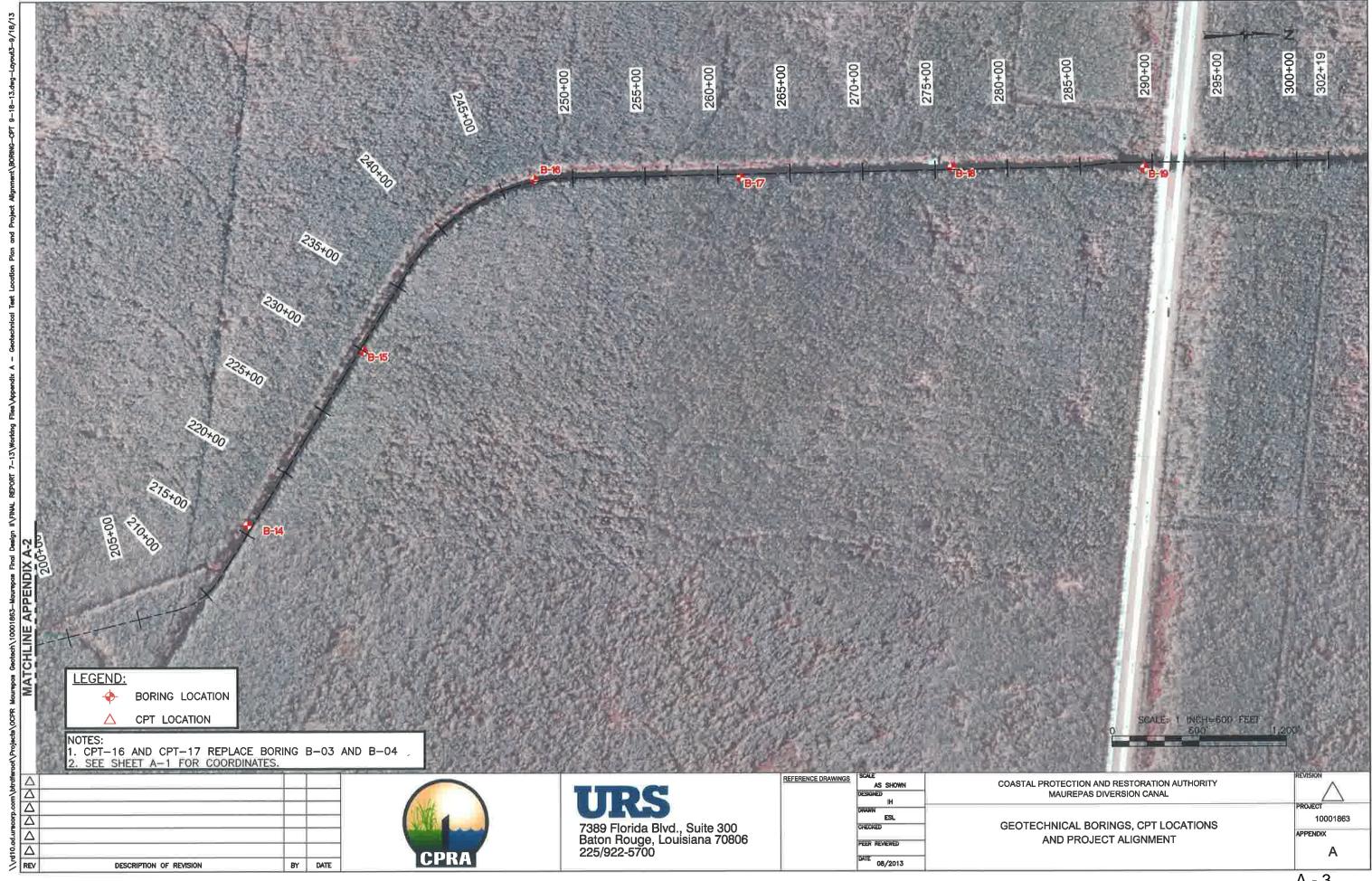
- A pre-job hazard analysis will be completed and reviewed with the crew.
- Daily tool box safety meeting will be given onsite
- A daily review of the Job Safety Analysis (JSA) will be completed by the entire crew. The JSA will
  also be reviewed and modified should a changed condition occur
- Crews will be advised of the locations of fire extinguishers, shelter in place points, first-aid kits and all contacts in the event of an emergency
- PPE to be worn at all times will include hard hats, safety glasses, gloves, hearing protection, and steel toe shoes.

## APPENDIX A

**EXISTING DATA LOCATIONS** 

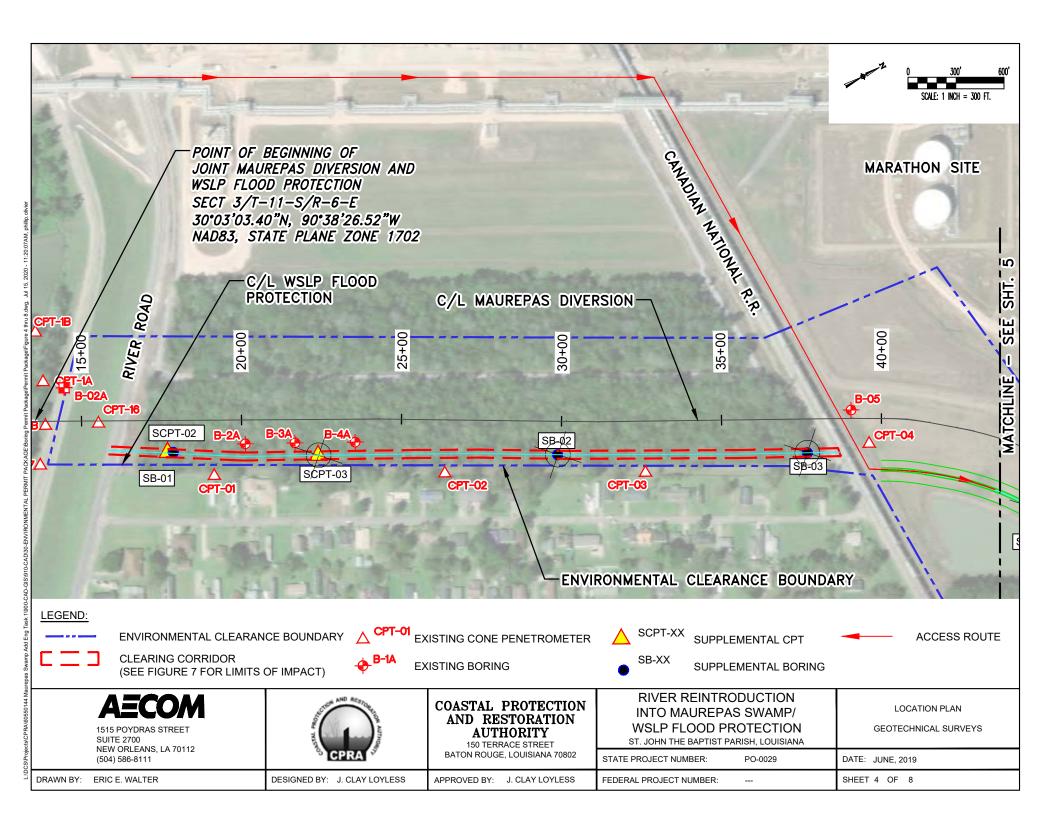


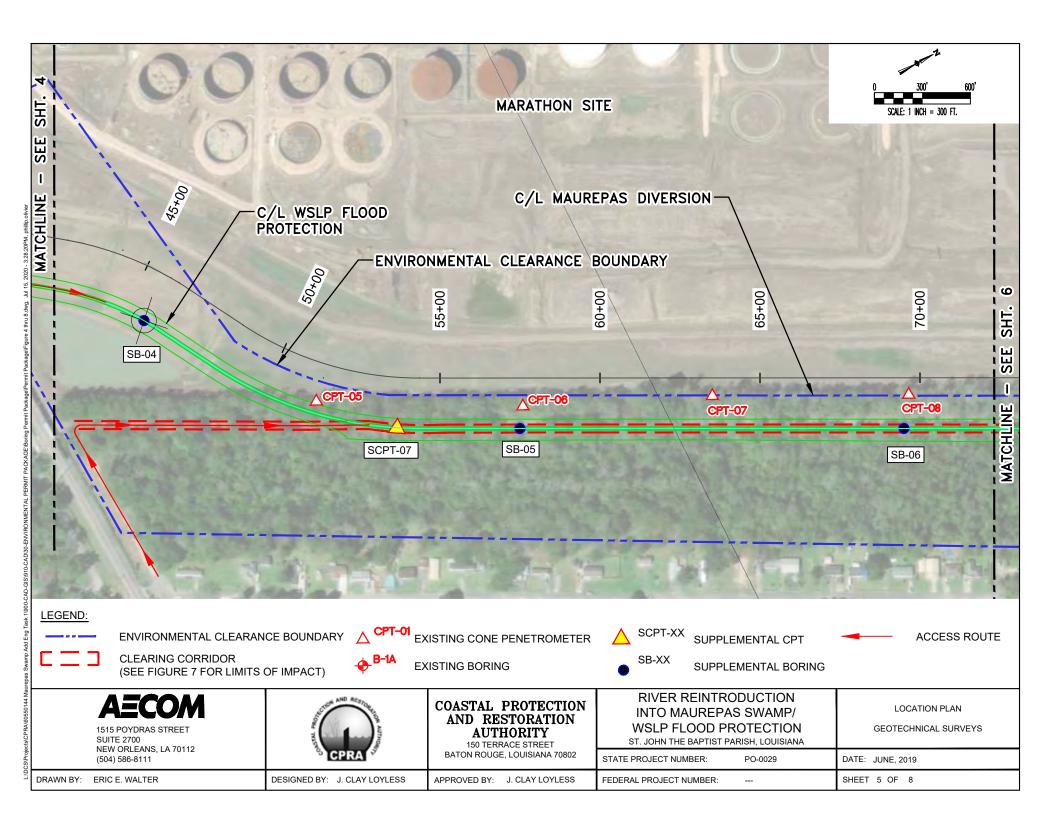


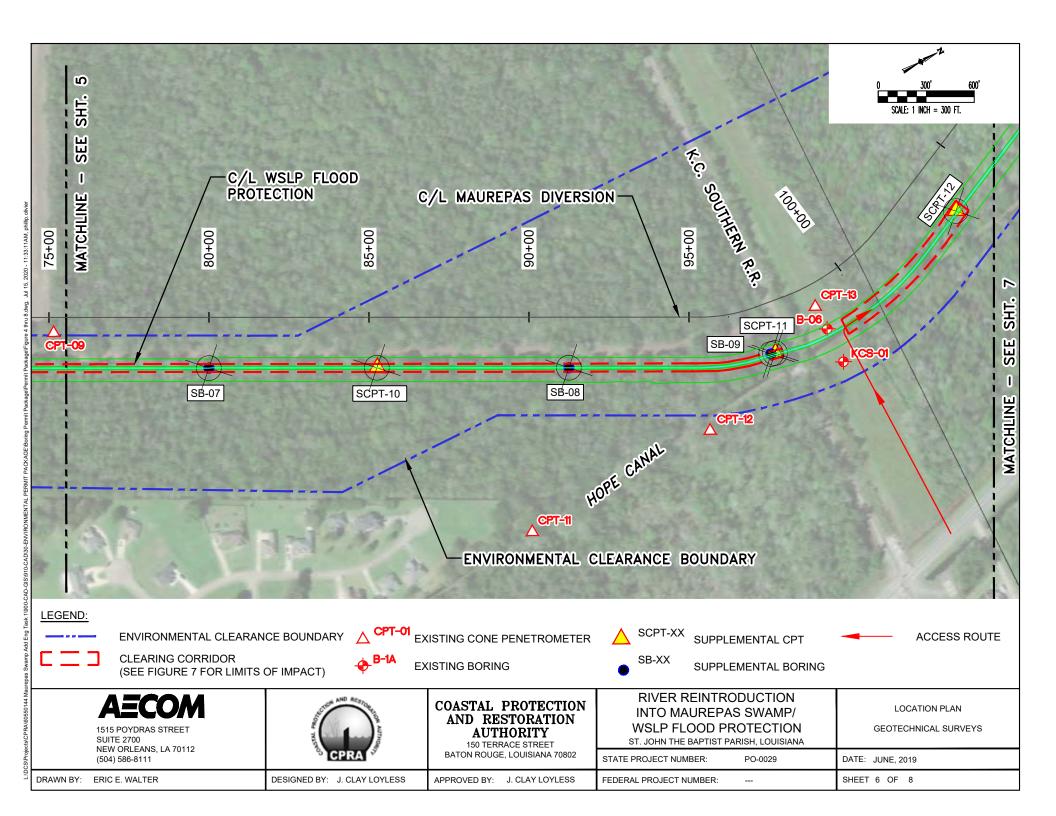


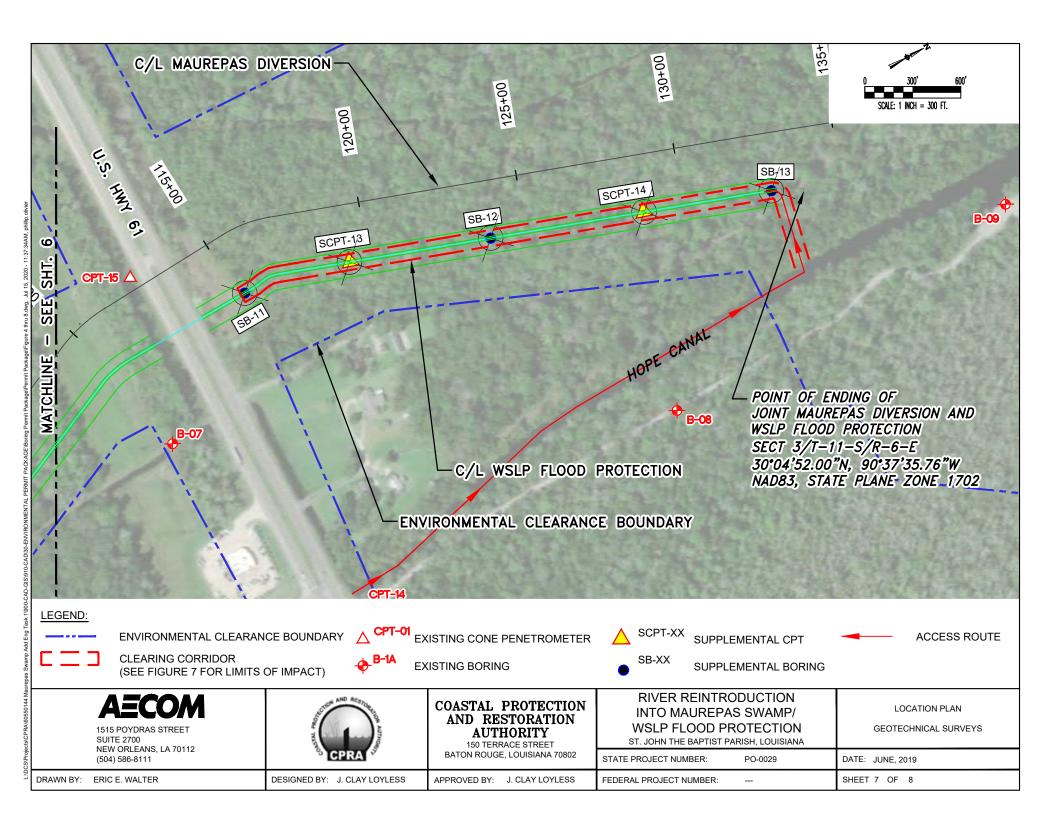
## APPENDIX B

PROPOSED EXPLORATION LOCATIONS







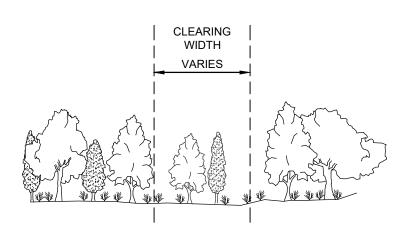


SUPPLEMENTAL CPT SCHEDULE (SCPT-XX)								
ITEM No.	DEPTH (FT)	NORTHING	EASTING	LAT.	LONG.			
SCPT-02	125	565340.39	3500398.28	N30° 03' 09.90"	W90° 38' 21.49"			
SCPT-03	60	565743.43	3500604.94	N30° 03' 13.88"	W90° 38' 19.11"			
SCPT-07	50	568231.16	3502393.88 N30° 03' 38.39		W90° 37' 58.58"			
SCPT-10	50	571062.03	3503797.33	N30° 04' 06.33"	W90° 37' 42.42"			
SCPT-11	125	572188.96	3504311.40	N30° 04' 17.46"	W90° 37' 36.49"			
SCPT-12	50	572904.30	3504166.45	N30° 04' 24.55"	W90° 37' 38.09"			
SCPT-13	50	574271.15	3503916.59	N30° 04' 38.09"	W90° 37' 40.83"			
SCPT-14	50	575165.77	3504185.25	N30° 04' 46.93"	W90° 37' 37.71"			

SUPPLEMENTAL CPT'S SHALL BE DRIVEN TO DESIGNATED DEPTH OR
TO REFUSAL, WHICH EVER OCCURS FIRST.

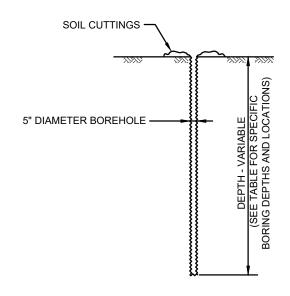
SUPPLEMENTAL BORING SCHEDULE (SB-XX)							
ITEM No.	DEPTH (FT)	NORTHING	EASTING	LONG.			
SB-01	125	565322.18	3500388.20	N30° 03' 09.72"	W90° 38' 21.61"		
SB-02	50	566414.48	3500934.90	N30° 03' 20.50"	W90° 38' 15.31"		
SB-03	125	567115.64	3501275.35	N30° 03' 27.42"	W90° 38' 11.39"		
SB-04	50	567671.70	3501742.96	N30° 03' 32.90"	W90° 38' 06.03"		
SB-05	50	568574.89	3502566.58	N30° 03' 41.79"	W90° 37' 56.60"		
SB-06	50	569650.22	3503099.19	N30° 03' 52.40"	W90° 37' 50.46"		
SB-07	50	570591.13	3503565.22	N30° 04' 01.68"	W90° 37' 45.09"		
SB-08	50	571599.70	3504063.64	N30° 04' 11.64"	W90° 37' 39.35"		
SB-09	125	572183.78	3504310.44	N30° 04' 17.40"	W90° 37' 36.50"		
SB-11	125	573933.09	3503858.41	N30° 04' 34.75"	W90° 37' 41.52"		
SB-12	50	574699.73	3504045.29	N30° 04' 42.33"	W90° 37' 39.34"		
SB-13	50	575551.89	3504301.98	N30° 04' 50.75"	W90° 37' 36.36"		

THE WIDTH OF CLEARING SHALL BE KEPT AS NARROW AS POSSIBLE, ONLY ENOUGH TO GET THE SOIL BORING RIG AND SUPPORT EQUIPMENT TO THE SITE. IF NEEDED, THE ACCESS ROUTE MAY VEER FROM A STRAIGHT PATH TO AVOID LARGE TREES OR OTHER ENVIRONMENTAL FEATURES, AS FEASIBLE.



\* WIDTH = 25' FROM RIVER RD. TO KCS RR WIDTH = 50' FOR ALL OTHER AREAS

**CLEARING CROSS-SECTION** 



#### TYPICAL BORING DETAIL



1515 POYDRAS STREET **SUITE 2700** NEW ORLEANS, LA 70112 (504) 586-8111



#### COASTAL PROTECTION AND RESTORATION **AUTHORITY**

150 TERRACE STREET

BATON ROUGE, LOUISIANA 70802

APPROVED BY: J. CLAY LOYLESS

#### RIVER REINTRODUCTION INTO MAUREPAS SWAMP/ WSLP FLOOD PROTECTION

ST. JOHN THE BAPTIST PARISH, LOUISIANA

BORING SCHEDULE CROSS SECTIONS BORING DETAILS GEOTECHNICAL SURVEYS

STATE PROJECT NUMBER: PO-0029 DATE: JUNE, 2019

DRAWN BY: ERIC E. WALTER

DESIGNED BY: J. CLAY LOYLESS

FEDERAL PROJECT NUMBER:

SHEET 8 OF 8

31 December 2020

AECOM Technical Services, Inc. **Suite 2700** 1515 Poydras Street New Orleans, Louisiana 70112

Attention Mr. Clay Loyless, P.E.

Ladies and Gentlemen:

Geotechnical Data Report State of Louisiana Coastal Protection and Restoration Authority (CPRA) 15% Design Phase Maurepas Diversion and West Shore Lake Pontchartrain St. John the Baptist Parish, Louisiana S.P. Purchase Order No. 0029 Eustis Engineering Project No. 24384

We are transmitting an electronic copy of our geotechnical data report covering a geotechnical exploration for the subject project. Hard copies are available upon request.

Thank you for asking us to perform these services.

Yours very truly,

**EUSTIS ENGINEERING L.L.C.** 

SEAN G. WALSH, P.E.

SGW:sh

LOUISIANA

TEXAS MISSISSIPPI

### GEOTECHNICAL DATA REPORT

### STATE OF LOUISIANA

## COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA)

#### **15% DESIGN PHASE**

## MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

ST. JOHN THE BAPTIST PARISH, LOUISIANA

S.P. PURCHASE ORDER NO. 0029

**EUSTIS ENGINEERING PROJECT NO. 24384** 

FOR AECOM TECHNICAL SERVICES, INC. NEW ORLEANS, LOUISIANA

> By Eustis Engineering L.L.C. Metairie, Louisiana

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Appendix VIII – Compilation of Shear Data Reports (SDR-A, SDR-B, and SDR-C)

Appendix IX – Compilation of Consolidation Test Reports (SDR-A, SDR-B, and SDR-C)

#### GEOTECHNICAL DATA REPORT

#### STATE OF LOUISIANA

### COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA)

#### **15% DESIGN PHASE**

#### MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

ST. JOHN THE BAPTIST PARISH, LOUISIANA

S.P. PURCHASE ORDER NO. 0029

EUSTIS ENGINEERING PROJECT NO. 24384

#### INTRODUCTION

- This draft geotechnical data report (GDR) presents the results of Eustis Engineering L.L.C.'s field exploration, laboratory testing program, and development of soil design parameters for the subject contract of the West Shore Lake Pontchartrain (WSLP) protection project in St. John the Baptist Parish in Louisiana. A final version of this report will be issued upon completion of agency and internal team reviews.
- 2. This project is located along the Maurepas Diversion and western shore of Lake Pontchartrain in St. John the Baptist Parish, Louisiana. Our geotechnical services for the project were performed in general accordance with our revised proposal, dated 21 May 2020. Authorization to proceed with these services were provided by AECOM Technical Services, Inc. (AECOM) by MCSS Task / Purchase Order No. 126099, Change Order 01, executed 1 July 2020. AECOM is the project civil and structural engineer. Eustis Engineering is the project geotechnical engineer. C. H. Fenstermaker & Associates, L.L.C.. (Fenstermaker) is the project surveyor. The Coastal Protection and Restoration Authority (CPRA) is the project owner and director for the Maurepas Diversion and the U. S. Army

Corps of Engineers (USACE) is the project sponsor for the future WSLP flood protection system that will be alongside CPRA's diversion.

### **SCOPE OF SERVICE**

3. Based on the furnished Scope of Work (SOW) dated 26 November 2019, we understand project comprises review of existing geotechnical data, development of a geotechnical data collection plan, collection of geotechnical data, laboratory analyses, and geotechnical design. These services were performed to support the 15% design phase for the subject project. This project includes the Maurepas Diversion structure and the WSLP flood protection. The scope of this effort focuses on the portion of the joint project which comprises a diversion structure running parallel the WSLP flood protection. The geotechnical exploration we present in this report was completed to define soil and foundation conditions along the future WSLP levee alignment, but we also include geotechnical information for the diversion beyond the area of the WSLP features. Refer to Sheet 1 of Figure 1 for a site vicinity plan of the alignment.

### AVAILABLE GEOTECHNICAL DATA

4. AECOM furnished available historic geotechnical data, analyses, and reports to Eustis Engineering for review. These data include major reports, published in 2008 and 2013, based on obtained geotechnical data consisting of soil borings and cone penetration tests (CPTs) performed throughout the project area at previous times. In addition, select correspondence included written approval from the USACE regarding strength lines on the Mississippi River batture to support a proposed cofferdam design submitted by AECOM in 2013. We present the pertinent soil boring logs and CPT logs from the prior

explorations in Appendix I and Appendix II, respectively. Refer to Table 1 for a summary of field data utilized for this GDR.

TABLE 1: SUMMARY OF AVAILABLE FIELD EXPLORATION DATA

EXPLORATION NAME/DESIGNATION	TYPE
B-05	Boring
B-06	Boring
B-07	Boring
B-08	Boring
B-09	Boring
B-10	Boring
B-11	Boring
B-12	Boring
B-13	Boring
B-14	Boring
B-15	Boring
B-16	Boring
B-17	Boring
B-18	Boring
B-19	Boring
B-2A	Boring
B-3A	Boring
B-4A	Boring
KCS-01	Boring
CPT-03	СРТ
CPT-04	СРТ
CPT-05	СРТ
CPT-06	СРТ
CPT-07	СРТ
CPT-08	СРТ
CPT-09	СРТ
CPT-12	СРТ
CPT-13	СРТ
CPT-15	СРТ

# **CURRENT FIELD EXPLORATION**

5. In addition to the furnished data, Eustis Engineering performed additional field explorations to provide current soil conditions at the site at a spacing to achieve HSDRRS requirements for the future levee and structure foundations. The field exploration

consisted of 12 borings and 8 cone penetration tests (CPTs) which were performed between 23 July and 5 November 2020. Refer to Appendix III and Appendix IV for the soil boring logs and CPT logs, respectively, we developed for this exploration. The field exploration done by Eustis Engineering was designated with an "S" before the name/designation to indicate supplemental work. All soil borings drilled in 2020 were 5-in. diameter undisturbed borings and follow the USACE's format for presentation on the boring logs for consistency with other WSLP geotechnical explorations. Refer to Appendix III and IV for exact dates of completion. Refer to Table 2 for a summary of our current field explorations.

TABLE 2: SUMMARY OF CURRENT FIELD EXPLORATION

EXPLORATION NAME/DESIGNATION	ТҮРЕ
SB-01	BORING, 5" UNDISTURBED
SB-02	BORING, 5" UNDISTURBED
SB-03	BORING, 5" UNDISTURBED
SB-04	BORING, 5" UNDISTURBED
SB-05	BORING, 5" UNDISTURBED
SB-06	BORING, 5" UNDISTURBED
SB-07	BORING, 5" UNDISTURBED
SB-08	BORING, 5" UNDISTURBED
SB-09	BORING, 5" UNDISTURBED
SB-11	BORING, 5" UNDISTURBED
SB-12	BORING, 5" UNDISTURBED
SB-13	BORING, 5" UNDISTURBED
SCPT-02	СРТ
SCPT-03	СРТ
SCPT-07	СРТ
SCPT-10	СРТ
SCPT-11	СРТ
SCPT-12	СРТ
SCPT-13	СРТ
SCPT-14	СРТ

### LABORATORY TESTING

- All 4-ft samples collected from 5-in. diameter borings were extruded in the laboratory after being transported from the field to the laboratory in racks that hold the samples in a vertical orientation. After extruding, dividing, and labeling the samples, each 12-in. segment was visually classified and a moisture content specimen was taken from each segment. A laboratory log of sample numbers, depths, soil classifications, and moisture contents was prepared by Eustis Engineering and shared with AECOM and CPRA to develop soil laboratory testing assignments. The 1-ft sample segments were preserved in moisture proof containers pending laboratory testing. Testing assignments were developed in general accordance with those followed by the USACE for the other portions of the West Shore of Lake Pontchartrain (WSLP) alignments (101 through 109). We did not perform testing in accordance with the LFPDG per instruction from CPRA in July 2020.
- 7. After development of our preliminary test results, additional test assignments were made comprising unit weight determinations, unconfined compression shear tests (UC), three-point unconsolidated undrained triaxial compression shear tests (UU), and one-point unconsolidated undrained triaxial compression shear tests (OB), and Atterberg liquid and plastic limits determinations. Three-point "UU" tests were performed on three specimens trimmed from a single 5-in. diameter 1-ft segment of the "U" borings. One-point "OB" tests were performed on similar sample sizes as the UU tests as well as from other samples using a larger specimen size of 2.8 inches. To define the compressibility potential of foundation soils, we performed consolidation tests on select samples from Holocene soils along the alignment. We also performed consolidation testing in some instances to assist with assessing shear strengths of a silty clay deposit positioned above the Pleistocene interface because conventional shear testing results of that material produced unexpectedly low strengths. We also performed direct simple shear testing and

hydrometer testing on the same material to aid in strength characterization and material classification. These additional tests were selected through collaboration with CPRA.

- 8. We present a compilation of laboratory test results for all new borings in Appendix V of this report. The summaries include results of natural moisture content, total unit weight, unconfined compression shear, one-point unconsolidated-undrained shear, three-point unconsolidated-undrained shear, and torvanes. Also shown are the results of percent passing the No. 200 sieve, Atterberg Limits, and percent organic contents. The summary tables identify which samples were tested for sieve analysis and hydrometer analysis. These summaries also show which samples were selected for direct simple shear and consolidation testing. In Appendix VI we present the results of our grain size gradation analyses (sieve and hydrometer). In Appendix VII we present the results of our consolidation tests. In Appendix VIII we present all shear test results. In Appendix IX we present the individual consolidation test results.
- 9. CPRA requested Eustis Engineering to perform additional testing within the near-surface soils to aid in a preliminary assessment of using on-site soils as a levee borrow source. Tests comprised organic content, percent passing the No. 200 sieve, Atterberg limits, and natural moisture content. All parties are aware that this testing was done to aid in a preliminary assessment of potential suitability of using clays on-site for levee construction. A detailed suitability study is not part of the scope of service for this report and will need to be performed at a future date. The results of this additional testing is shown on the testing summary reports in Appendix V.

### **CPT INTERPRETED STRENGTHS**

10. Interpreted undrained shear strength plots from the CPT data are based on an N<sub>kt</sub> value of 15. We plan to proceed using a N<sub>kt</sub> value of 15 for the project alignment based on the results we present on Figure 3. CPT records shown in Appendix IV present soil behavior types based on non-normalized soil behavior types developed by Robertson & Campanella (1986).

## **SOIL DESIGN REACHES**

- 11. The Maurepas Diversion was separated into three soil design reaches (SDRs). In Figure 2 (Sheets 1 through 9), we present subsurface soil profiles of the soil borings and CPTs. Refer to Table 3 for a summary of soil design "reaches" and station numbers that correspond to individual Sheets in Figure 2.
- Subsurface Soil Conditions & Design Parameters. We present our interpretation of subsurface soil conditions on the sheets provided in Figure 3. In general, we found a crust of soil overlying compressible soils of the Holocene Epoch and underlain by stiff to very stiff deposits from the Pleistocene Epoch. Of significance to the levee design will be the presence of compressible soils within the Holocene and the depth to which Pleistocene is encountered along the future levee alignment. Based on our review of the subsurface profiles in Figure 2 we prepared three distinct soil design reaches which we present in Table 3 below.

TABLE 3: OUTLINE OF SUBSURFACE SOIL PROFILE FIGURE DESIGNATIONS

FIGURE 2 SHEETS	SOIL DESIGN REACH	STATION NUMBERS
1 through 3	А	15+00 to 55+00
4 through 6	В	55+00 to 100+00
7 through 9	С	100+00 to 290+00

- 13. SDR-A extends from River Road to Canadian Rail (CN) approximately. This reach has a stiffer crust material but this reach also has the deepest depths to the Pleistocene. We approximated the Pleistocene interface at el -32 for SDR-A. SDR-B generally extends between CN and the south side of US-61 in which the Holocene deposits extend to approximately el -25 (on average). SDR-C then extends from US-61 to the end of the project alignment. Based on our review SDR-C we approximated the interface of the Holocene and Pleistocene at el -15. Particular deposits of interest that we encountered within the Holocene soils are a thicker layer of organic clays and peats at SB-04 (drilled within the adjacent Marathon property) and a deposit of silty clay positioned above the Pleistocene interface that produced relatively low laboratory shear strength results. To aid in finer classifications of the material positioned above the Pleistocene we performed further strength and classification testing (as defined in the laboratory testing paragraph of this report).
- 14. We present our soil design parameters in Figure 3. On Sheet 1 we present soil design parameters comprising natural moisture content, total unit weight, undrained shear strength (using results from laboratory testing and interpreted CPT strengths), and Soil Behavior Types (I<sub>c</sub>) as interpreted from CPT data. On Sheets 2 and 3 we present our settlement and compressibility design parameters comprising natural moisture content, Atterberg limits, compression index (C<sub>c</sub>), estimates of initial void ratio (e<sub>o</sub>), overconsolidation ratio estimates (OCR), Soil Behavior Types (I<sub>c</sub>) from interpreted CPT data, and estimates of the Coefficient of Permeability (C<sub>v</sub>). On Sheet 4 we present correlation data from testing performed to correlate a 5-inch diameter undisturbed boring and a CPT for SDR-A and SDR-B. Refer to Table 4 for a summary of soil design reaches (SDR) and station numbers that correspond to each respective Figure.

TABLE 4: OUTLINE OF SOIL DESIGN PARAMETER FIGURE DESIGNATIONS

FIGURE 3 SHEETS	SOIL DESIGN REACH	STATION NUMBERS
3A, Sheets 1 through 4	А	15+00 to 55+00
3B, Sheets 1 through 4	В	55+00 to 100+00
3C, Sheets 1 through 3	С	100+00 to 290+00

- 15. On Figure 3 we present our estimates of vertical permeability rates (C<sub>v</sub>) from consolidation testing and through correlations from the liquid limits of soil samples. Our current plans are to construct wick drain fields for the levee tie-ins at the proposed structures for this contract. Because of this, estimates of time-rate settlement which are governed by C<sub>v</sub>, will have less significance on our results because we plan to use ground improvement where C<sub>v</sub> will not govern long-term design.
- 16. Areas of levee construction in the absence of ground improvement will require other consideration. The near-surface swamp and marsh deposits between the Kansas City Railway and Interstate I-10 comprise extremely soft, highly-compressible organic clays and peats and based on our interpretation of the available data the typical correlations values indicate low settlement rates because those correlations are not intended for such swampy/marsh soils. That approach can mislead estimates of settlement if we use correlations. We instead opted to rely on area experience with levee construction on similar soils in which we anticipate settlement to occur rapidly in these very soft near surface soils due to horizontal drainage paths. Further to this, we anticipate localized plastic deformations, during embankment construction, are most likely to manifest resulting in considerable displacements of the near-surface materials (i.e., mudwave propagation).
- 17. Our preparation of these soil design parameter estimates is based on our assessment of the available geotechnical data for the subject project alignment as included in the Furnished Information and our recent exploration. Consideration of data in adjacent

project reaches (WSLP and nearby Hope Canal Pump Station) may be appropriate to develop geotechnical project reaches. In particular, the use of consolidation data in adjacent project reaches or within similar geologic conditions may provide greater accuracy of the Holocene soils. We understand the development of soil design parameters by multiple parties is on-going for the various WSLP project reaches. We request for the discussion with the USACE (and possibly the other design teams) to assess how our soil design parameter estimates align with the other project reaches. This can ensure our assumptions align with the overall project goals for design and construction. We also understand the State of Louisiana, Office of Coastal Protection and Restoration (CPRA) may have developed an overarching set of design parameters that considers all of the geotechnical data and geologic variations along the entire protection alignment. Thus, CPRA's participation in a meeting(s) is also considered to offer value to these designs. Coordination during this design phase will enable final design plans developed for construction to be more seamless and also facilitate the USACE's plans to combine aspects of the construction testing such as the test pile programs.

## **LIMITATIONS**

18. This report has been prepared in accordance with generally accepted geotechnical engineering practice for the exclusive use of AECOM for specific application to the subject site. In the event of any changes in the nature or location of the proposed features, the information contained in this report shall not be considered valid unless the changes are reviewed and this report is modified and verified in writing. Should these data be used by anyone other than AECOM, the user should contact Eustis Engineering for interpretation of data and to secure any other information pertinent to this project.

- 19. Our findings in this report are based on selected points of field exploration, laboratory testing, and our understanding of the proposed project. Further variations in soil or ground water conditions could exist between and beyond the exploration points. The nature and extent of these variations may not become evident until construction. Variations in soil or ground water may require additional studies, consultation, and possible revisions to our recommendations.
- 20. Eustis Engineering has striven to provide our services in accordance with accepted geotechnical engineering practices in this locality at this time. No warranty or guarantee is expressed or implied. The results of the soil borings, CPTs, and laboratory tests contained in Appendices I through IX of this report may be included in the plans and specifications.
- 21. The scope of our services does not include an environmental assessment or an investigation for the presence or absence of wetlands and hazardous or toxic materials in the soil; surface water; ground water; or air on, below, or adjacent to the subject property. Furthermore, the scope does not include the investigation or detection of biological pollutants at the site. The term "biological pollutants" includes but is not limited to molds, fungi, spores, bacteria, viruses, and the byproducts of any such biological organisms.

\_\_\_\_\_



NOT TO SCALE

# VICINITY MAP



E	DRAWN BY: S.T.S.	JOB NO.: 24384
	CHECKED BY: J.M.W.	DATE: 30 DEC 2020
	CADD FILE:	FIGURE 1



NOT TO SCALE

Tenotes locations of undisturbed soil borings drilled under Eustis Engineering Project No. 24384 between 16 october and 2 november 2020

- ▲ DENOTES LOCATIONS OF CONE PENETRATION TESTS PERFORMED UNDER EUSTIS ENGINEERING PROJECT NO. 24384 BETWEEN 23 JULY AND 4 NOVEMBER 2020
- DENOTES LOCATIONS OF UNDISTURBED SOIL BORINGS DRILLED BY COASTAL PROTECTION AND RESTORATION AUTHORITY BETWEEN 1 SEPTEMBER 2007 AND 22 JANUARY 2013

△ DENOTES LOCATIONS OF CONE PENETRATION TESTS PERFORMED BY COASTAL PROTECTION AND RESTORATION AUTHORITY BETWEEN 12 SEPTEMBER 2007 AND 9 APRIL 2013

# BORING AND CPT LOCATION PLAN



DRAWN BY: S.T.S.	JOB NO.: 24384
CHECKED BY: J.M.W.	DATE: 29 DEC 2020
CADD FILE:	FIGURE 1 (SHEET 2 OF 6)



NOT TO SCALE

- The denotes locations of undisturbed soil borings drilled under Eustis Engineering Project No. 24384 between 16 october and 2 november 2020
- ▲ DENOTES LOCATIONS OF CONE PENETRATION TESTS PERFORMED UNDER EUSTIS ENGINEERING PROJECT NO. 24384 BETWEEN 23 JULY AND 4 NOVEMBER 2020
- DENOTES LOCATIONS OF UNDISTURBED SOIL BORINGS DRILLED BY COASTAL PROTECTION AND RESTORATION AUTHORITY BETWEEN 1 SEPTEMBER 2007 AND 22 JANUARY 2013
- ▲ DENOTES LOCATIONS OF CONE PENETRATION TESTS PERFORMED BY COASTAL PROTECTION AND RESTORATION AUTHORITY BETWEEN 12 SEPTEMBER 2007 AND 9 APRIL 2013

## BORING AND CPT LOCATION PLAN



DRAWN BY: S.T.S.	JOB NO.: 24384	
CHECKED BY: J.M.W.	DATE: 29 DEC 2020	
CADD FILE:	FIGURE 1 (SHEET 3 OF 6)	



NOT TO SCALE

- The denotes locations of undisturbed soil borings drilled under Eustis Engineering Project No. 24384 between 16 october and 2 november 2020
- ▲ DENOTES LOCATIONS OF CONE PENETRATION TESTS PERFORMED UNDER EUSTIS ENGINEERING PROJECT NO. 24384 BETWEEN 23 JULY AND 5 NOVEMBER 2020
- DENOTES LOCATIONS OF UNDISTURBED SOIL BORINGS DRILLED BY COASTAL PROTECTION AND RESTORATION AUTHORITY BETWEEN 1 SEPTEMBER 2007 AND 22 JANUARY 2013
- ▲ DENOTES LOCATIONS OF CONE PENETRATION TESTS PERFORMED BY COASTAL PROTECTION AND RESTORATION AUTHORITY BETWEEN 12 SEPTEMBER 2007 AND 9 APRIL 2013

## BORING AND CPT LOCATION PLAN



DRAWN BY: S.T.S.	JOB NO.: 24384	
CHECKED BY: J.M.W.	DATE: 29 DEC 2020	
CADD FILE:	FIGURE 1 (SHEET 4 OF 6)	



NOT TO SCALE

- The denotes locations of undisturbed soil borings drilled under Eustis Engineering Project No. 24384 between 16 october and 2 november 2020
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# BORING AND CPT LOCATION PLAN



DRAWN BY: S.T.S.	JOB NO.: 24384
CHECKED BY: J.M.W.	DATE: 29 DEC 2020
CADD FILE:	FIGURE 1



NOT TO SCALE

The denotes locations of undisturbed soil borings drilled under Eustis Engineering Project No. 24384 between 16 october and 2 november 2020

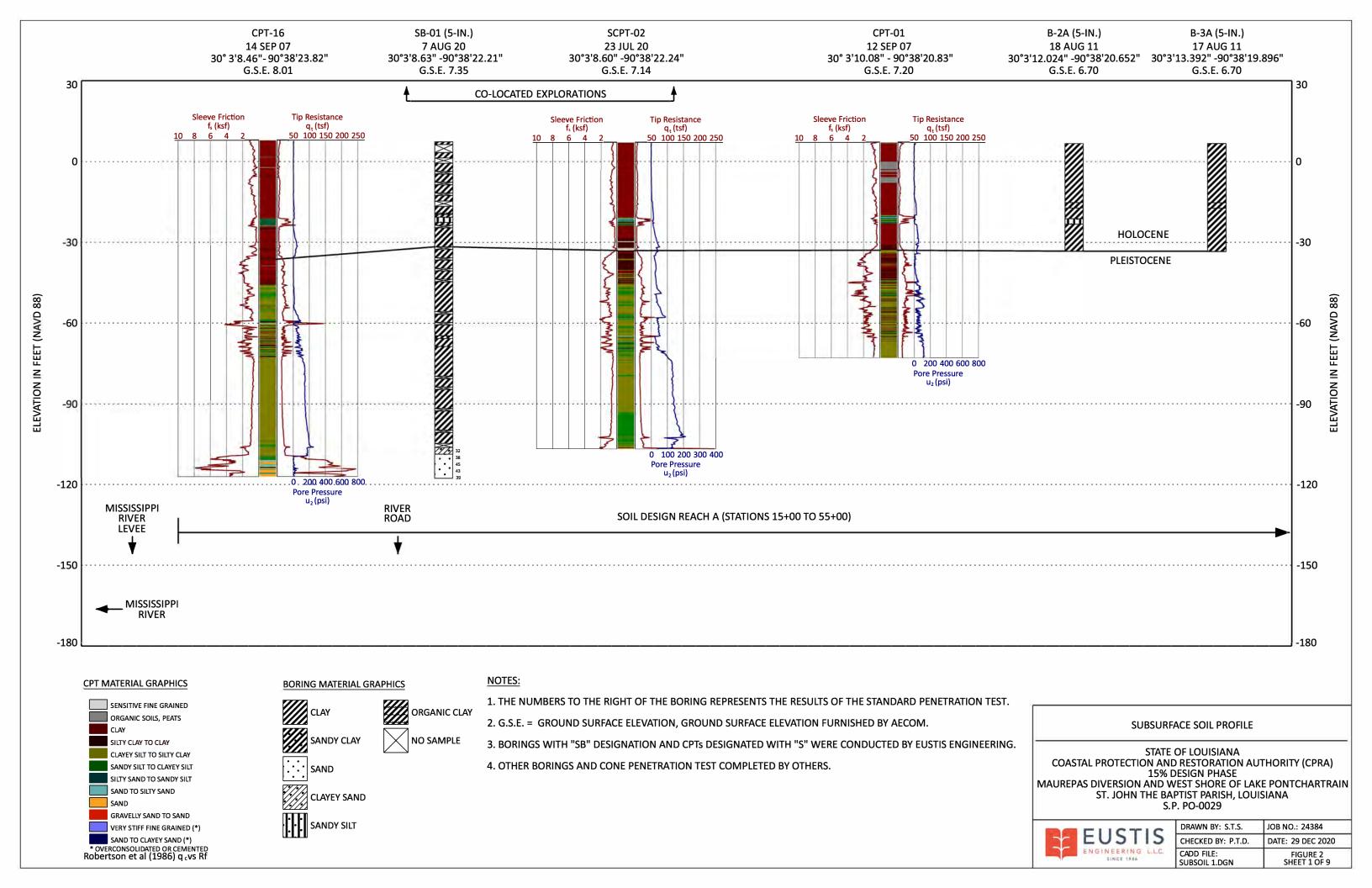
- ▲ DENOTES LOCATIONS OF CONE PENETRATION TESTS PERFORMED UNDER EUSTIS ENGINEERING PROJECT NO. 24384 BETWEEN 23 JULY AND 4 NOVEMBER 2020
- DENOTES LOCATIONS OF UNDISTURBED SOIL BORINGS DRILLED BY COASTAL PROTECTION AND RESTORATION AUTHORITY BETWEEN 1 SEPTEMBER 2007 AND 22 JANUARY 2013

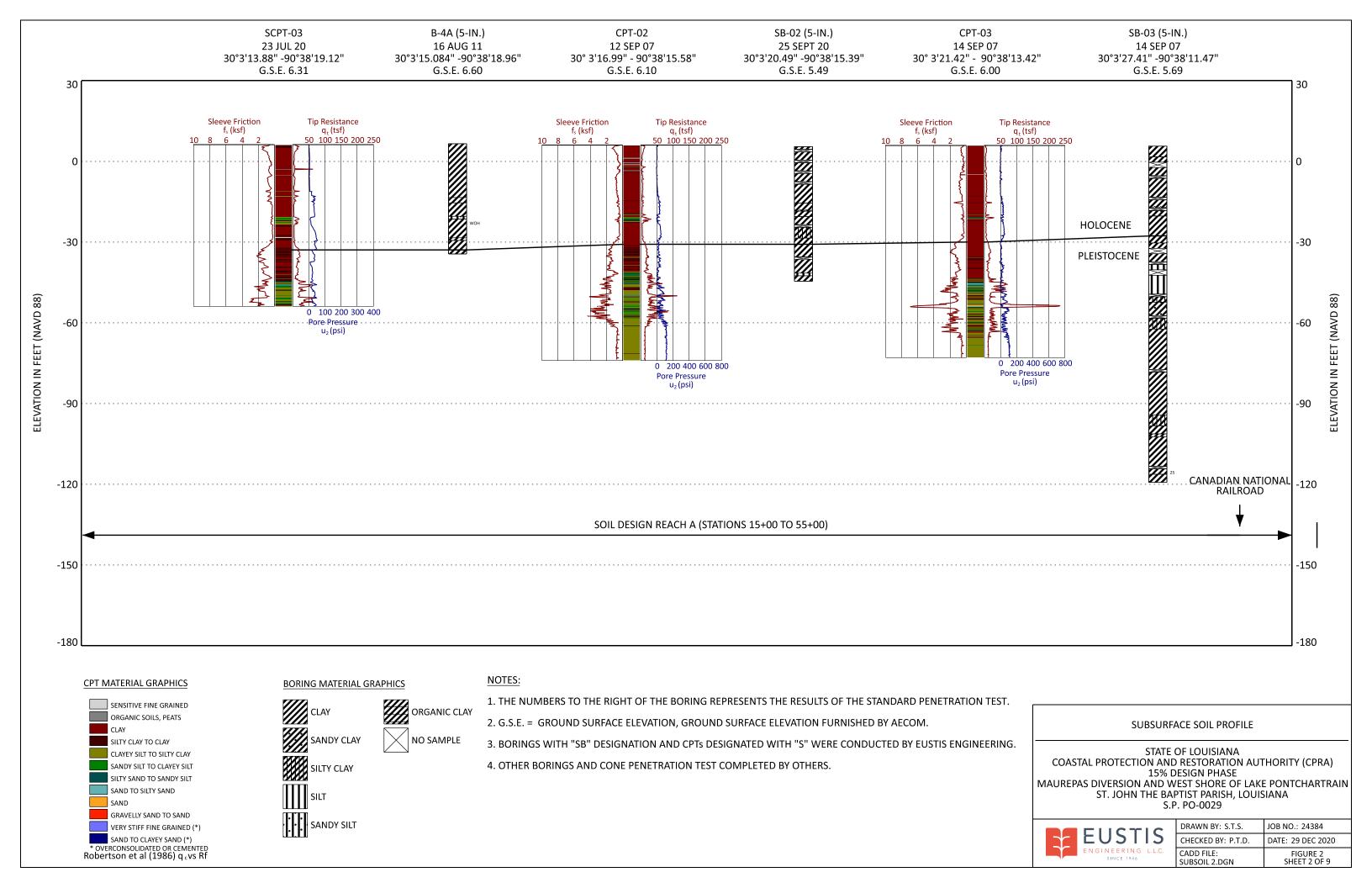
▲ DENOTES LOCATIONS OF CONE PENETRATION TESTS PERFORMED BY COASTAL PROTECTION AND RESTORATION AUTHORITY BETWEEN 12 SEPTEMBER 2007 AND 9 APRIL 2013

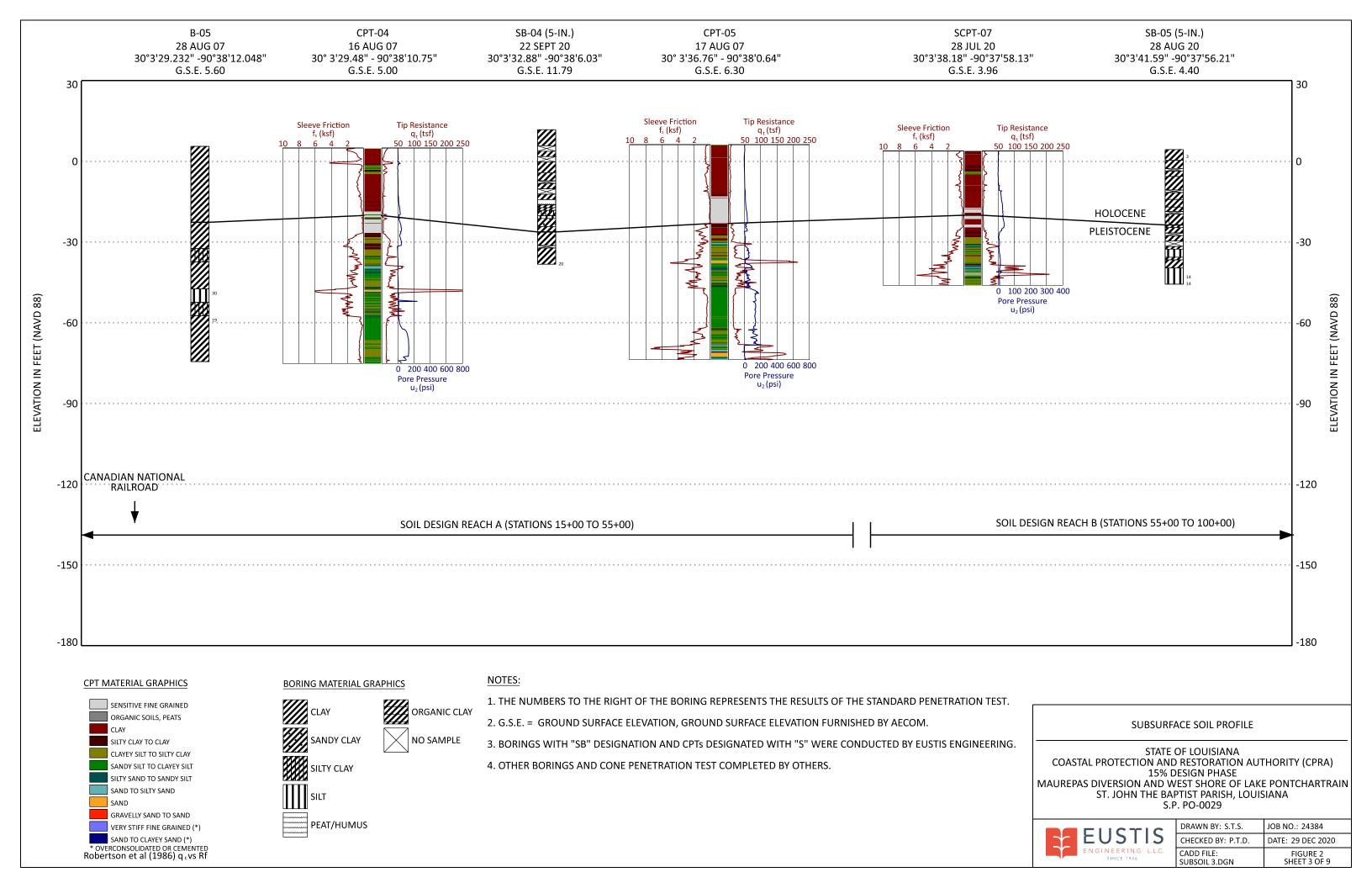
### BORING AND CPT LOCATION PLAN

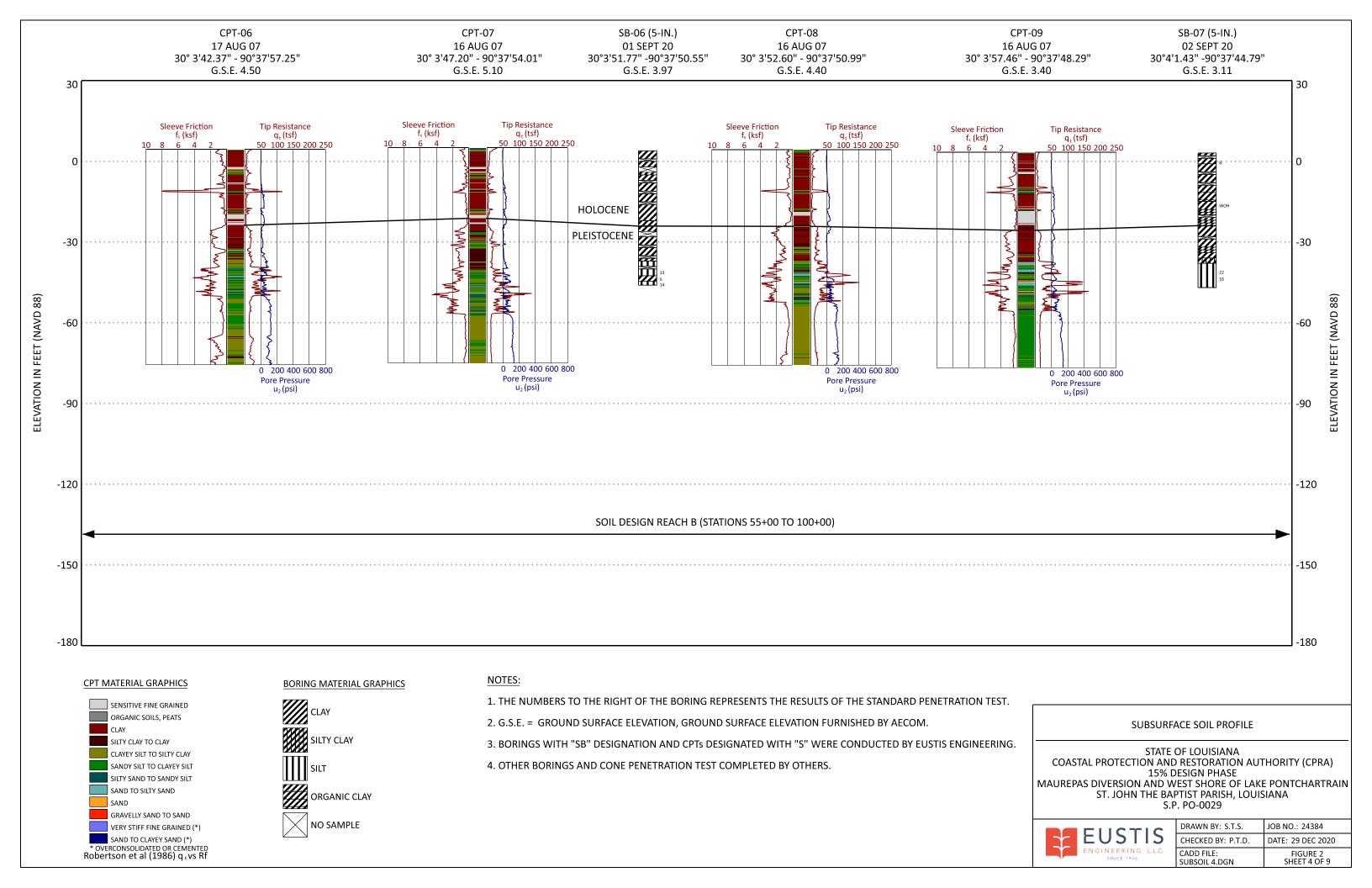


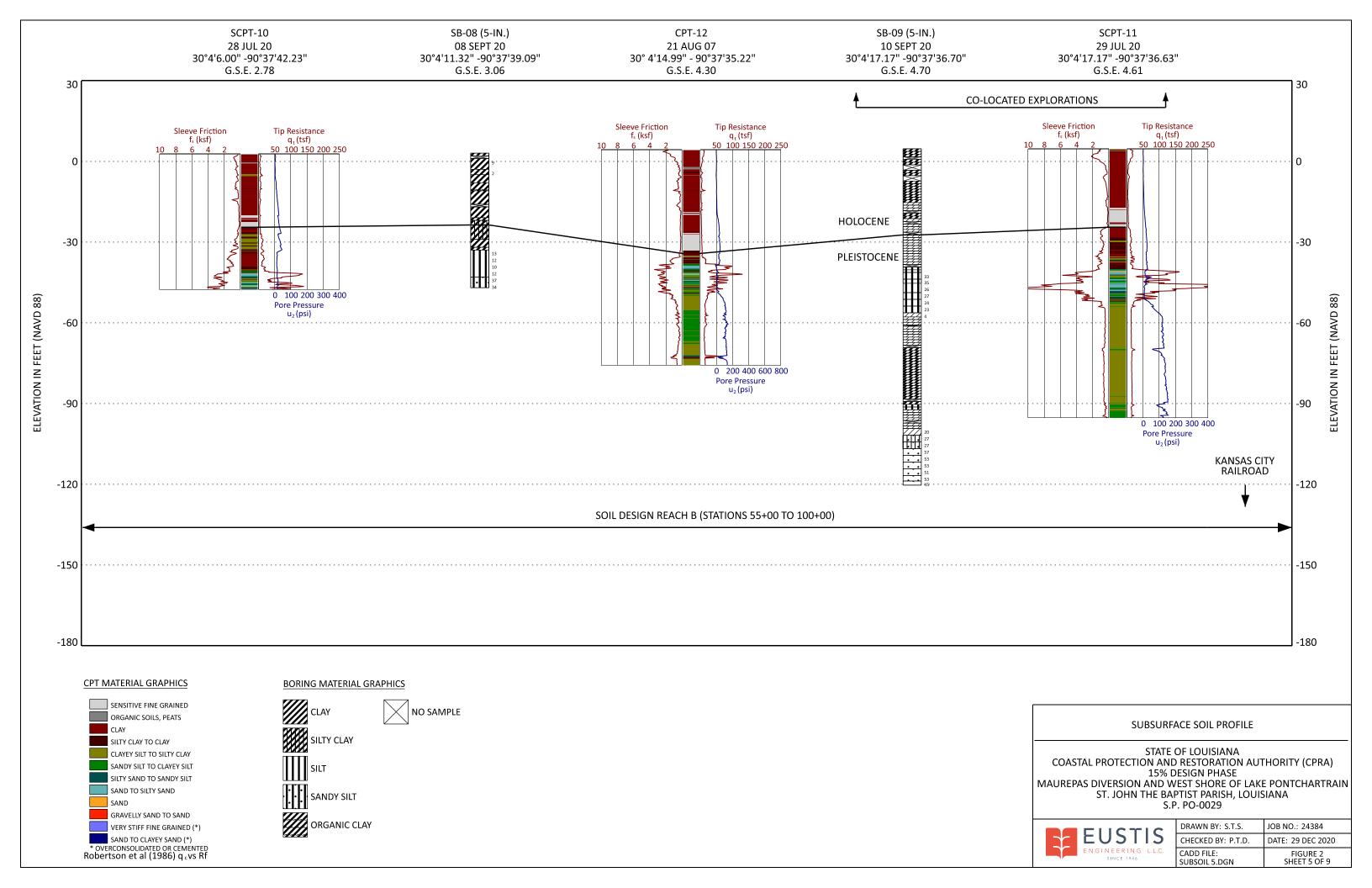
DRAWN BY: S.T.S.	JOB NO.: 24384
CHECKED BY: J.M.W.	DATE: 29 DEC 2020
CADD FILE: LOCATION 5.DGN	FIGURE 1 (SHEET 6 OF 6)

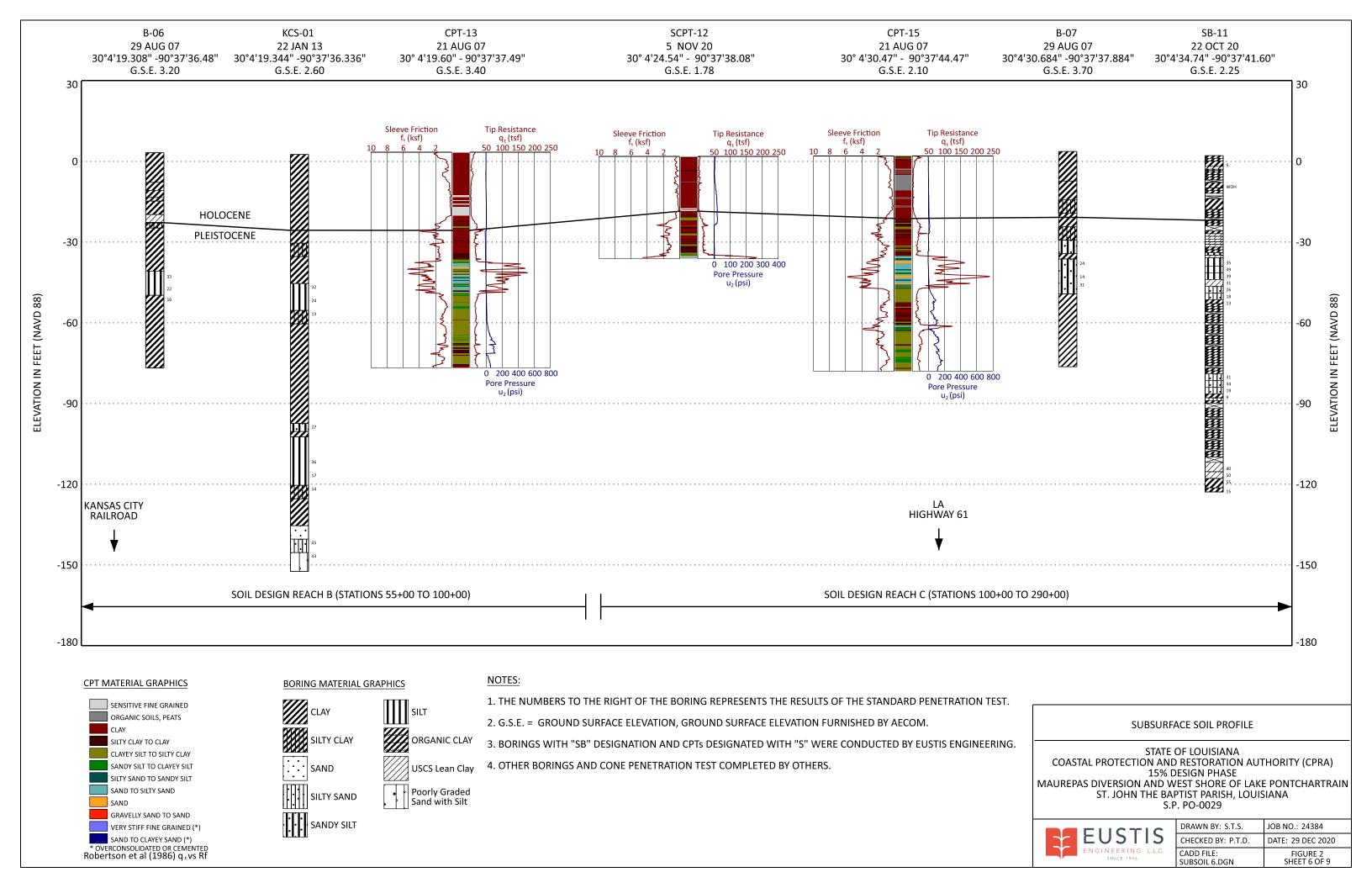


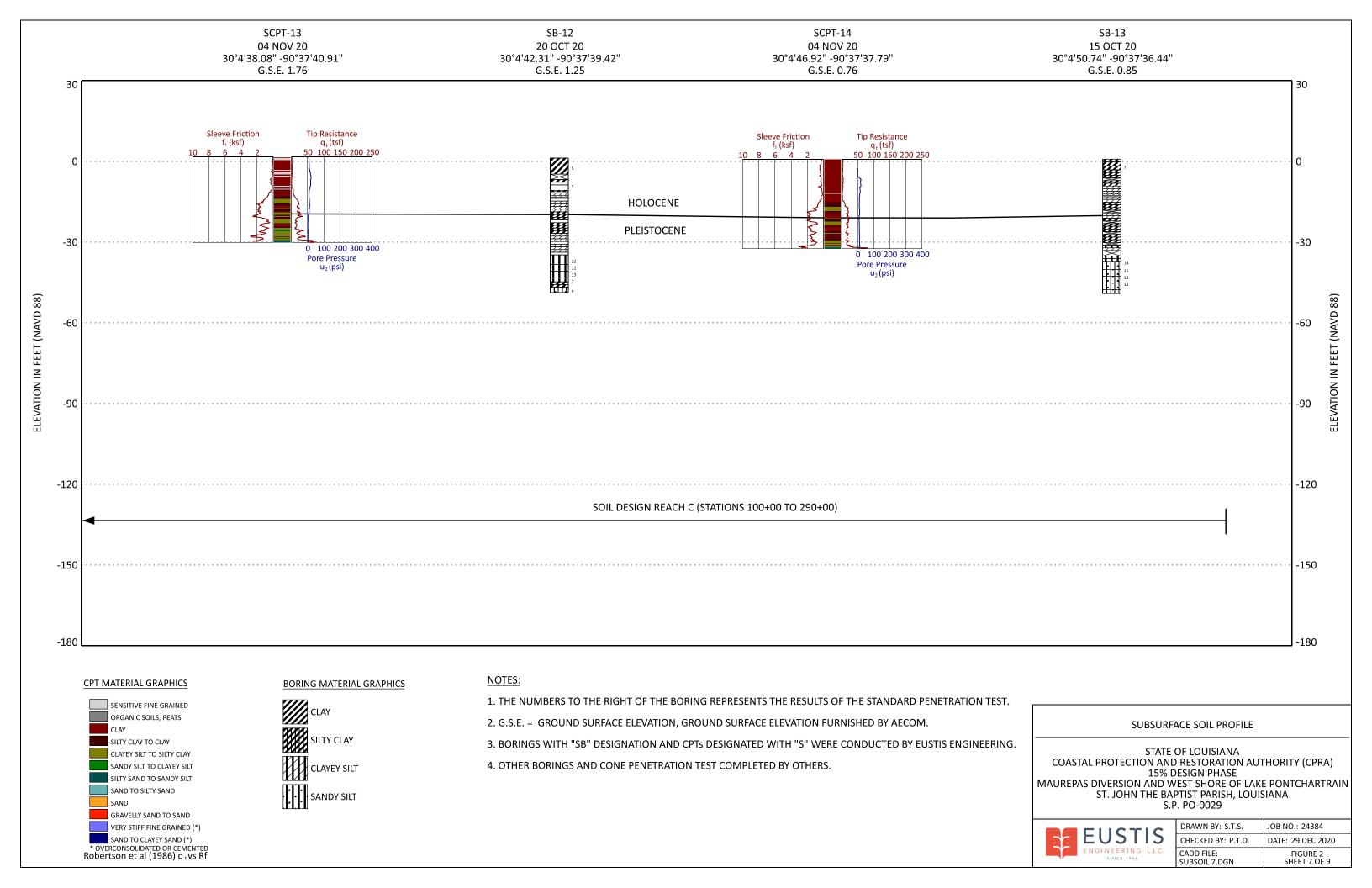


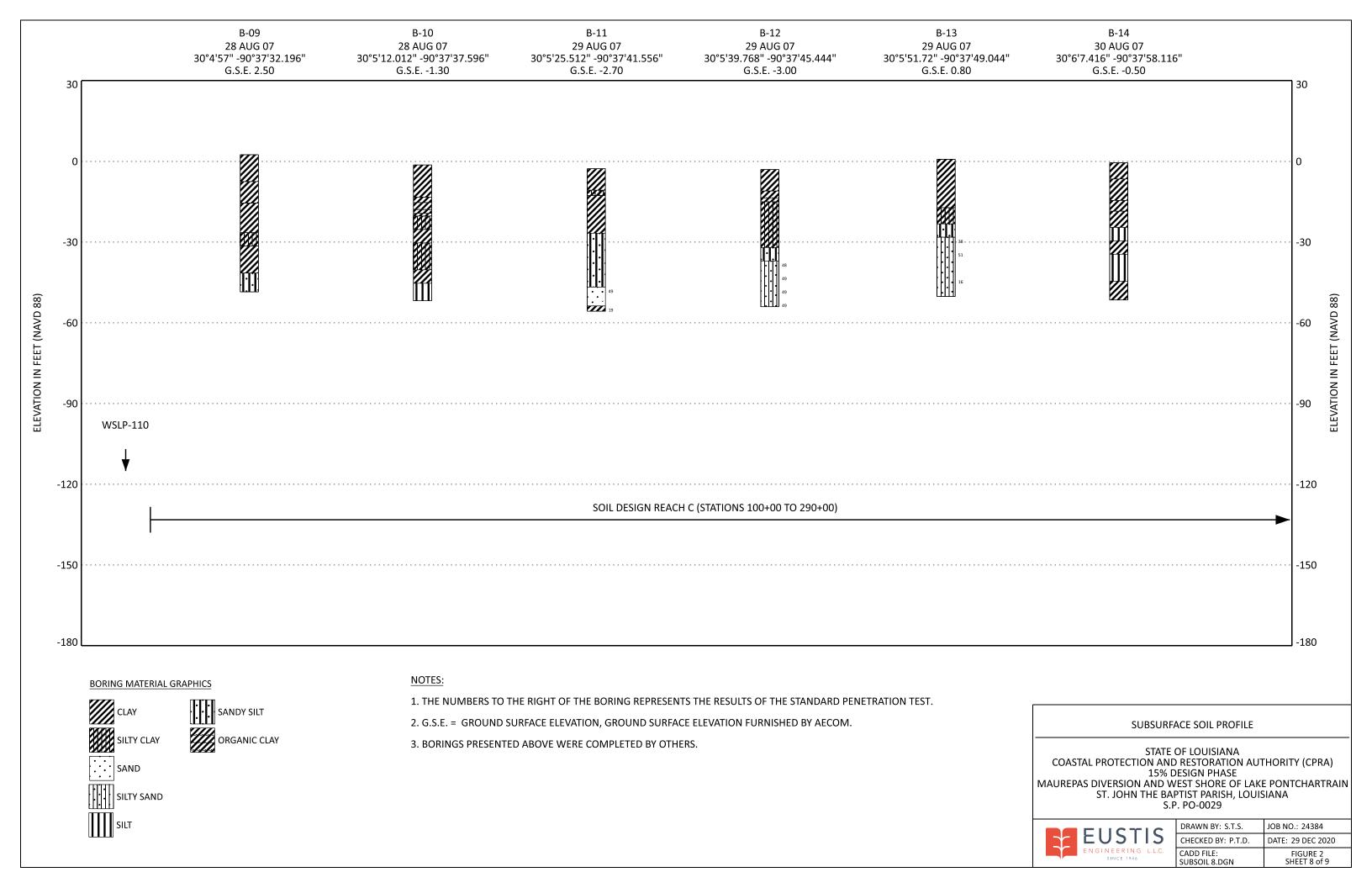


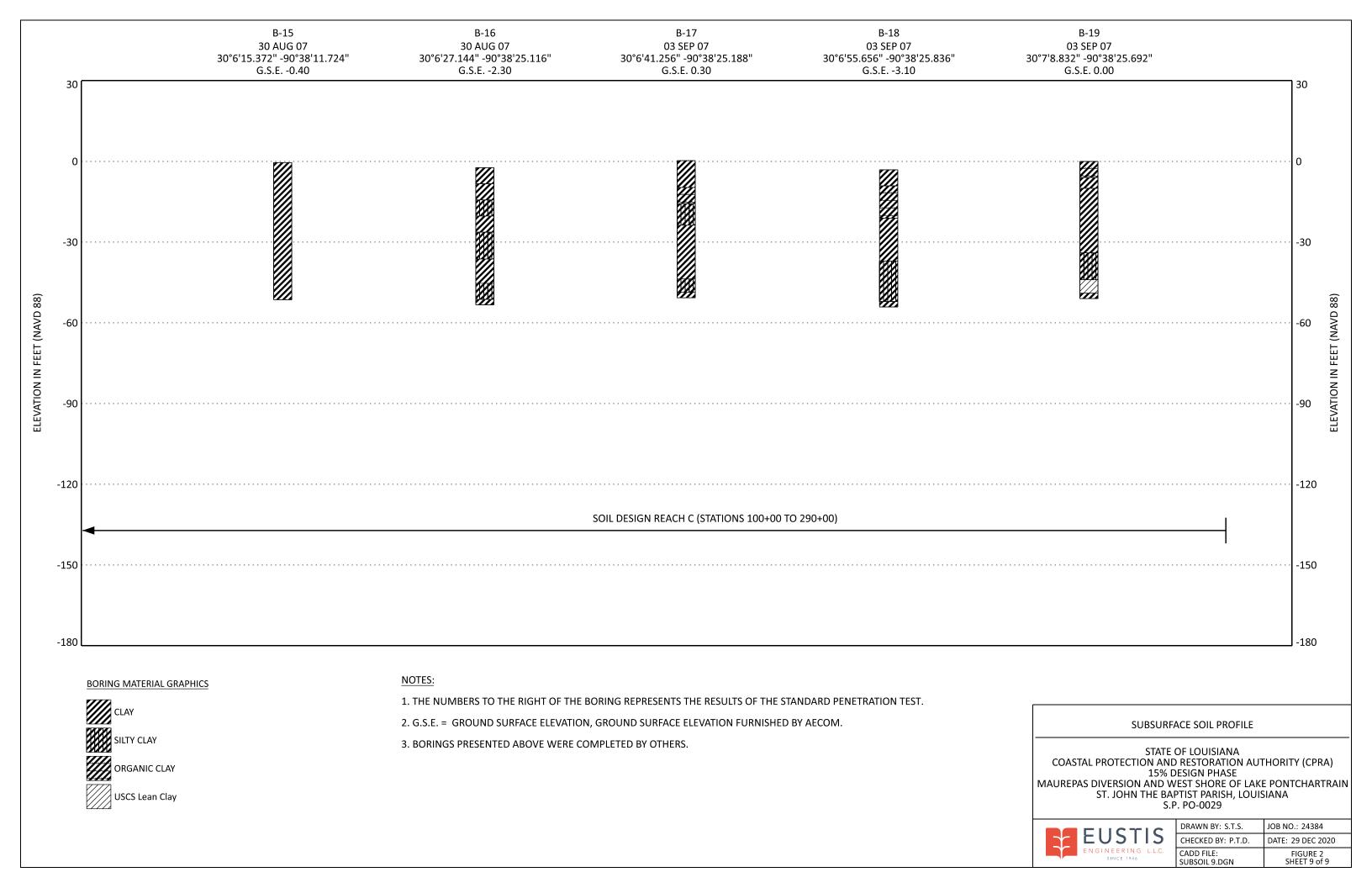


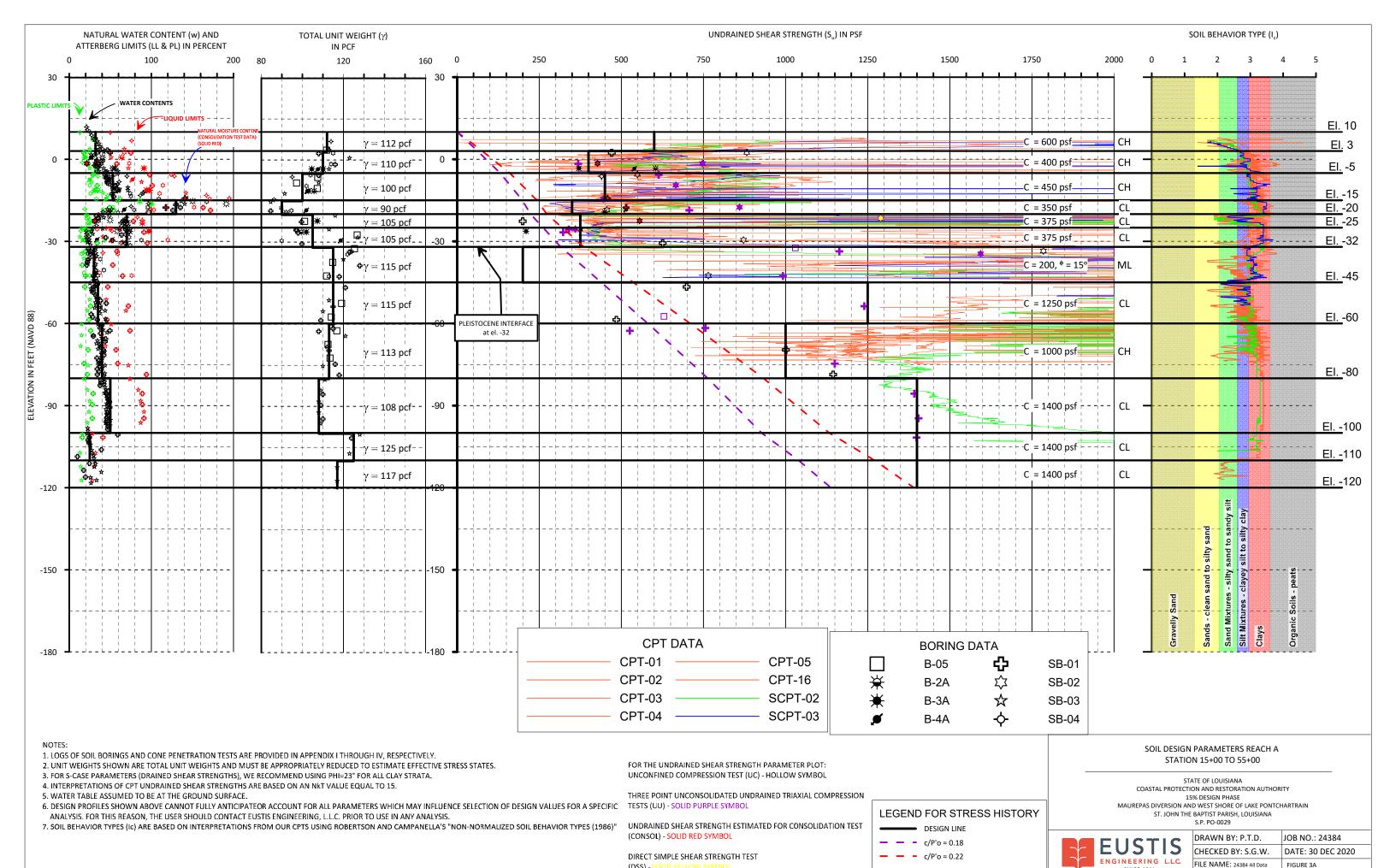






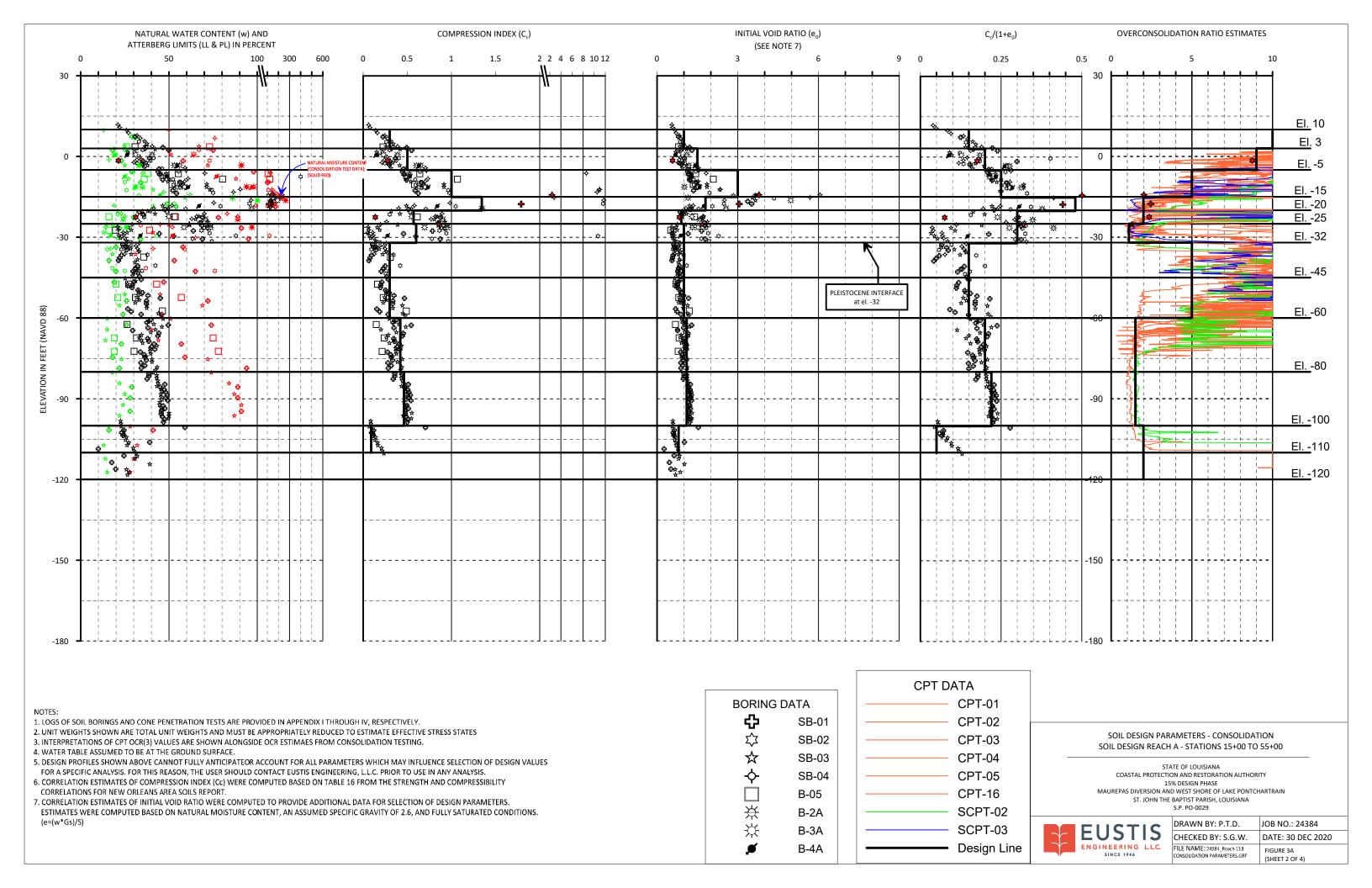


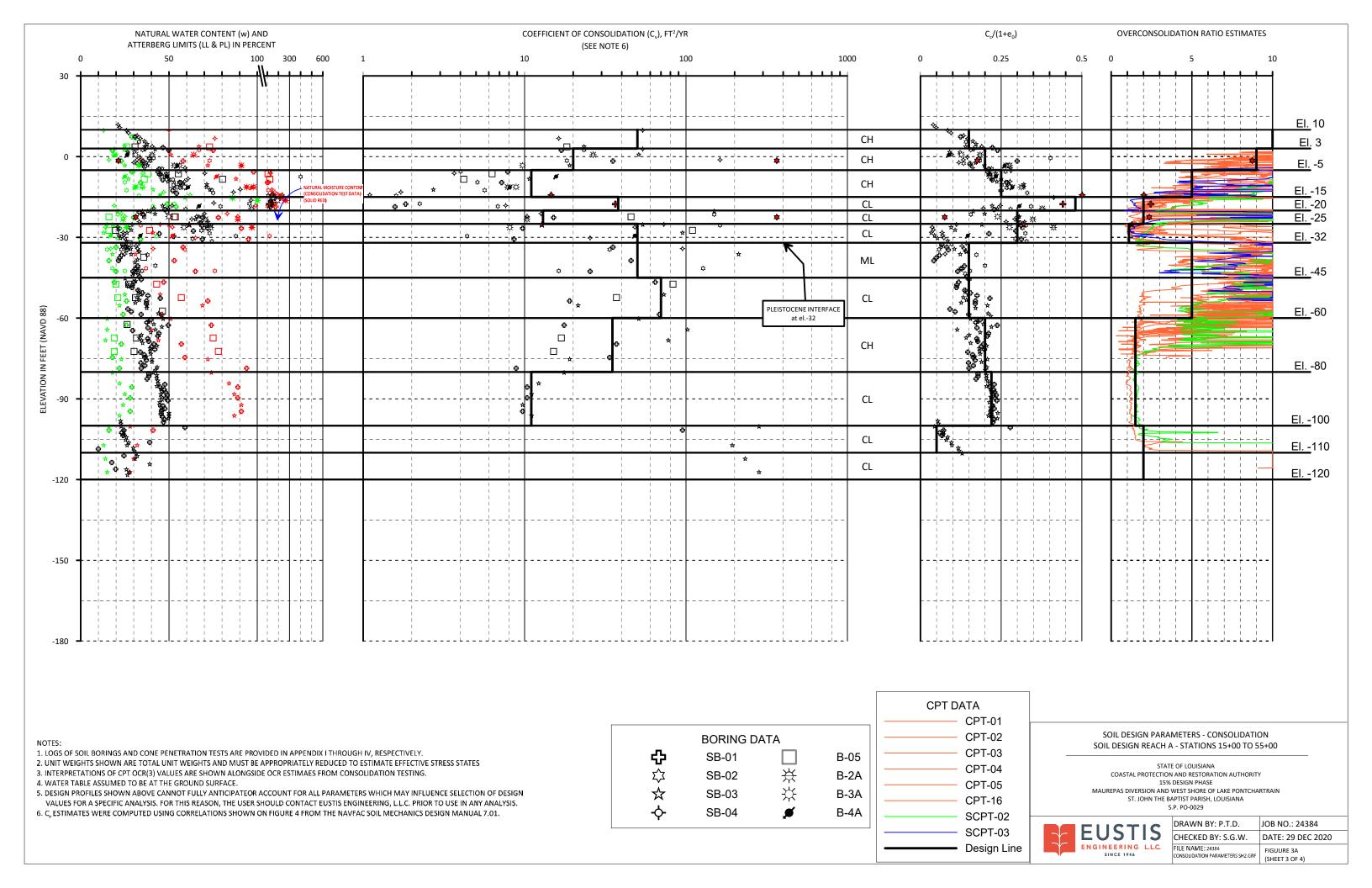


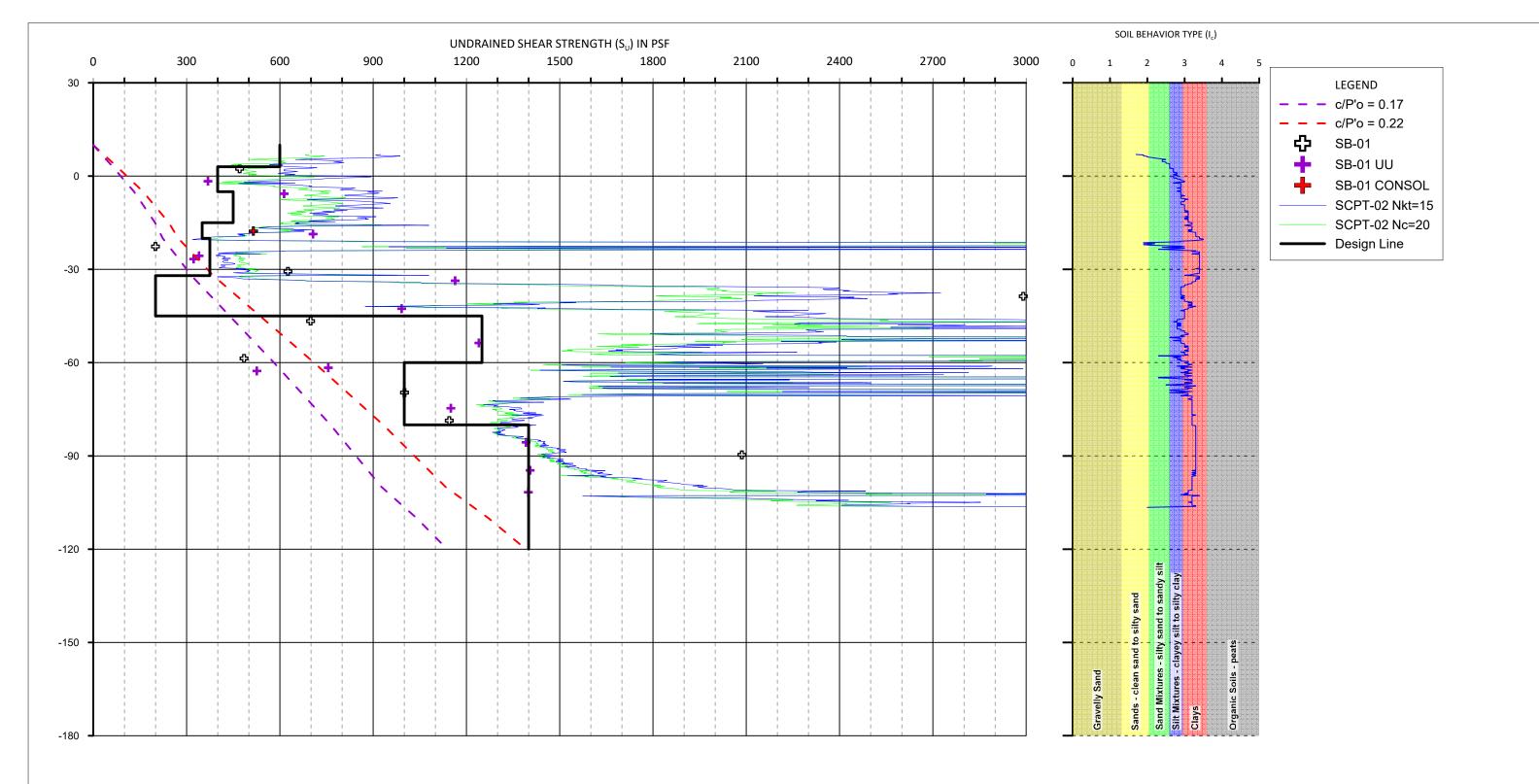


(DSS) - SOLID YELLOW SYMBO

FIGURE 3A







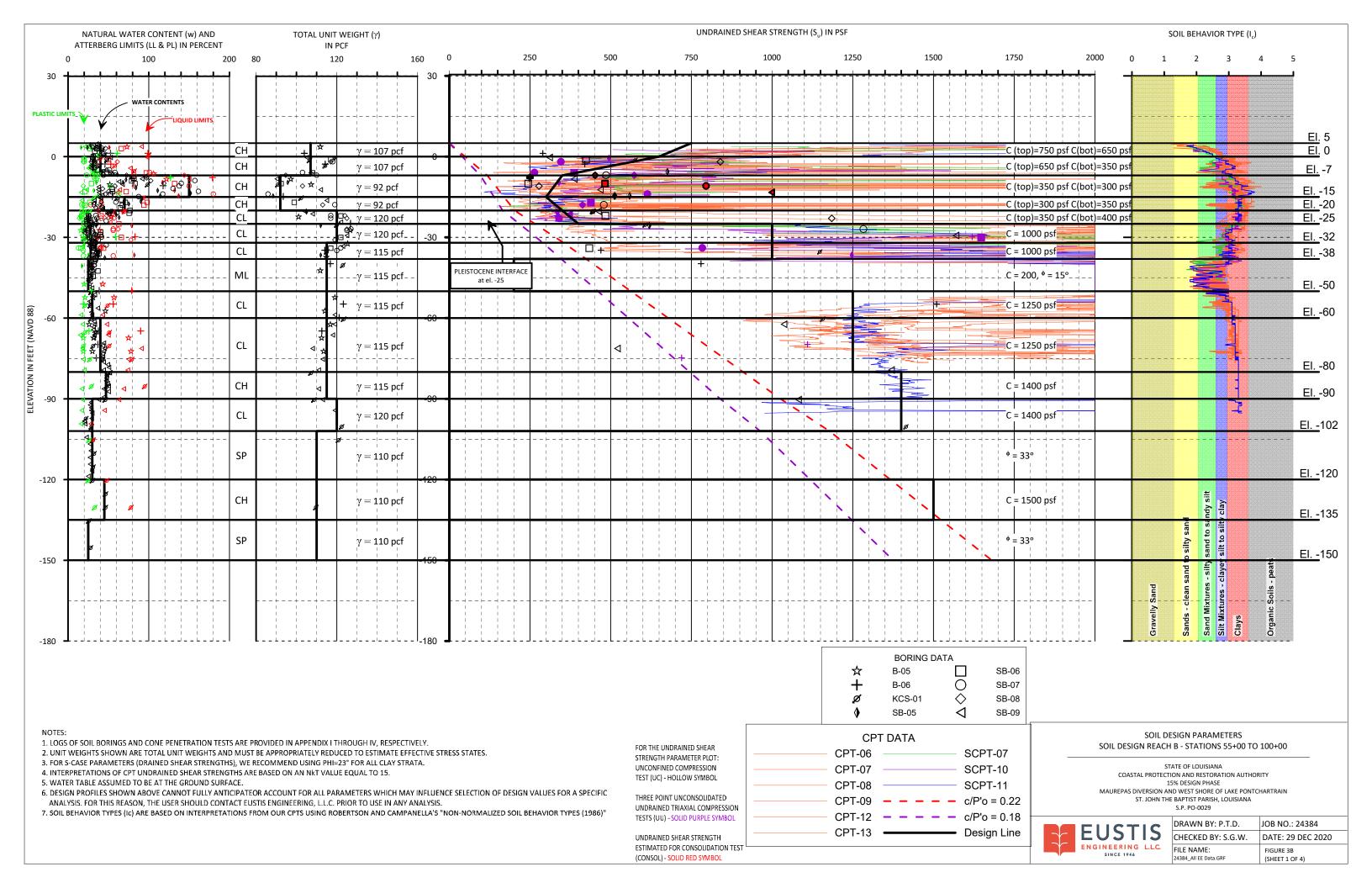
#### NOTES:

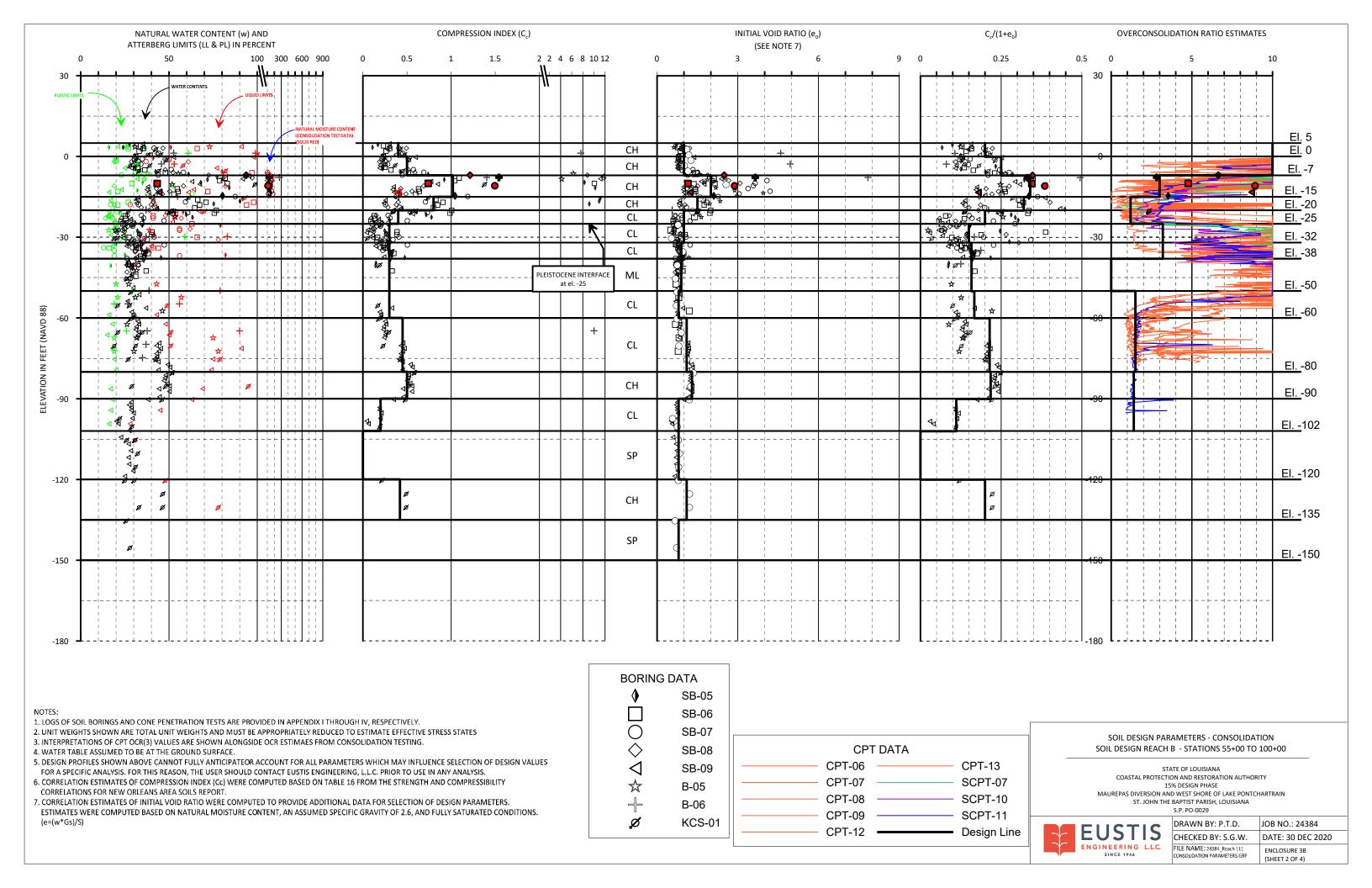
- 1. LOGS OF SOIL BORINGS AND CONE PENETRATION TESTS ARE PROVIDED IN APPENDIX I THROUGH IV, RESPECTIVELY.
- 2. INTERPRETATIONS OF CONE PENETRATION TEST SHEAR STRENGTH ESTIMATES ARE COMPUTED WITH  $N_{kT} = 15$ , AND  $N_{c}=20$  FOR COMPARISON. AS SHOWN ON SHEET 2, WE SELECTED  $N_{kT}=15$  FOR DESIGN LINE SELECTIONS.
- 3. WATER TABLE ASSUMED TO BE AT THE GROUND SURFACE.
- 4. DESIGN PROFILES SHOWN ABOVE CANNOT FULLY ANTICIPATEOR ACCOUNT FOR ALL PARAMETERS WHICH MAY INFLUENCE SELECTION OF DESIGN VALUES FOR A SPECIFIC ANALYSIS. FOR THIS REASON, THE USER SHOULD CONTACT EUSTIS ENGINEERING, L.L.C. PRIOR TO USE IN ANY ANALYSIS.
- 5. SOIL BEHAVIOR TYPES (Ic) ARE BASED ON INTERPRETATIONS FROM OUR CPTS USING ROBERTSON AND CAMPANELLA'S "NON-NORMALIZED SOIL BEHAVIOR TYPES (1986)"

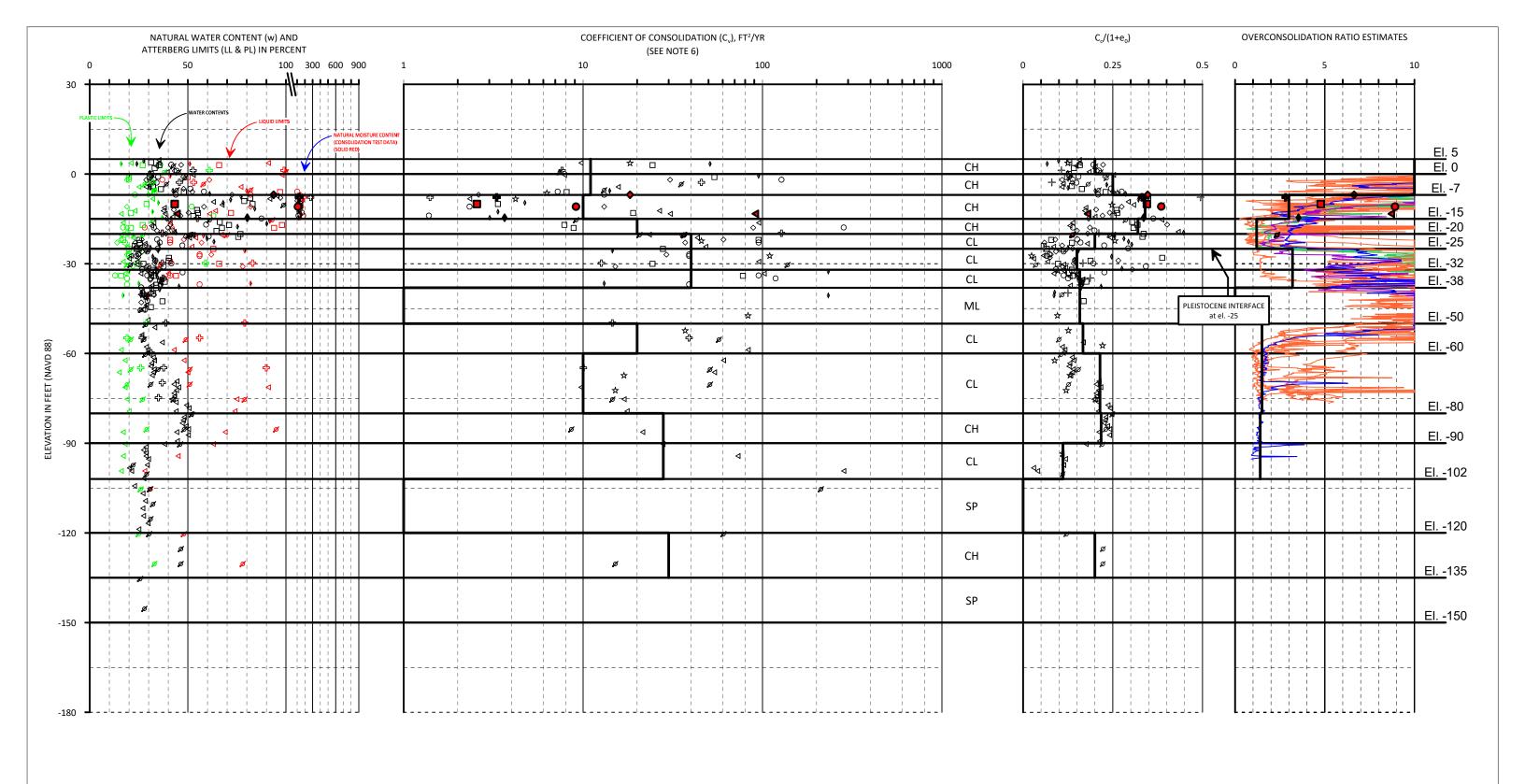
#### SOIL DESIGN REACH A STATION 15+00 TO 55+00



DRAWN BY: P.T.D.	JOB NO.: 24384
CHECKED BY: S.G.W.	DATE: 30 DEC 2020
FILE NAME: 24384_REACH 113 CPT CORRELATION.GRF	FIGURE 3A (SHEET 4 OF 4)

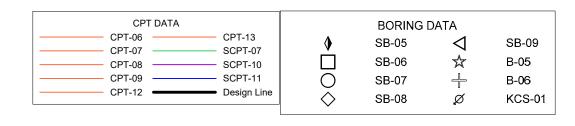






#### NOTES

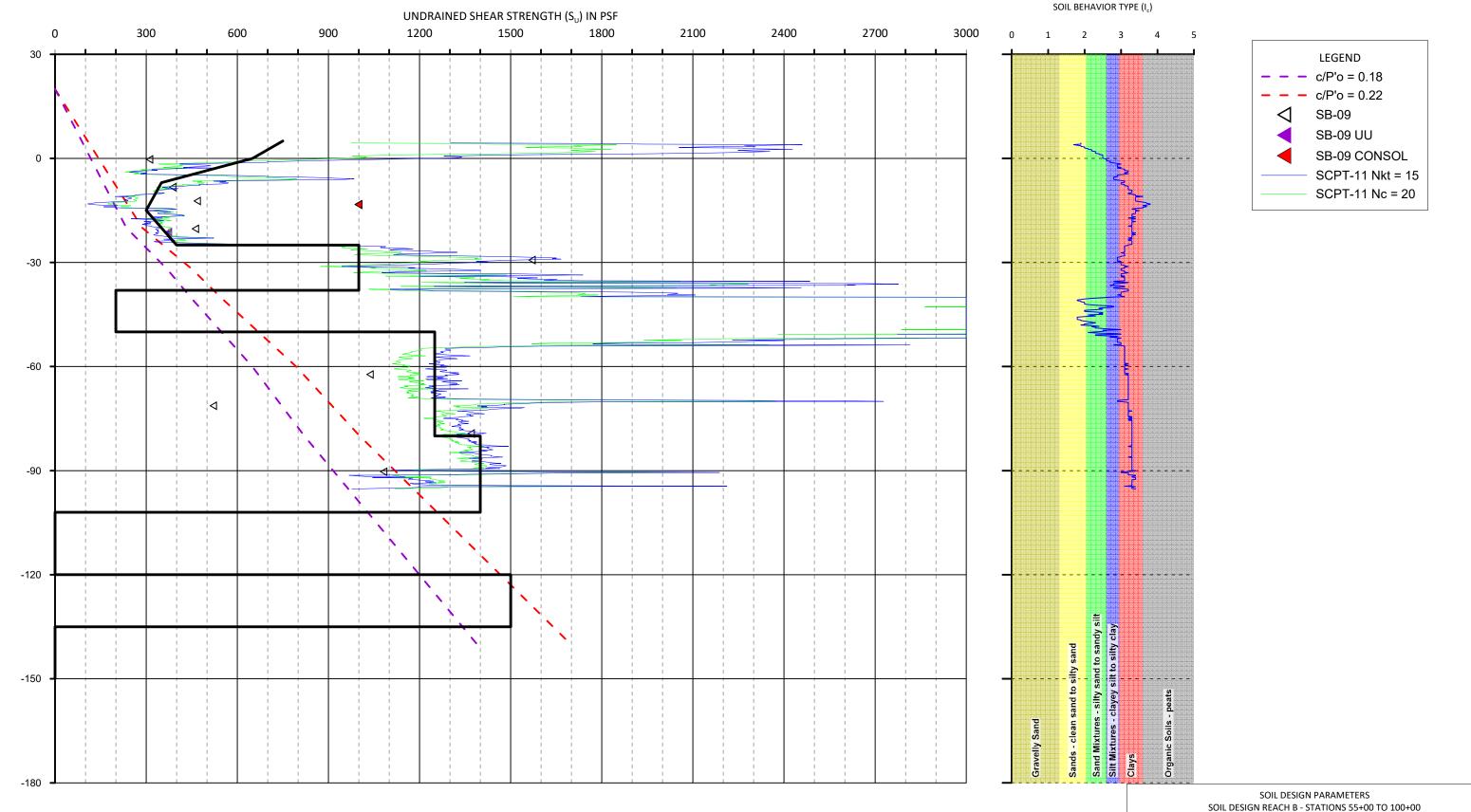
- 1. LOGS OF SOIL BORINGS AND CONE PENETRATION TESTS ARE PROVIDED IN APPENDIX I through IV, RESPECTIVELY.
- 2. UNIT WEIGHTS SHOWN ARE TOTAL UNIT WEIGHTS AND MUST BE APPROPRIATELY REDUCED TO ESTIMATE EFFECTIVE STRESS STATES
- 3. INTERPRETATIONS OF CPT OCR(3) VALUES ARE SHOWN ALONGSIDE OCR ESTIMAES FROM CONSOLIDATION TESTING.
- 4. WATER TABLE ASSUMED TO BE AT THE GROUND SURFACE.
- 5. DESIGN PROFILES SHOWN ABOVE CANNOT FULLY ANTICIPATEOR ACCOUNT FOR ALL PARAMETERS WHICH MAY INFLUENCE SELECTION OF DESIGN VALUES FOR A SPECIFIC ANALYSIS. FOR THIS REASON, THE USER SHOULD CONTACT EUSTIS ENGINEERING, L.L.C. PRIOR TO USE IN ANY ANALYSIS.
- 6. C<sub>v</sub> ESTIMATES WERE COMPUTED USING CORRELATIONS SHOWN ON FIGURE 4 FROM THE NAVFAC SOIL MECHANICS DESIGN MANUAL 7.01.



SOIL DESIGN PARAMETERS - CONSOLIDATION
SOIL DESIGN REACH B - STATIONS 55+00 TO 100+00



•	DRAWN BY: P.T.D.	JOB NO.: 24384
	CHECKED BY: S.G.W.	DATE: 29 DEC 2020
	FILE NAME: 24384 CONSOLIDATION PARAMETERS SH2.GRF	ENCLOSURE 3B SHEET 3 OF 4

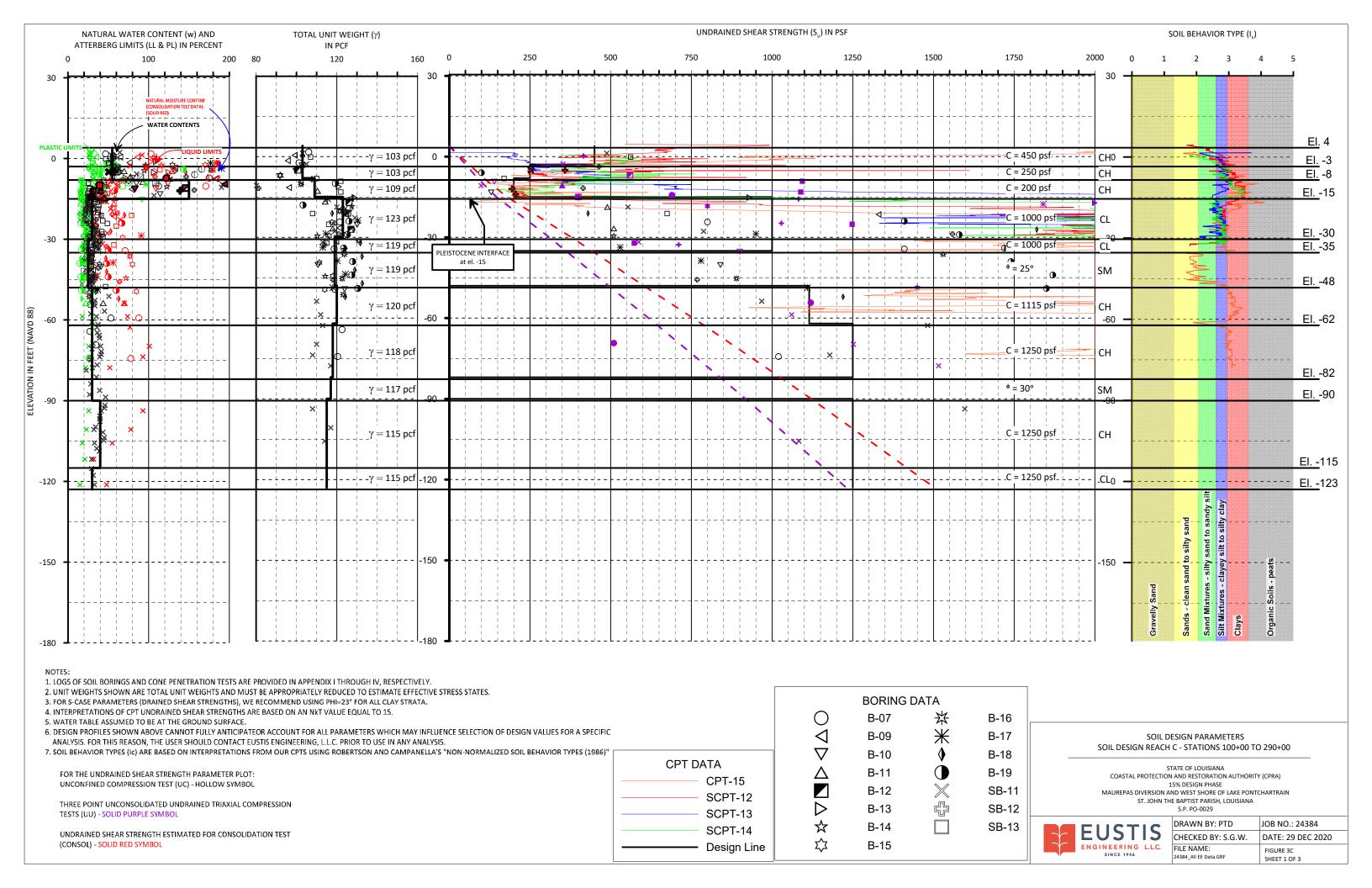


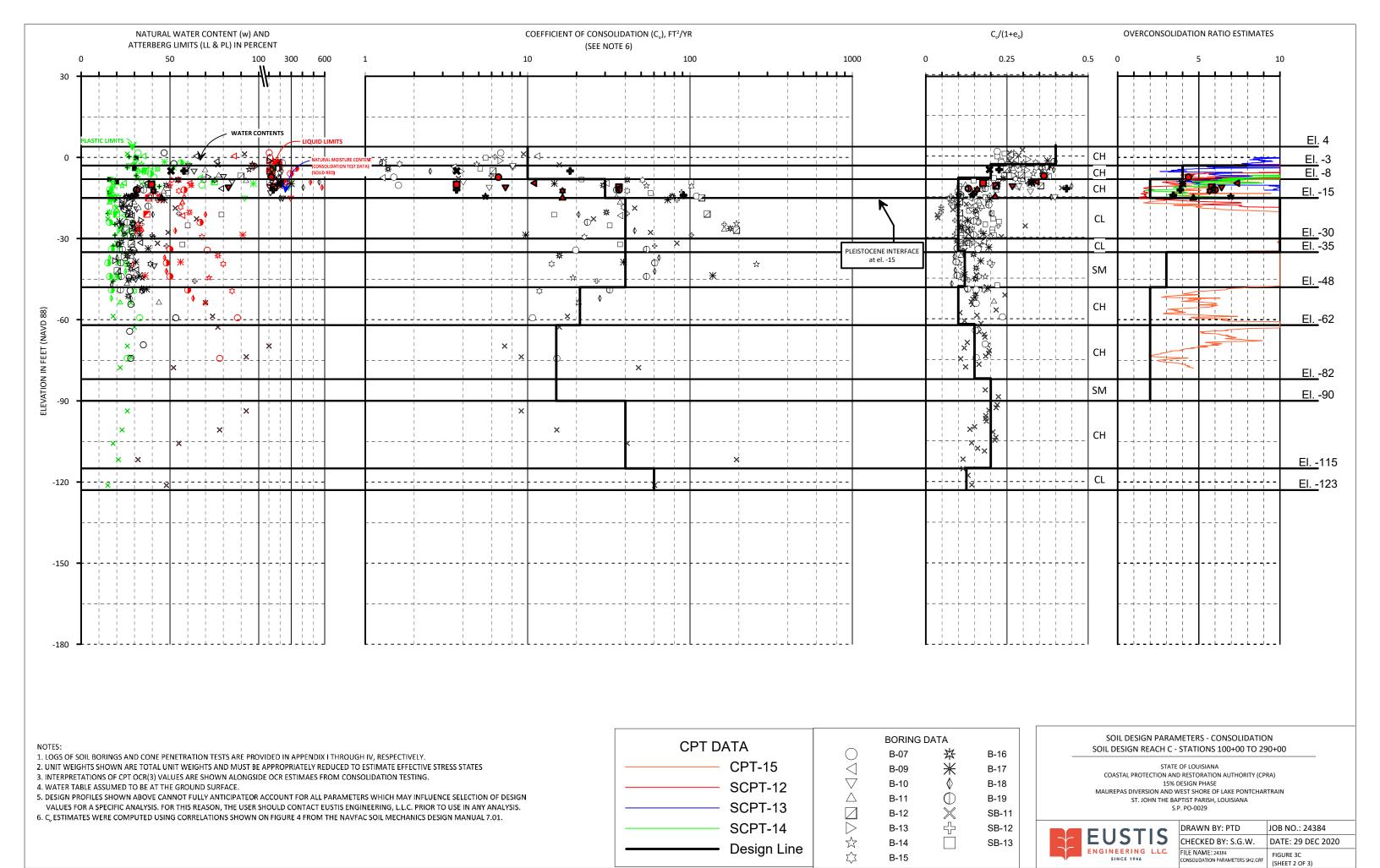
#### NOTES:

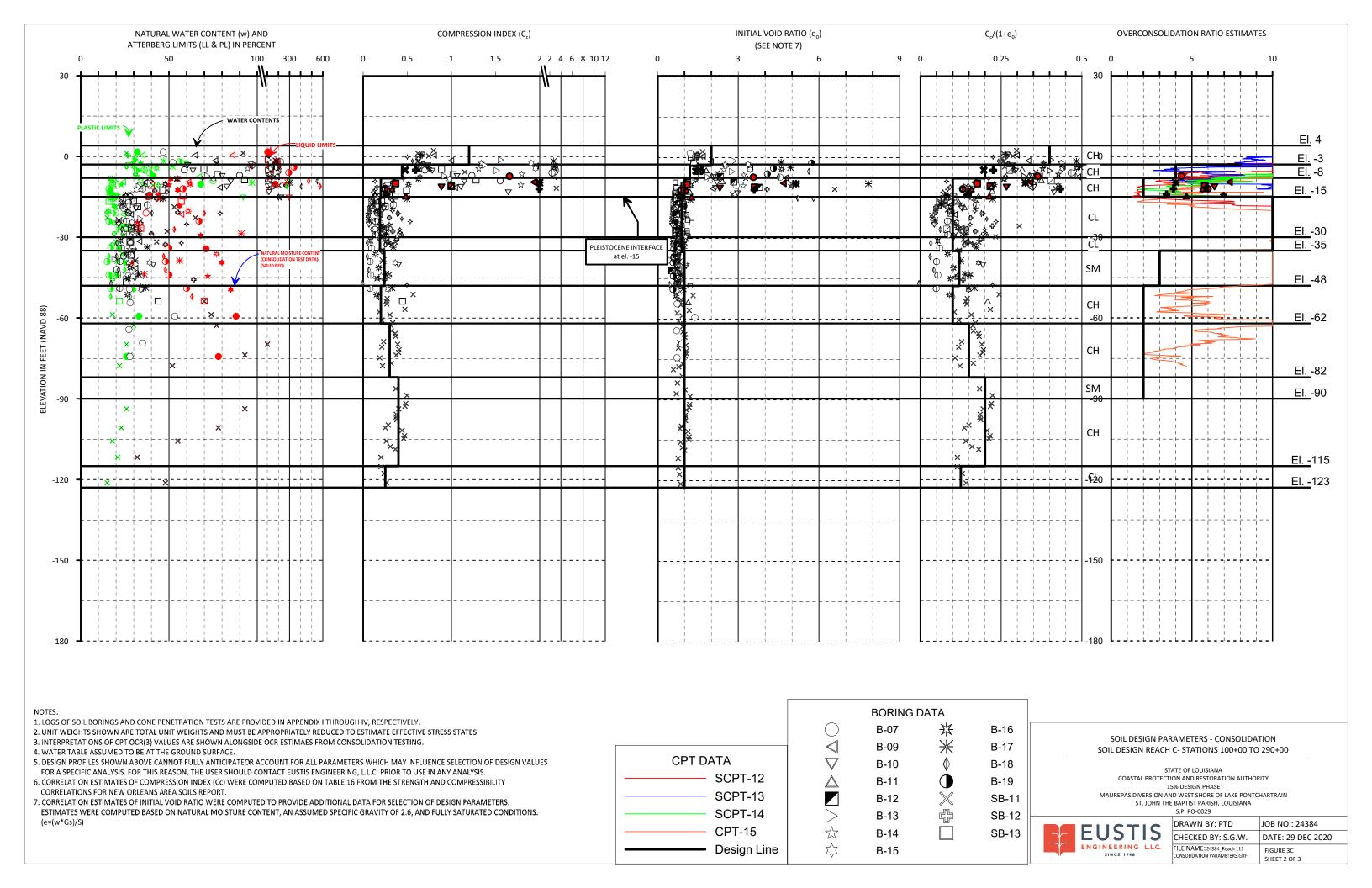
- 1. LOGS OF SOIL BORINGS AND CONE PENETRATION TESTS ARE PROVIDED IN APPENDIX I THROUGH IV.
- 2. INTERPRETATIONS OF CONE PENETRATION TEST SHEAR STRENGTH ESTIMATES ARE COMPUTED WITH  $N_{\rm kT}$  = 15, AND  $N_{\rm C}$ =20 FOR COMPARISON. AS SHOWN ON SHEET 2, WE SELECTED  $N_{\rm kT}$ =15 FOR DESIGN LINE SELECTIONS.
- 3. WATER TABLE ASSUMED TO BE AT THE GROUND SURFACE.
- 4. DESIGN PROFILES SHOWN ABOVE CANNOT FULLY ANTICIPATEOR ACCOUNT FOR ALL PARAMETERS WHICH MAY INFLUENCE SELECTION OF DESIGN VALUES FOR A SPECIFIC ANALYSIS. FOR THIS REASON, THE USER SHOULD CONTACT EUSTIS ENGINEERING, L.L.C. PRIOR TO USE IN ANY ANALYSIS.
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	DRAWN BY: P.T.D.	JOB NO.: 24384
	CHECKED BY: S.G.W.	DATE: 29 DEC 2020
- 1	FILE NAME: 24384_REACH 112 CPT CORRELATION.GRF	FIGURE 3 (SHEET 4 OF 4)







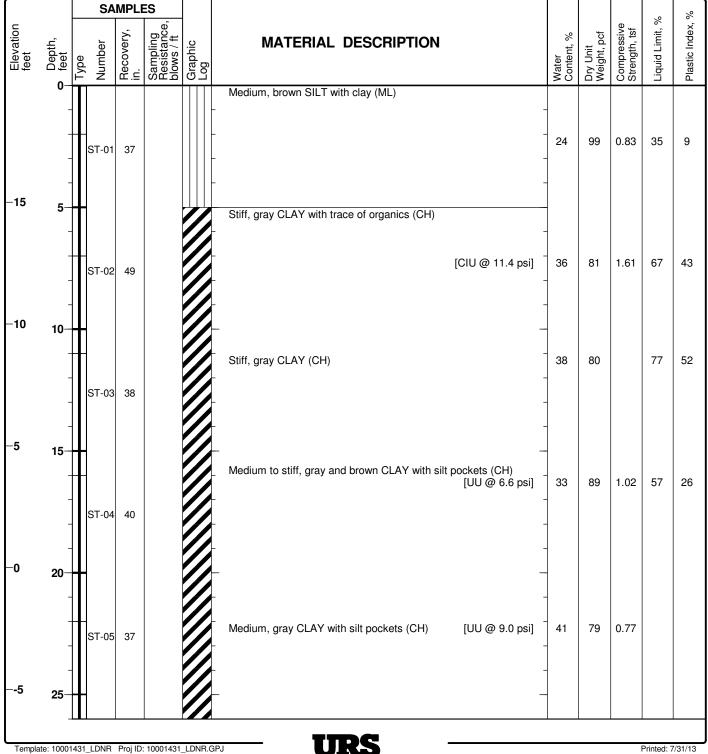
# APPENDIX I EXISTING GEOTECHNICAL BORINGS

## SOIL BORING LOG AND CPT LOG SHEETS



#### Log of Boring B-01A

Date(s)	Logged	K. Gunderson	Checked
Drilled 8/27/07 - 9/1/07	By		By M. Shewalla
Drilling	Drill Bit	6" Wing Bit	Total Depth
Method Rotary Wash 0'-125'	Size/Type		Drilled (feet) 125.0
Drill Rig	Drilling	FUGRO	Sampler
Type CME 750 Buggy	Contractor		Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Automatic	Approximate Surface Elevation 19.79 '
Location N-564823.73, E3500005.11			Borehole Backfill Grout full depth



## Log of Boring B-01A

	SAMPLES		ES					_	<u>%</u>	
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	-	ST-06	45		Medium, gray CLAY with silt pockets (CH) [UU @ 11.0 psi]	38	81	0.79	87	50
<b>−-10</b>	30-				- Medium, gray CLAY with silt pockets and trace of shells (CH)					
<b>−-15</b>	-	ST-07	40		[UU @ 13.0 psi]	50	70	0.66		
-13	35 <del></del> - -	ST-08	47		Medium to stiff, gray, dark gray, black and brown CLAY with organics (OH)	-				
20	40-	31-00	47		[UU @ 15.4 psi] - - -	114	41	1.09	222	142
	- - -	ST-09	49		Gray SILTY SAND with trace of clay (SM)  [35.6% passing #200 sieve]	30				
<b>−-25</b>	- 45				Medium to stiff, gray CLAY with shells (CH)	_				
	-	ST-10	48		[UU @ 19.0 psi] - -	53	69	0.99	97	60
- <b>-30</b>	50 <b>−</b>									
	- - -	ST-11	37		Stiff, light gray CLAY with trace of silt (CH)	26	97	1.62		
<b>⊢-3</b> 5	55 <del>-</del>					-				
Templa	ite: 10001	431_ LDNR	Proj ID	: 10001431_LDNR	GPJ URS —				Printed:	7/31/13

## Log of Boring B-01A

Sheet 3 of 5

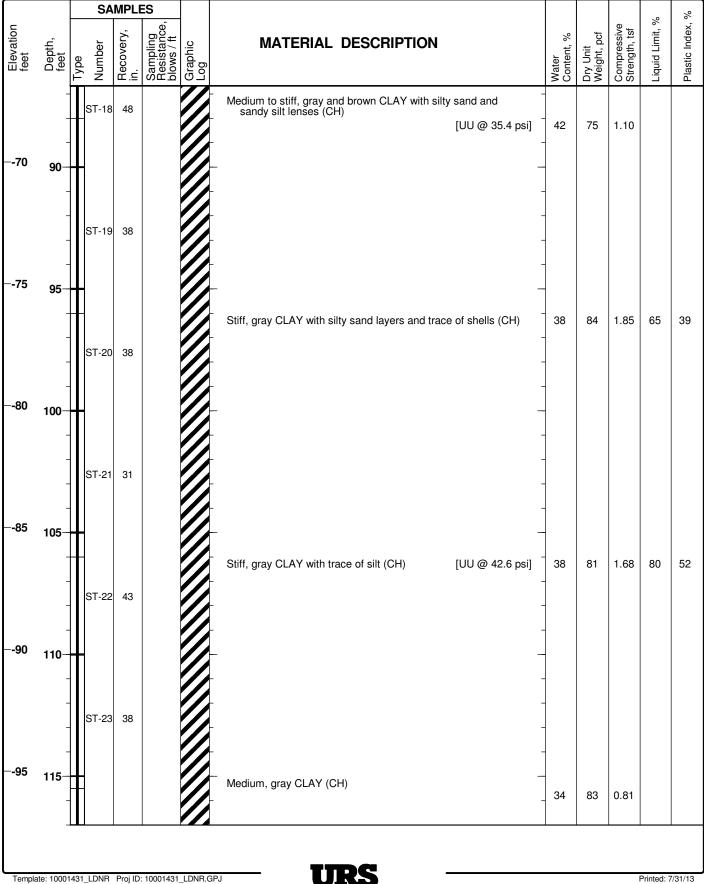
		SA	MPLI							%	%,
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	-	ST-12	44			Very stiff, light gray and greenish gray CLAY with silt (CH) [UU @ 23.4 psi]	21	108	3.10	56	33
<b>40</b>	<b>60</b> -					- -	_				
	-	ST-13	46			Stiff, tan, light gray and brown CLAY with concretions (CH)	30	91	1.17		
<b>−-45</b>	65 -					Soft light gray red and brown CLAV with silt pockets streaks	35	88	0.48	61	34
50	- - <b>70</b>	ST-14	48			Soft, light gray, red and brown CLAY with silt pockets, streaks and layers (CH)			0.40	01	04
	-	ST-15	48			Stiff to very stiff, light gray and brown CLAY (CH)	-				
- <b>-5</b> 5	- 75—					[UU @ 26.9 psi]	32	92	2.03	71	44
	-	ST-16	48				-				
60	80- -						_				
	-	ST-17	47			Medium, red, brown, tan and light gray SILTY CLAY with sand layers and lenses (CL)	38	81	0.91		
<b>−-65</b>	85— -					Medium to stiff, gray and brown CLAY (CH)	-				
Town	nto: 10004	431_LDNR	Droi In	. 10001401	I DND C	URS —	•			Printed:	7/21/10

**Project: LDNR Lake Maurepas Project Location: Lake Maurepas** 

Project Number: 10001431.30001

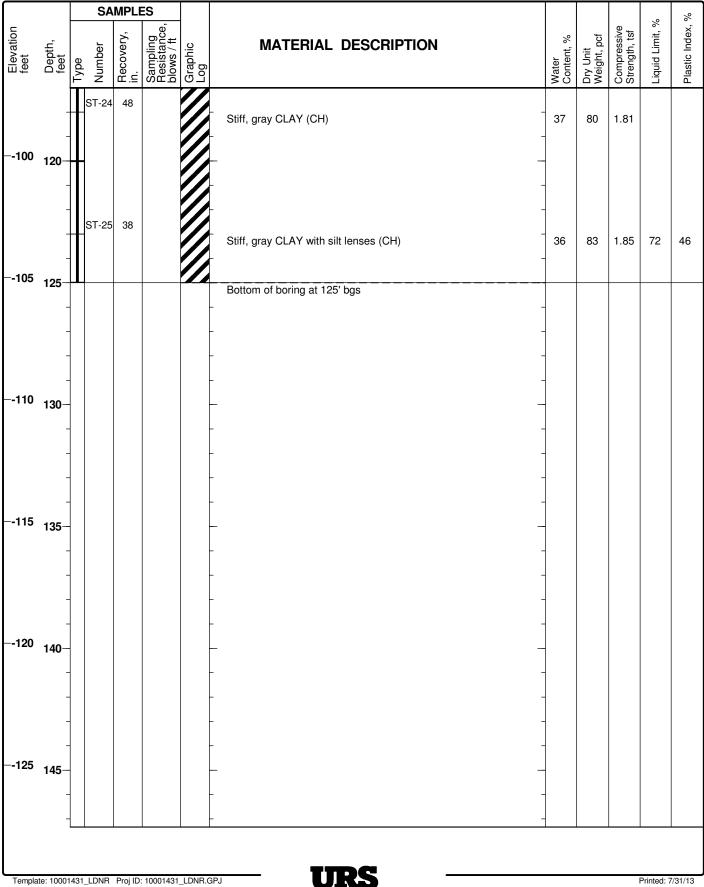
#### Log of Boring B-01A

Sheet 4 of 5



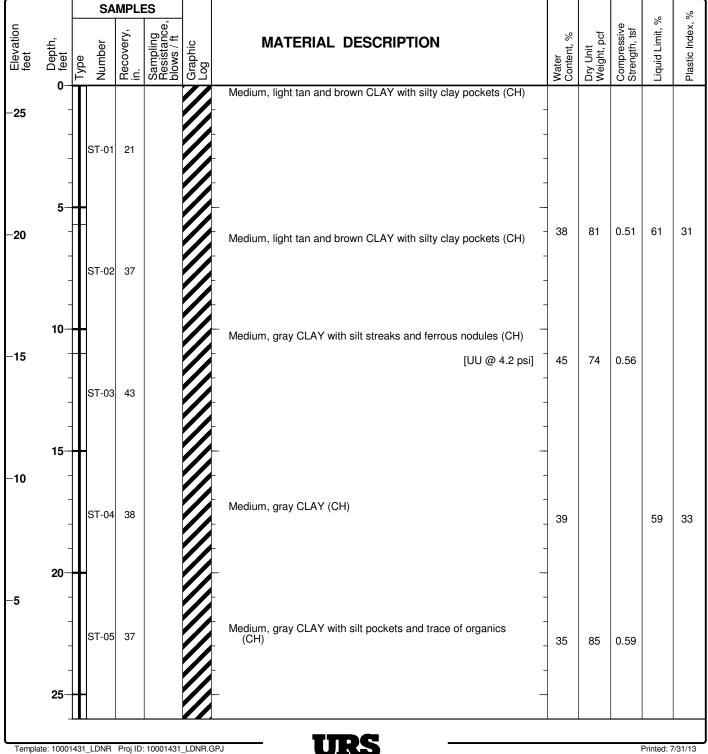
#### Log of Boring B-01A

Sheet 5 of 5



## Log of Boring B-02A

Date(s) Drilled 9/12/07 - 9/15/07	Logged By	K. Gunderson	Checked By M. Shewalla
Drilling Method Rotary Wash 0'-125'	Drill Bit Size/Type	6" Wing Bit	Total Depth Drilled (feet) 150.0
Drill Rig Type CME 750 Buggy	Drilling Contractor	FUGRO	Sampler Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Automatic	Approximate Surface Elevation 26.13 '
Location N-565124.36, E-3500062.41			Borehole Backfill Grout full depth



## Log of Boring B-02A

		SA	MPLI	ES							\ <sub>0</sub>
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
-0	-	ST-06	36			Stiff, gray and light gray CLAY with trace of silt (CH) [UU @ 11.0 psi]	31	89	1.12	58	34
5	30-	ST-07	39			Medium, light gray and brown CLAY with silt and ferrous nodules (CH)	74	65	0.78		
- <b>-1</b> 0	<b>35</b> —	ST-08	37			Stiff to very stiff, light gray and brown CLAY with silt pockets (CH) [CIU @ 15.4 psi]	37	86	2.05	58	36
<b>15</b>	40-	ST-09	49			Medium, gray CLAY with silt pockets and trace of organics (CH)	43	74	0.58		
20	<b>45</b> —	ST-10	37			Medium, gray CLAY with organic pockets (CH)  [UU @ 19.0 psi]	50	72	0.82		
<b>25</b>	<b>50</b> —	ST-11	38								
30	55-					Stiff, gray, dark gray and light gray CLAY with organic pockets (CH)  Medium, gray SILTY CLAY with trace of shells (CL)	49 35	78 69	1.07	65 47	23
	ate: 10001	431_LDNR	Proj ID	: 10001431_	LDNR.GI	URS —		•	,	Printed:	7/31/13

## Log of Boring B-02A

Sheet 3 of 6

	SAMPLES											%
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	-						Medium, gray SILTY CLAY (CL)	-				
	60-	s	T-12	46			Medium, gray CLAY with trace of shells (CH)	55	69	0.96	90	57
- <b>-3</b> 5	- - -	S	T-13	47				27	97	0.90		
- <b>-40</b>	65	S	iT-14	45			Very stiff, light gray and greenish gray SILTY CLAY with calcareous nodules (CL)					
<b>4</b> 5	70- - - -	-	:T-15	45			[UU @ 27.4 psi]	22	107	3.45	49	30
<b>−-50</b>	75- - -		T-16	48			Medium, tan and light gray SILTY CLAY with clayey silt pockets (CL)	28	100	0.97	35	14
55	80-	S	T-17	47			Medium, brownish tan and light gray SILTY CLAY with trace of organics (CL) [UU @ 33.0 psi]	29	97	0.70	43	22
<b>60</b>	85- -						- - -	-				
Templa	ite: 10001	1431_L	.DNR	Proj ID	: 10001431 <sub>_</sub>	_LDNR.G	TURS -			ļ	Printed:	7/31/13

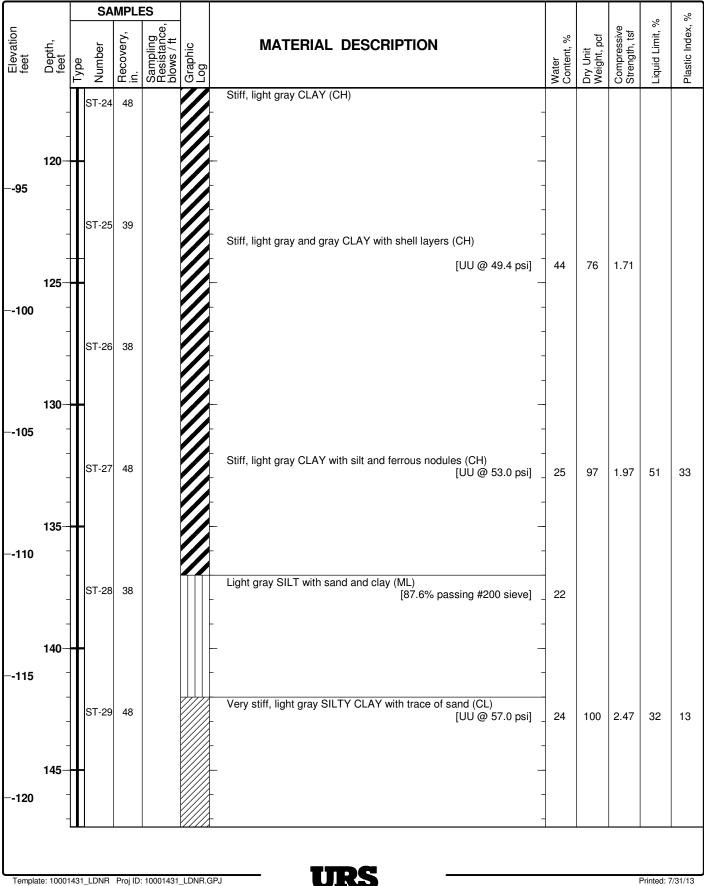
## Log of Boring B-02A

Sheet 4 of 6

		SA	AMPL							_	%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	-	ST-18	3 48			Medium, brownish tan and light gray SILTY CLAY (CL)					
	_					Very stiff, tan and light gray CLAY with calcerous nodules (CH)	42	78	2.08	90	58
	90-					- -					
<b>−-65</b>	_					-	_				
	-					-	-				
	-	ST-19	47			Medium, light gray SILTY CLAY (CL) [UU @ 37.4 psi]	37	80	0.82	45	20
	-					-					
	95-					-					
<b>⊢-70</b>	-										
	-	ST-20	47			-	-				
	-										
	100-					_	_				
<b>−-75</b>	-										
	_	ST-2	1 47			· -					
	-					-					
	105-	$\mathbb{H}$				_	_				
80	-					-	_				
	-	ST-22	2 38			Stiff, light gray CLAY (CH) [UU @ 43.0 psi]	35	88	1.53	65	42
	_					- [2.5 C .314 bal]					
	110-	Щ				_	-				
<b>−-85</b>	-					-					
	-	ST-20	3 47			-					
	-		7			-	_				
	115-	$\coprod$				_	46	73	1.84	92	66
90	- 115					ONE Park was OLAY (OLD)					
	-					Stiff, light gray CLAY (CH)					
						_					
Templa	ate: 10001	1431_LDNR	Proj ID	: 10001431_	LDNR.G	URS —			-	Printed: 7	7/31/13

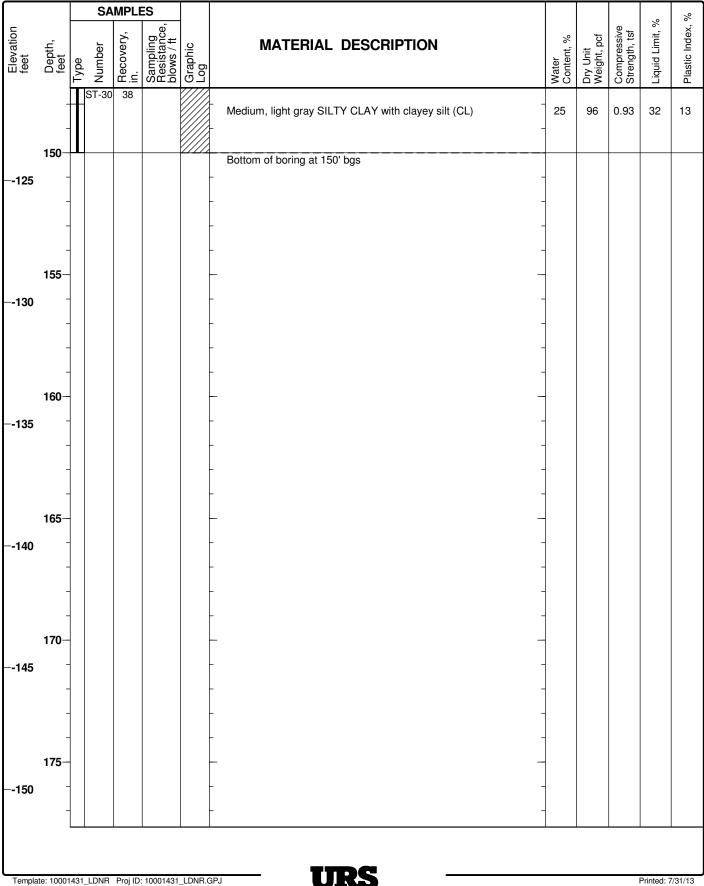
## Log of Boring B-02A

Sheet 5 of 6



## Log of Boring B-02A

Sheet 6 of 6

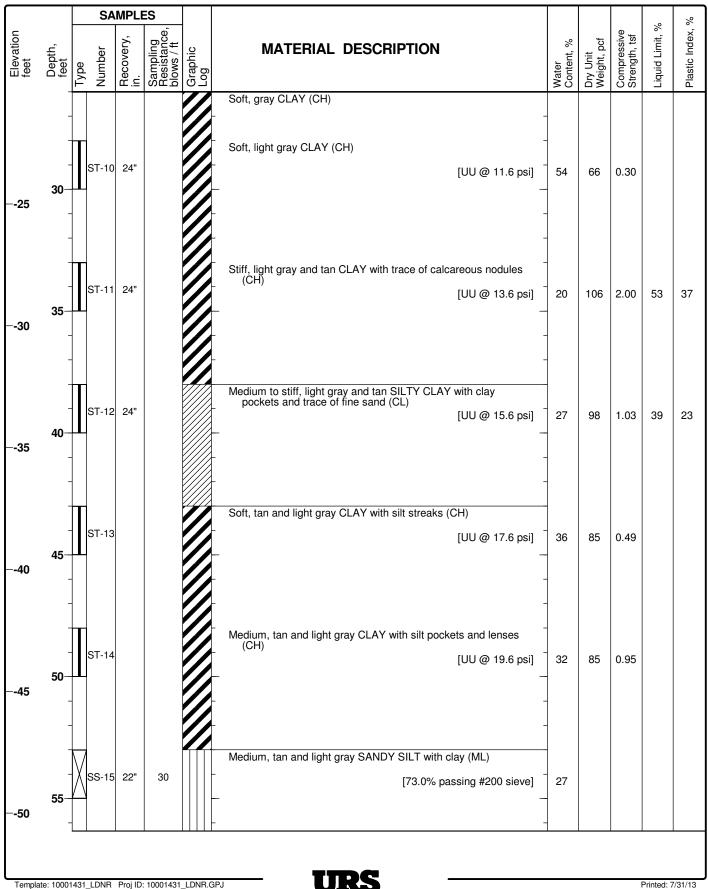


## Log of Boring B-05

Date(s)	Logged	J. Pratt	Checked
Drilled 8/28/07	By		By M. Shewalla
Drilling	Drill Bit	3 7/8" Wing Bit	Total Depth
Method Rotary Mud	Size/Type		Drilled (feet) <b>80.0</b>
Drill Rig	Drilling	SESI	Sampler
Type <b>BK 66</b>	Contractor		Type(s) Shelby Tube/Split Barrel
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Automatic	Approximate Surface Elevation 5.60 '
Location N-567296.62, E-3501215.7			Borehole Backfill Grout full depth

	SAMPLES		ES						. 0	%	
Elevation feet		Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
-5	0-	ST-01	24"			Medium to stiff, tan and light gray CLAY with ferrous and calcareous nodules (CH)					
	-	ST-02	13"			- -	31	86	1.05	73	47
-0	5-	ST-03	14"			- -	-				
	-	ST-04									
<b>−-5</b>	10	ST-05	16"			- - -					
	-	ST-06	20"			Medium, gray and greenish gray CLAY with trace of organics (CH)	55	69	0.64	106	68
	+	П				Medium, gray CLAY with organic pockets (CH)					
10	15-	ST-07	21"			[CIU @ 3.0 psi] -	80	54	0.52	122	86
	-	T   ST-08				Soft, gray CLAY with silt pockets and trace of organics (CH)	52	71	0.29		
<b>−-15</b>	20	1				[UU @ 6.8 psi]		, ,	0.20		
	- - -	T ST-09	18"			- -	-				
- <b>-20</b>	25	<u>I</u>				_	-				
Templa	te: 10001	431_LDNR	Proj ID	: 10001431	LDNR.G	URS —				Printed: 7	7/31/13

#### **Log of Boring B-05**



## Log of Boring B-05

Sheet 3 of 3

			SAI	MPLE	S							, o
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	_						Medium SILT with sand and clay (ML)					
	-						Stiff, tan and light gray SILTY CLAY with silt lenses (CL)					
	-	ST	-16				UU @ 23.6 psi]	31	91	1.53	43	23
	60-							-		1.00		
<b>−-55</b>	-	$\left  \right\rangle \left _{\mathrm{SS}}$	6-17		19							
	-											
	-						Medium, gray CLAY with sandy silt lenses (CH)					
	-	ST	-18				[UU @ 25.6 psi]	46	78	0.63	57	36
-60	65-						_	1				
	-							-				
	-						-	-				
	-						Medium, gray and light gray CLAY with silt streaks and calcareous nodules (CH)					
		ST	<sup>-</sup> -19					26	93	0.84		
<b>−-65</b>	70-											
	-						<u>.</u>					
	_											
	_		- 00				Medium to stiff, light gray CLAY with silt streaks and pockets (CH)					
	75-		-20				[UU @ 29.6 psi]	32	86	1.06	75	56
<b>70</b>	-						-					
	_											
	_						Madium light group CLAV with ailt poolsets and strooks (CLI)					
	_	ST	-21				Medium, light gray CLAY with silt pockets and streaks (CH)  [UU @ 31.6 psi]	30	87	0.90	78	59
	80-						Bottom of boring at 80' bgs	- 50	07	0.50	70	33
<b>−-75</b>	-											
	-											
	-							1				
	-						-	1				
80	85-						-	1				
<del>0</del> 0	-						-	_				
							URS -					
Templa	ite: 10001	1431_LD	NR	Proj ID:	10001431	_LDNR.G	iPJ U T				Printed:	7/31/13

## Log of Boring B-06

Date(s)	Logged	J. Pratt	Checked
Drilled 8/29/07	By		By M. Shewalla
Drilling	Drill Bit	3 7/8" Wing Bit	Total Depth
Method Rotary Mud	Size/Type		Drilled (feet) <b>80.0</b>
Drill Rig	Drilling	SESI	Sampler
Type <b>BK 66</b>	Contractor		Type(s) Shelby Tube/Split Barrel
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Automatic	Approximate Surface Elevation 3.20 '
Location N-572375.66, E-3504311.17	·		Borehole Backfill Grout full depth

		SA	MPLE	ES						. 0	%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
		ST-01	22"								
-0	-	ST-02	24"			Soft, gray and brown CLAY (CH)	53	68	0.29	99	61
	5-	ST-03	18"			- ·					
<b>−-</b> 5	-	ST-04	16"			Soft, gray and greenish gray CLAY with silt pockets (CH)	46	79	0.42	53	32
	10	ST-05	8"								
	-	ST-06	24"			Very soft, gray CLAY with organic pockets and organic CLAY (CH) to (OH) [UU @ 4.8 psi]	275	22	0.15	179	127
<b>−-10</b>	15-					Soft, gray and dark gray organic CLAY (OH)					
	13	ST-07	24"			[UU @ 6.0 psi]	118	35	0.32		
<b>−-1</b> 5	1	T ST-08				Very soft, gray CLAY with organics (CH)	55	60	0.19		
	20	1				[UU @ 7.6 psi] - -	55	60	0.19		
- <b>-20</b>	-	П				Soft, gray CLAY with silt pockets (CL)					
	25	ST-09	20"			[UU @ 9.6 psi] -	37	84	0.33	37	19
Tomnia	to: 10001:	121 I DND	Proi ID	: 10001431	L DNIP C'	URS -				Printed: 7	7/21/12

## **Log of Boring B-06**

Solid   Soli			SA	MPLI	ES							%
Soft, gray SiLTY CLAY (CL)  Very stiff, light gray and tan CLAY with trace of silt pockets [CH] [UU @ 11.6 ps] 24 102 2.19  Silff, brown, tan and light gray CLAY with concretions (CH) [UU @ 13.6 ps] 37 84 1.62 83 59  Soft, tan, brown and light gray CLAY with concretions (CH)  Soft, tan, brown and light gray CLAY with concretions (CH)  Soft, tan, brown and light gray CLAY with concretions (CH)  Soft, tan, brown and light gray CLAY with concretions (CH)  Soft, tan, brown and light gray CLAY with concretions (CH)  Soft, tan, brown and light gray CLAY with sandy silt lenses and pockets (CH)  Soft, tan, brown and light gray CLAY with sandy silt lenses and gray (ML)  Soft, tan, brown and light gray CLAY with sandy silt lenses and gray (ML)  Soft, tan, brown and light gray CLAY with sandy silt lenses and gray (ML)  Soft, tan, brown and light gray CLAY with sandy silt lenses and gray (ML)  Soft, tan, brown and light gray CLAY with sandy silt lenses and gray (ML)  Soft, tan, brown and light gray CLAY with sandy silt lenses and gray (ML)  Soft, tan, brown and light gray CLAY with sandy silt lenses and gray (ML)  Soft, tan, brown and light gray CLAY with sandy silt lenses (CH)  Soft, tan, brown, tan and light gray CLAY with silt lenses (CH)  Soft, tan, brown, tan and light gray CLAY with silt lenses (CH)  Soft gray Silf, lenses and gray CLAY with silt lenses (CH)  Soft gray Silf, lenses and gray CLAY with silt lenses (CH)  Soft gray Silf, lenses and gray CLAY with silt lenses (CH)  Soft gray Silf, lenses and gray CLAY with silt lenses (CH)	Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log		Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
30   ST-10   22"   (CH)   [UU @ 11.6 ps]   24   102   2.19    -30   ST-11   23"   Stiff, brown, tan and light gray CLAY with concretions (CH)   37   84   1.62   83   59    -35   ST-12   22"   37   Soft, tan, brown and light gray CLAY with concretions (CH)   37   83   0.47    -40   ST-13   19"   Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)   Medium to dense, tan and light gray SANDY SILT with clay ((ML)   T9.8% passing #200 sieve)   28   28   28   28   29   29   29   29		-					Soft, gray SILTY CLAY (CL)					
30   ST-10   22"   C(CH)   [UU @ 11.6 psi]   24   102   2.19    Sitif, brown, tan and light gray CLAY with concretions (CH)   37   84   1.62   83   59    Soft, tan, brown and light gray CLAY with concretions (CH)   37   83   0.47    Soft, tan, brown and light gray CLAY with concretions (CH)   37   83   0.47    Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)   Medium to dense, tan and light gray SANDY SiLT with clay (ML)   (ML)	<b>−-25</b>	-					Very stiff, light gray and tan CLAY with trace of silt pockets	_				
-30  ST-11 23*  Stiff, brown, tan and light gray CLAY with concretions (CH)  [UU @ 13.6 psi] 37 84 1.62 83 59  ST-12 22*  Soft, tan, brown and light gray CLAY with concretions (CH)  37 83 0.47  Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)  Medium of dense, tan and light gray SANDY SILT with clay  (ML)  -45 SS-16 21* 22  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24* 16  Stiff, red, tan and light gray CLAY with silt lenses (CH)  38 9 0.78		-	ST-10	22"			(CH)	24	102	2.19		
ST-11 23"  Soft, tan, brown and light gray CLAY with concretions (CH)  ST-12 22"  Soft, tan, brown and light gray CLAY with sandy silt lenses and pockets (CH)  Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)  Medium to dense, tan and light gray SANDY SILT with clay (ML)  Fig. 83 59  Nedium, light gray and tan CLAY with sandy silt lenses and pockets (CH)  SS-14 20" 33 Fig. 84 1.62 83 59  Soft, tan, brown and light gray CLAY with sandy silt lenses and pockets (CH)  SS-15 SS-16 21" 22 Fig. 83 59  Stiff, red, tan and light gray CLAY with silt lenses (CH)  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16		30-					_					
ST-11 23"  Soft, tan, brown and light gray CLAY with concretions (CH)  ST-12 22"  Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)  Medium to dense, tan and light gray SANDY SILT with clay (ML)  Fig. 83 59  No.78  SS-14 20" 33  SS-16 21" 22  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16  Stiff, red, tan and light gray CLAY with silt lenses (CH)  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16		-						_				
ST-11 23"  Soft, tan, brown and light gray CLAY with concretions (CH)  ST-12 22"  Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)  Medium to dense, tan and light gray SANDY SILT with clay (ML)  Fig. 83 59  No.78  SS-14 20" 33  SS-16 21" 22  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16  Stiff, red, tan and light gray CLAY with silt lenses (CH)  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16												
35   Soft, tan, brown and light gray CLAY with concretions (CH)   37   83   0.47     37   83   0.47     38   38   38   38   38   38   38	<b>⊢-30</b>	_	ST-11	23"								
To stiff, red, tan and light gray CLAY with silt lenses (CH)  ST-12 22"  Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)  Medium to dense, tan and light gray SANDY SILT with clay  (ML)  SS-14 20" 33  [79.8% passing #200 sieve] 28  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16		35-		23			[UU @ 13.6 psi] -	37	84	1.62	83	59
-40  -40  -41  -42  -43  -44  -45  -45  -45  -45  -45  -45		-					-	-				
-40  -40  -41  -42  -43  -44  -45  -45  -45  -45  -45  -45		_										
40	<b>−-35</b>	=					Soft, tan, brown and light gray CLAY with concretions (CH)	_				
-40  ST-13 19"  Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)  Medium to dense, tan and light gray SANDY SILT with clay (ML)  Fig. 88 passing #200 sieve]  SS-16 21" 22  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16  To stiff, red, tan and light gray CLAY with silt lenses (CH)  31 89 0.78  To stiff, red, tan and light gray CLAY with silt lenses (CH)  32 79 50		-	ST-12	22"			-	37	83	0.47		
Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)  Medium to dense, tan and light gray SANDY SILT with clay (ML)  SS-14 20" 33  [79.8% passing #200 sieve] 28  -45  50- SS-16 21" 22  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16		40-					-					
Medium, light gray and tan CLAY with sandy silt lenses and pockets (CH)  Medium to dense, tan and light gray SANDY SILT with clay (ML)  SS-14 20" 33  [79.8% passing #200 sieve] 28  -45  50- SS-16 21" 22  Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16		-										
45- SS-14 20" 33   Medium to dense, tan and light gray SANDY SILT with clay [79.8% passing #200 sieve] 28   ST-15   SS-16 21" 22   SS-16 21" 22   SS-17 24" 16   Stiff, red, tan and light gray CLAY with silt lenses (CH)   39   79   50								31	89	0.78		
45— SS-14 20" 33 [79.8% passing #200 sieve] 28  -45	<b>40</b>	_	ST-13	19"								
-45 T ST-15 SS-16 21" 22 Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16 39 79 50		45-					Medium to dense, tan and light gray SANDY SIL1 with clay (ML)  —					
50— SS-16 21" 22 — — — — — — — — — — — — — — — — — —		-	SS-14	20"	33		[79.8% passing #200 sieve]	28				
50— SS-16 21" 22 — — — — — — — — — — — — — — — — — —		_					-					
50 SS-16 21" 22 Stiff, red, tan and light gray CLAY with silt lenses (CH)  SS-17 24" 16 79 50	<b>45</b>	-	T et 1				-	-				
Stiff, red, tan and light gray CLAY with silt lenses (CH)  Stiff, red, tan and light gray CLAY with silt lenses (CH)		-						1				
SS-17 24" 16 79 50		50-	SS-16	21"	22		-					
SS-17 24" 16 79 50		-					-	1				
SS-17 24" 16 79 50												
	<b>⊢-50</b>	_	\\\_{88-1}	7 24"	16		Stiff, red, tan and light gray CLAY with silt lenses (CH)					
		55-					_	39			79	50
		-										
Template: 10001431_LDNR	Temple	ate: 10001	431 I DNP	Proi ID	: 10001431	LDNB G					Printed:	7/31/13

## Log of Boring B-06

Sheet 3 of 3

		SA	MPLI	ES							%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	-					Stiff, CLAY (CH)					
- <b>-</b> 55	60	ST-18	25"			Stiff, light gray and tan CLAY with sandy silt streaks (CH)	27	97	1.51	56	37
- <b>-60</b>	-	ST-19	23"			Stiff, light gray CLAY with trace of silt pockets (CH)  [UU @ 25.6 psi]	31	92	1.49		
<b>65</b>	65	T				Medium to stiff, gray CLAY with shell fragments (CH)	-				
	70 -	ST-20	22"			[UU @ 27.6 psi]	38	82	1.03	90	64
- <b>-70</b>	75	ST-21	24"			Stiff, light gray and greenish gray CLAY with trace of silt pockets (CH)  [UU @ 29.6 psi]	37	85	1.10		
<b>−-75</b>	+					Medium, light gray and greenish gray CLAY (CH)					
	-	ST-22	14"			[UU @ 31.6 psi]	35	86	0.72		
	80 -				-	Bottom of boring at 80' bgs					
- <b>-80</b>	- 85-				  -  -	-	-				
Tomple		131 I DNIP	Proi ID	: 10001431_	I DNP C	URS —				Printed:	7/21/12

## Log of Boring B-07

Date(s) Drilled 8/29/07	Logged By	J. Pratt	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 7/8" Wing Bit	Total Depth Drilled (feet) <b>80.0</b>
Drill Rig Type <b>BK 66</b>	Drilling Contractor	SESI	Sampler Type(s) Shelby Tube/Split Barrel
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Automatic	Approximate Surface Elevation 3.70 '
Location N-573523.53, E-3504179.84	·		Borehole Backfill Grout full depth

		SA	MPLI	ES						_	%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
		ST-01	16"				-				
-0	-	ST-02	14"			Medium, brown and gray CLAY with ferrous nodules (CH)  [UU @ 1.2 psi]	47	72	0.64	103	71
	5-	ST-03	20"			Soft, gray CLAY with organic pockets (CH)	_				
	-	ST-04	22"			[UU @ 2.8 psi]	52	67	0.35		
<b>⊢-5</b>	10	ST-05	23"			_	_				
	1	T				Soft, gray and brown CLAY with organic pockets (CH to OH)	-				
<b>−-10</b>		ST-06	24"			[UU @ 4.8 psi]	175	30	0.32	175	134
-10	15-	Τ				Very soft, gray and brown CLAY with organic pockets (CH to OH)					
	15	ST-07	7"			[UU @ 6.0 psi] -	112	38	0.16	171	103
<b>−-15</b>		TI				Medium, light gray SILTY CLAY with trace of fine sand and organics (CL)					
	20	ST-08	26"			[UU @ 7.6 psi] - -	29	95	0.69	39	21
20	-	ST-09	23"			Stiff, light gray, greenish gray and tan CLAY with trace of silt (CH)	25	101	1.05		
	25					[UU @ 9.6 psi] 	25	101	1.95		
Templa	ite: 10001	431 LDNR	Proj ID	: 10001431	LDNR.G	URS —				Printed:	7/31/13

## **Log of Boring B-07**

			SA	MPLE	ES						. 0	<b>%</b>
Elevation feet	Depth, feet	Туре	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	_						Stiff, CLAY (CH)					
- <b>-2</b> 5	30-		ST-10	24"			Medium, tan, brown and light gray SILTY CLAY with clayey silt with sand layers (CL)	29	94	0.80		
	-	_										
<b>30</b>	35-	-	ST-11	24"		-	Stiff, tan and light gray SANDY SILT with clay pockets (ML)  [UU @ 13.6 psi]	29	96	1.29		
	-	-				-						
<b>−-35</b>	-		ST-12	22"			Stiff, light gray and brown CLAY with sandy silt pockets (CH)	29	92	1.41	71	49
	40-						Medium brown, tan and light gray SANDY SILT with clay (ML)					
	-		SS-13	14"	24		[90.6% passing #200 sieve]	27				
- <b>-40</b>	- - 45		ST-14			-	_					
	<b>43</b> -		SS-15	14"	14	-						
<b>−-4</b> 5	-	M					Medium to dense, tan SILTY SAND or SANDY SILT with trace of clay (ML)					
	50-		SS-16	12"	31		[72.7% passing #200 sieve]	27				
	-	-										
- <b>-50</b>	_		эт 47	04"			Stiff, greenish gray, light gray and tan CLAY with silt pockets (CH)					
	55-		ST-17	∠4¨			[UU @ 21.6 psi] -	28	93	1.46		
	_						<u> </u>	1				
Templa	ate: 10001	1431	LDNR	Proj ID:	: 10001431	LDNR.GF	URS —			-	Printed:	7/31/13

## Log of Boring B-07

Sheet 3 of 3

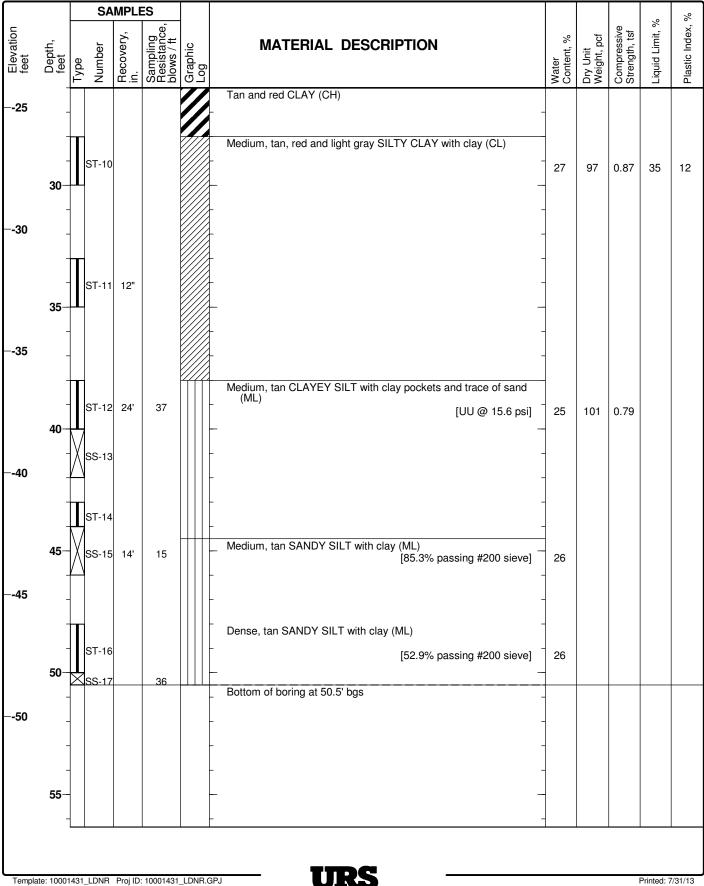
		SA	MPLI								%
Elevation feet	Depth, feet Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
- <b>-5</b> 5	60	ST-18	24"			Stiff, gray CLAY with silt pockets, streaks and shells (CH)  [UU @ 23.6 psi]	28	94	1.12		
<b>60</b>	65	ST-19	0" 5"			Medium, gray and dark gray CLAY with organics (CH)	53			88	55
<b>65</b>	70	ST-20	22"			Stiff, greenish gray, light gray and tan CLAY (CH)  [UU @ 27.6 psi]	27	96	1.75		
<b>70</b>	75	ST-21	18"			Soft to medium, greenish gray, light gray and tan CLAY with clay and silty sand pockets (CH)  [UU @ 29.6 psi]	35	87	0.51		
<b>−-75</b>	80	ST-22	22"			Medium to stiff, light gray and bluish gray CLAY with silt pockets (CH)  Bottom of boring at 80' bgs	28	94	1.02	78	56
- <b>-80</b>	85-				- - -	- -	-				
Templa	ate: 10001431	LDNR	Proi ID	: 10001431_L[	ONR.GP	URS -		l		Printed: 7	7/31/13

## Log of Boring B-08

Date(s)	Logged	J. Pratt	Checked
Drilled 8/30/07	By		By M. Shewalla
Drilling	Drill Bit	3 7/8" Wing Bit	Total Depth
Method Rotary Mud	Size/Type		Drilled (feet) <b>50.5</b>
Drill Rig	Drilling	SESI	Sampler
Type <b>BK 66</b>	Contractor		Type(s) Shelby Tube/Split Barrel
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Automatic	Approximate Surface Elevation 1.80 '
Location N-574980.94, E-3504784.22			Borehole Backfill Grout full depth

ferrous nodules (CH)  5- ST-03 7" Medium, gray and brown CLAY with organic pockets (CH)  Soft, gray CLAY with organic pockets (CH)	% '%' Plastic Index' % '95 59 59 59 59 59 59 59 59 59 59 59 59 5
ST-01 15"  ST-02 20"  Medium, gray and brown CLAY with trace of organics and ferrous nodules (CH)  To soft, gray CLAY with organic pockets (CH)  ST-04 25"  Wery soft, gray CLAY with organic pockets (CH)  ST-05 18"  Very soft, gray CLAY with organic pockets (CH)  Very soft, gray CLAY with organic and organic clay pockets (CH)	97 56
5— ST-03 7" Medium, gray and brown CLAY with organic pockets (CH)  Soft, gray CLAY with organic pockets (CH)  ST-04 25"  [UU @ 2.8 psi] 62 63 0.35  Very soft, gray CLAY with organic pockets (CH)  [UU @ 3.6 psi] 97 52 0.21  Very soft, gray CLAY with organics and organic clay pockets (CH)	97 56
Soft, gray CLAY with organic pockets (CH)  Soft, gray CLAY with organic pockets (CH)  ST-04 25"  [UU @ 2.8 psi] 62 63 0.35  Very soft, gray CLAY with organic pockets (CH)  [UU @ 3.6 psi] 97 52 0.21  Very soft, gray CLAY with organics and organic clay pockets (CH)	
ST-04 25"    ST-04 25"   Figure 2.8 psi   Figure 2.8 psi	79 43
ST-05 18" [UU @ 3.6 psi] 97 52 0.21  Very soft, gray CLAY with organics and organic clay pockets (CH)	1
Very soft, gray CLAY with organics and organic clay pockets  (CH)	
10	
Very soft, gray and brown CLAY with trace of organics (CH)	
15 ST-07 22" [UU @ 6.0 psi] 168 30 0.13	
15	
Stiff, greenish gray and tan CLAY with trace of organics (CH)  ST-08 25"  Stiff, greenish gray and tan CLAY with trace of organics (CH)  28 99 1.27	
20	
20	
Stiff, light gray, tan and red CLAY with trace of silt (CH)	
ST-09 17" 22 102 1.66	
Template: 10001431_LDNR Proj ID: 10001431_LDNR.GPJ	

#### Log of Boring B-08

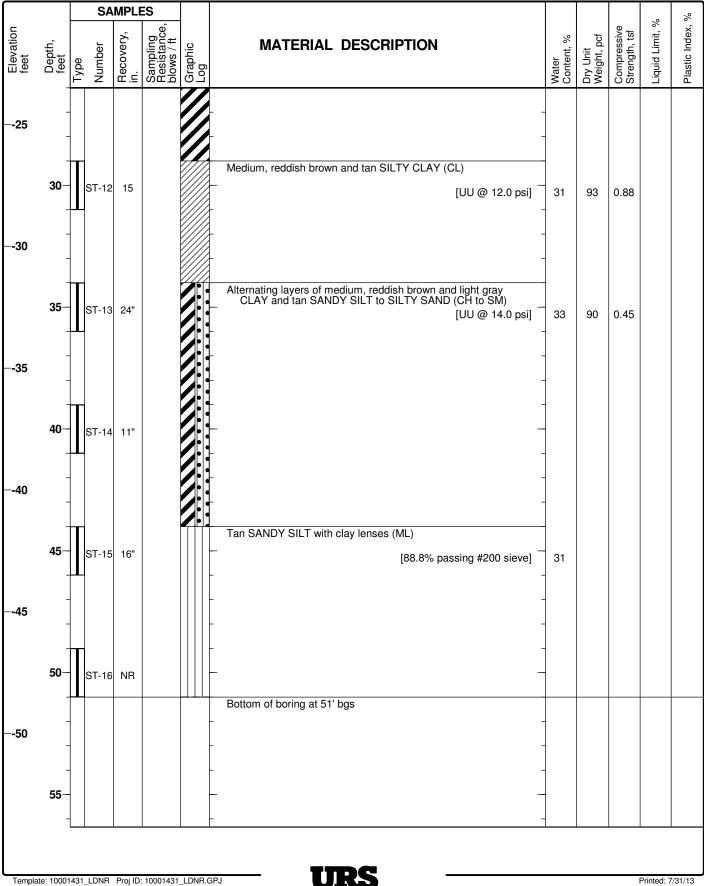


## Log of Boring B-09

Date(s) Drilled 8/28/07	Logged By	J. Murray	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 1/2" Wing Bit	Total Depth Drilled (feet) <b>51.0</b>
Drill Rig Type Air Boat	Drilling Contractor	ESS	Sampler Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation 2.50 '
Location N-576186.24, E-3504662.39			Borehole Backfill Grout full depth

	SAMPLES										<u>%</u>
Elevation feet	Depth, Feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	-	ST-01	24"			-					
-0	-	ST-02	24"			Very soft, gray and dark gray CLAY with silt pockets (CH) [UU @ 1.2 psi]	65	60	0.13	86	51
	5	ST-03	24"			Soft, gray and dark gray CLAY with organic pockets (CH) [UU @ 2.0 psi]	76	54	0.34		
	-	ST-04	24"			Very soft, gray CLAY with organic pockets (CH)	133	38	0.21	120	91
<b>⊢-</b> 5	-	H				[UU @ 2.8 psi] -	133	36	0.21	130	91
	10-	ST-05	24"			Very soft, gray and dark gray CLAY with organic clay and organic pockets (CH to OH)					
	-	ST-06	24"			[UU @ 4.4 psi]	183	27	0.11	185	118
<b>−-10</b>	=	ST-07	24"			Very soft, gray CLAY with organic clay layers and silt pockets (CH to OH)  [UU @ 5.2 psi]	37	75	0.25	61	37
	- 15	ST-08	24"			Soft, gray and brown CLAY with organic pockets and organic clay layers (CH to OH)	79	54	0.34		
	_	H				[00 @ 0.0 pol] 	, , ,	04	0.04		
<b>−-15</b>	_	ST-09	NR			Medium to stiff, greenish gray and light gray CLAY with trace of					
	20-	ST-10	24"			silt (CH) [UU @ 7.6 psi]	23	103	1.07		
	-					-					
20	-					<del>-</del> -					
	- 25	T				Stiff, tan and light gray CLAY with calcareous nodules and silty clay pockets (CH)					
	<b>2</b> 0	ST-11	20				28	97	1.33	57	37
Templa	ate: 10001	431_LDNR	Proi ID	: 10001431	LDNR.G	TRS —				Printed: 1	7/31/13

#### **Log of Boring B-09**

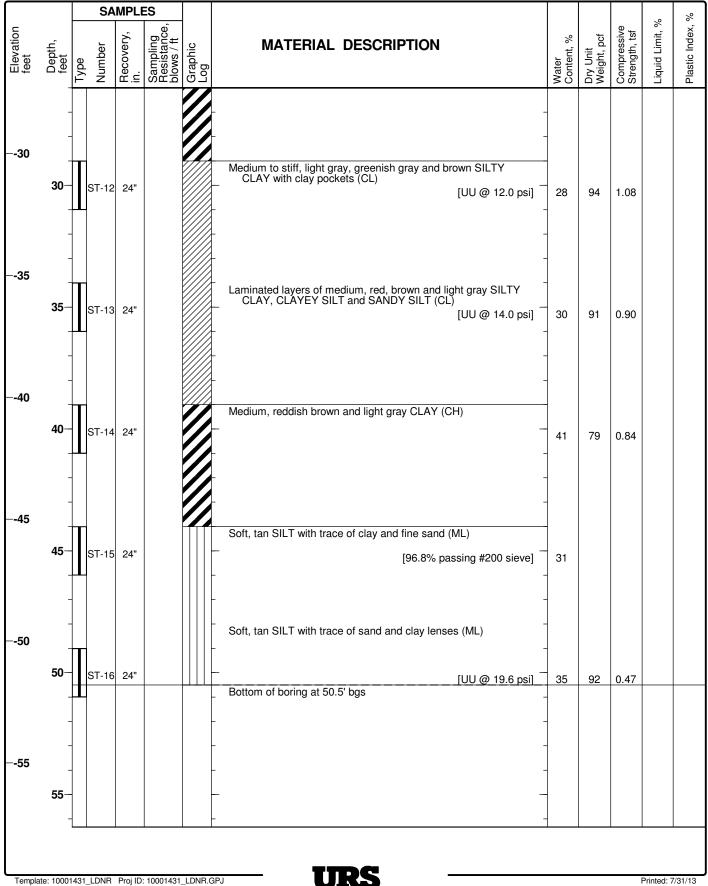


## **Log of Boring B-10**

Date(s) Drilled 8/28/07	Logged By	J. Murray	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 1/2" Wing Bit	Total Depth Drilled (feet) <b>50.5</b>
Drill Rig Type Air Boat	Drilling Contractor	ESS	Sampler Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation -1.30 '
Location N-577699.95, E-3504180.38			Borehole Backfill Grout full depth

		SA	MPLE	S							. 0	%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION		Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, '
	-	ST-01	8"				-					
<b>⊢-5</b>	-	ST-02	20"			Soft, gray CLAY with organic pockets (CH)	[UU @ 1.2 psi]	61	64	0.46	97	70
	5-	ST-03	24"			Medium, gray CLAY with organic pockets (CH)	[UU @ 2.0 psi]	64	61	0.66		
		ST-04	20"			Very soft, gray and dark gray CLAY with organic	pockets (CH)	80	51	0.19		
10	-	ST-05	24"			Very soft, gray CLAY with organic pockets (CH)	[UU @ 3.6 psi]	85	48	0.14	124	90
	10-	ST-06	24"			Very soft, gray CLAY with organic pockets (CH)	-	83			109	73
<b>⊢-15</b>	-	ST-07	24"			Very soft, gray and brown CLAY with organic po organic clay layers (CH to OH)	ckets and	114	42	0.13		
	15-	ST-08	24"			Very soft, brown and gray organic CLAY with wo	ood (OH) [UU @ 6.0 psi]	223	23	0.18	295	203
	-	ST-09	24"				-					
- <b>-20</b>	20	ST-10	20"			Soft, gray CLAY with silt pockets and trace of or Medium, light gray SILTY CLAY (CL)	ganics (CH) [UU @ 2.6 psi] [UU @ 7.6 psi]	34 26	89 97	0.39 0.63		
- <b>-25</b>	- - -					Vory etiff groonich gray and light gray CLAV	- - -					
	25-	ST-11	24"			Very stiff, greenish gray and light gray CLAY wit and calcareous nodules (CH)	ii siit pockets —	22	107	2.35		
Templa	te: 10001	431_LDNR	Proj ID:	10001431_	LDNR.G	urs -					Printed:	7/31/13

## Log of Boring B-10

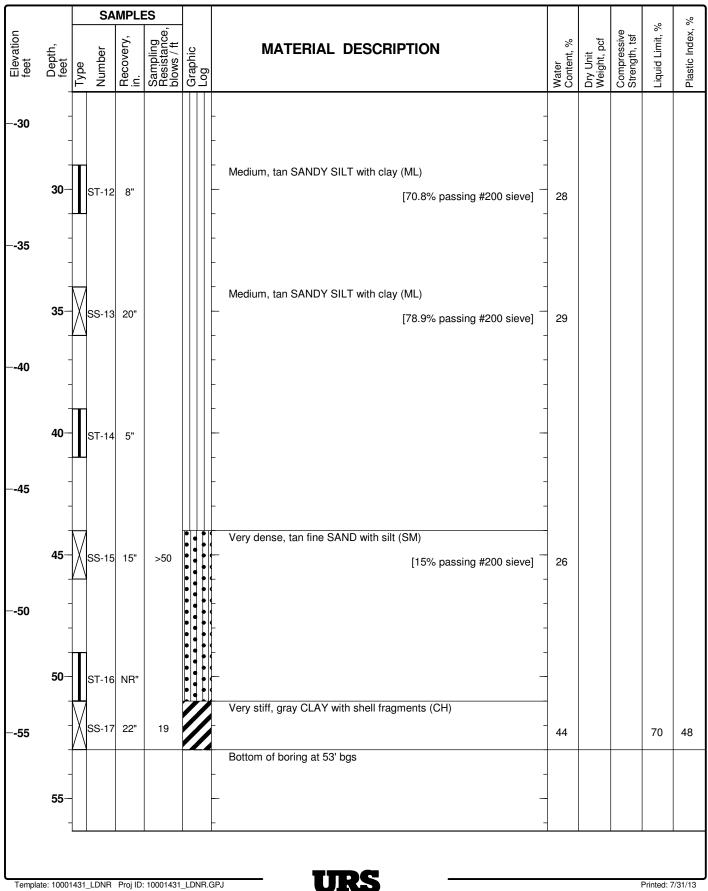


## Log of Boring B-11

Date(s) Drilled 8/29/07 - 8/31/07	Logged By	J. Murray	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 1/2" Wing Bit	Total Depth Drilled (feet) 53.0
Drill Rig Type Air Boat	Drilling Contractor	ESS	Sampler Type(s) Shelby Tube/Split Spoon
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation -2.70 '
Location N-579059.47, E-3503823.50			Borehole Backfill Grout full depth

		SA	MPLI	ES							%
Elevation feet	Depth,   feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	-	ST-01	8"			-					
<b>⊢-</b> 5	=	ST-02	20"			Soft, gray CLAY with organic pockets (CH) [UU @ 1.2 psi]	70	55	0.37	129	97
	-	Ħ				Stiff, gray CLAY with organic pockets and 3/4" vertical root	1				
	5-	ST-03	21"			 [UU @ 2.0 psi]	78	52	1.20		
						Very soft, gray CLAY with organic pockets (CH)					
<b>−-10</b>		ST-04	15"			[UU @ 2.8 psi]	93	45	0.22		
		T				Soft, gray SILTY CLAY with organic pockets (CL)					
	40	ST-05	24"			[UU @ 3.6 psi]	34	88	0.35	40	22
	10-	Ħ				Very soft, gray CLAY with trace of silt and organic pockets (CH)					
	-	ST-06	24"				46	76	0.07		
<b>−-15</b>	-	ST-07	24"			Soft, greenish gray and light gray CLAY with trace of organics (CH)	1				
						_	39			58	39
	15	07.00				Soft, greenish gray and light gray CLAY with trace of organics and silt pockets (CH)					
	19_	ST-08	24"			[UU @ 6.0 psi]	30	88	0.49	57	41
		T				Soft, greenish gray and light gray CLAY with silt pockets (CH)	1				
<b>−-20</b>		ST-09	10"			_	32	93	0.49		
	_	П				-	1				
		ST-10	24"			-	1				
	20-	•				_					
	-					-					
<b>25</b>	-					-	1				
	=					-	-				
	-	Т				Medium, tan SANDY SILT with clay pockets (ML)	_				
	25-	ST-11	15"			_	29	92	0.51	34	8
	_			1					1		1
Temple	ate: 10001	431_LDNR	Proi ID	: 10001431	LDNR.G	PJ URS —			_	Printed:	7/31/13

#### Log of Boring B-11

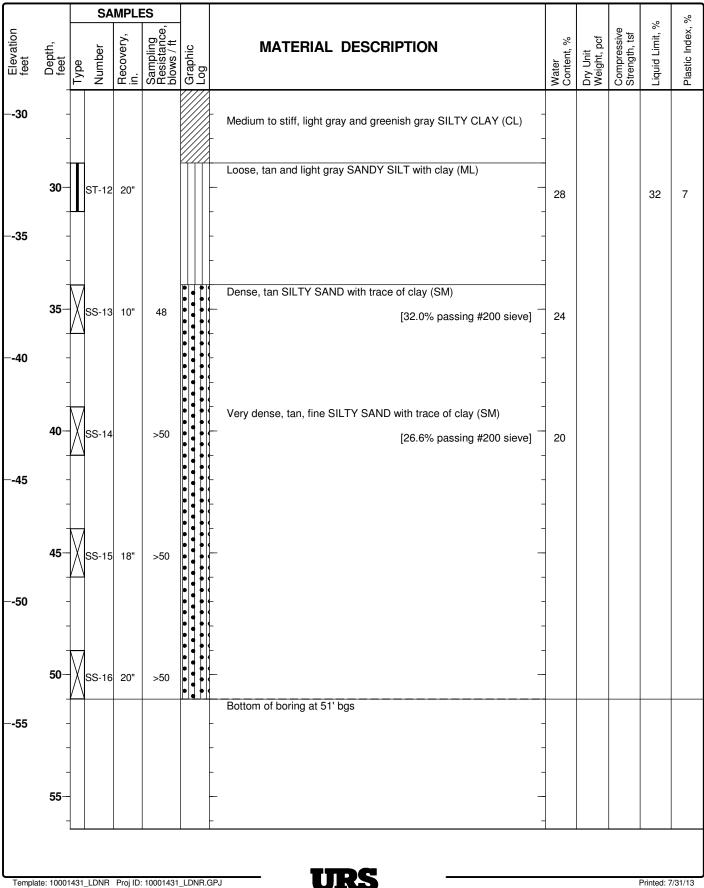


## **Log of Boring B-12**

Date(s) Drilled 8/29/07 - 9/1/07	Logged By	J. Murray	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 1/2" Wing Bit	Total Depth Drilled (feet) 51.0
Drill Rig Type Air Boat	Drilling Contractor	ESS	Sampler Type(s) Shelby Tube/Split Spoon
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation -3.00 '
Location N-580499.98, E-3503474.00			Borehole Backfill Grout full depth

ST-01   22   ST-03   24   ST-05   24   ST-06   5   ST-08   24   ST-08   25   ST-11   25   Soft to medium, gray, greenish gray and light gray SILTY CLAY with organic pockets (CL)   ST-08   S	SAMPLES								%			
ST-01   22*   Medium, light gray and greenish gray CLAY with ferrous   ST-02   24*   Medium, gray and dark gray CLAY with organic pockets (CH)   ST-03   24*   Very soft, gray and dark gray CLAY with organic pockets (CH)     UU @ 3.0 psi   90   49   0.56   Very soft, gray and dark gray CLAY with organic pockets (CH)     UU @ 2.8 psi   109   43   0.21   Soft, brown, black and gray organic CLAY with organic pockets (CH)     UU @ 3.6 psi   146   34   0.27   259   184     ST-05   24*   Soft, light gray and gray SILTY CLAY with organic pockets   UU @ 3.0 psi   28   93   0.40   Medium, greenish gray and light gray SILTY CLAY with organic pockets   UU @ 6.0 psi   27   99   0.66   38   21   ST-08   24*   Soft to medium, gray, greenish gray and light gray SILTY CLAY with organic pockets (CL)   UU @ 7.6 psi   27   99   0.49   O.56   O.49   O.49   O.56   O.49   O.49   O.56   O.49   O.49   O.49   O.56   O.49   O.49   O.49   O.56   O.49	Elevation feet		Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, 9
ST-02 24*    Medium, light gray and greenish gray CLAY with ferrous [UU @ 1.2 psi]   59   64   0.43   107   71     Medium, gray and dark gray CLAY with organic pockets (CH)   [UU @ 3.0 psi]   90   49   0.56     Very soft, gray and dark gray CLAY with organic pockets (CH)   [UU @ 2.8 psi]   109   43   0.21     ST-05 24*   ST-06 5*   [UU @ 3.6 psi]   146   34   0.27   259   184     ST-07 24*   Soft, light gray and gray SILTY CLAY with organic pockets (CL)   [UU @ 3.0 psi]   28   93   0.40     Medium, greenish gray and light gray SILTY CLAY with organic pockets (CL)   [UU @ 6.0 psi]   27   99   0.66   38   21     ST-08 24*   ST-09 5*   Soft to medium, gray, greenish gray and light gray SILTY CLAY with organic pockets (CL)   [UU @ 7.6 psi]   27   99   0.49     Medium to stiff, light gray and greenish gray SILTY CLAY (CL)   [UU @ 7.6 psi]   27   99   0.49	5	0-	ST-0				-	_				
5-   ST-03   24*		-	ST-02	2 24"			Medium, light gray and greenish gray CLAY with ferrous nodules (CH) [UU @ 1.2 psi]	59	64	0.43	107	71
Very soft, gray and dark gray CLAY with organic pockets (CH)  [UU @ 2.8 psi] 109 43 0.21  ST-05 24* [UU @ 3.6 psi] 146 34 0.27 259 184  ST-05 5* [UU @ 3.0 psi] 28 93 0.40  Medium, greenish gray and light gray SILTY CLAY with organic pockets (CL) [UU @ 6.0 psi] 27 99 0.66 38 21  Soft to medium, gray, greenish gray and light gray SILTY CLAY with organic pockets (CL) [UU @ 7.6 psi] 27 99 0.49  Medium to stiff, light gray and greenish gray SILTY CLAY (CL)		_ 1	П				Medium, gray and dark gray CLAY with organic pockets (CH)					
ST-04   24"   Soft, brown, black and gray organic CLAY with organic pockets (OH)   ST-05   24"   Soft, light gray and gray SILTY CLAY with organic pockets (CL)   ST-07   24"   ST-08   24"   ST-08   24"   ST-08   24"   ST-09   5"   ST-09   5"   Soft to medium, gray, greenish gray and light gray SILTY CLAY with organic pockets (CL)   ST-09   ST-10   24"   ST-10   ST-10		5-	ST-03	3 24"			[UU @ 3.0 psi]	90	49	0.56		
ST-05 24"   Soft, brown, black and gray organic CLAY with organic pockets (OH)   [UU @ 3.6 psi]   146   34   0.27   259   184    15		1	Ħ				Very soft, gray and dark gray CLAY with organic pockets (CH)					
ST-05 24" [UU @ 3.6 psi] 146 34 0.27 259 184  ST-06 5" ST-07 24" [UU @ 3.6 psi] 28 93 0.40  Medium, greenish gray and light gray SILTY CLAY with organic pockets (CL) [UU @ 3.0 psi] 28 93 0.40  Medium, greenish gray and light gray SILTY CLAY with organic pockets (CL) [UU @ 6.0 psi] 27 99 0.66 38 21  ST-09 5" Soft to medium, gray, greenish gray and light gray SILTY CLAY with silt pockets (CL) [UU @ 7.6 psi] 27 99 0.49  Medium to stiff, light gray and greenish gray SILTY CLAY (CL)	<b>⊢-10</b>	1	ST-04	1 24"			[UU @ 2.8 psi]	109	43	0.21		
ST-05   24"   ST-06   5"   ST-06   5"   Soft, light gray and gray SILTY CLAY with organic pockets (CL)   ST-08   24"   ST-08   24"   ST-09   5"   ST-10   24"   Soft to medium, gray, greenish gray and light gray SILTY CLAY with organic pockets (CL)   ST-10   24"   ST		1	Ħ				Soft, brown, black and gray organic CLAY with organic pockets (OH)					
ST-06 5"  Soft, light gray and gray SILTY CLAY with organic pockets (CL)  ST-07 24"  [UU @ 3.0 psi] 28 93 0.40  Medium, greenish gray and light gray SILTY CLAY with organic pockets (CL)  [UU @ 6.0 psi] 27 99 0.66 38 21  ST-09 5"  Soft to medium, gray, greenish gray and light gray SILTY CLAY with organic pockets (CL)  [UU @ 6.0 psi] 27 99 0.49  ST-10 24"  Soft to medium, gray, greenish gray and light gray SILTY CLAY with silt pockets (CL)  [UU @ 7.6 psi] 27 99 0.49  Medium to stiff, light gray and greenish gray SILTY CLAY (CL)		1	ST-05	24"			_ ` '	146	34	0.27	259	184
Str. light gray and gray SILTY CLAY with organic pockets  [UU @ 3.0 psi]	45	10-	ST-06	5 5"				-				
ST-07   24"	<b>⊢-15</b>	1	П				Soft, light gray and gray SILTY CLAY with organic pockets (CL)					
15— ST-08 24" [UU @ 6.0 psi] 27 99 0.66 38 21  ST-09 5" Soft to medium, gray, greenish gray and light gray SILTY CLAY with silt pockets (CL) [UU @ 7.6 psi] 27 99 0.49  20 Medium to stiff, light gray and greenish gray SILTY CLAY (CL)		1	ST-07	24"				28	93	0.40		
ST-08 24"  ST-09 5"  ST-10 24"  ST-10 24"  Medium to stiff, light gray and greenish gray SILTY CLAY (CL)  Medium to stiff, light gray and greenish gray SILTY CLAY (CL)		1	Ħ				Medium, greenish gray and light gray SILTY CLAY with organic pockets (CL)					
Soft to medium, gray, greenish gray and light gray SILTY CLAY with silt pockets (CL)  [UU @ 7.6 psi]  Amedium to stiff, light gray and greenish gray SILTY CLAY (CL)		15-	ST-08	3 24"				27	99	0.66	38	21
with silt pockets (CL)  [UU @ 7.6 psi] 27 99 0.49  -25  Medium to stiff, light gray and greenish gray SILTY CLAY (CL)	- <b>-20</b>	-	ST-09	5"			- -	-				
20 ST-10 24" [UU @ 7.6 psi] 27 99 0.49 25 Medium to stiff, light gray and greenish gray SILTY CLAY (CL)		1	Ħ				Soft to medium, gray, greenish gray and light gray SILTY CLAY with silt pockets (CL)					
Medium to stiff, light gray and greenish gray SILTY CLAY (CL)		20	ST-10	24"				27	99	0.49		
25	<b>−-25</b>	-					- -	-				
[UU @ 10.0 psi] 26   101   1.06   37   17		25-	T									
		25-	ST-1	15"			[UU @ 10.0 psi]	26	101	1.06	37	17
	Templa	ate: 10001	431_LDNR	Proj ID	: 10001431	LDNR.C	TRS —				Printed:	7/31/13

#### **Log of Boring B-12**

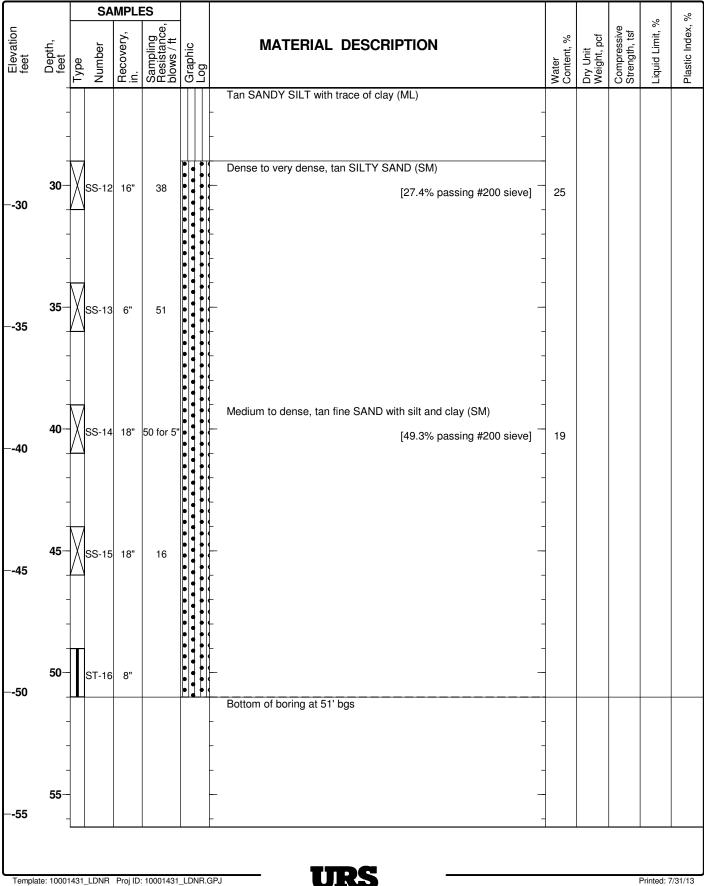


## **Log of Boring B-13**

Date(s) Drilled 8/29/07 - 8/29/07	Logged By	J. Murray	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 1/2" Wing Bit	Total Depth Drilled (feet) <b>51.0</b>
Drill Rig Type Air Boat	Drilling Contractor	ESS	Sampler Type(s) Shelby Tube/Split Spoon
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation 0.80 '
Location N-581703.87, E-3503150.12			Borehole Backfill Grout full depth

		SA	MPLI	ES							%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft Graphic	Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, '
-0	-	ST-01	24"				-				
		ST-02	24"			Soft, greenish gray and light gray CLAY with organic pockets [UU @ 1.2 psi]	109	47	0.42	103	72
	5-	ST-03	14"			Soft, gray CLAY with organic pockets (CH)  [UU @ 2.0 psi]	99	44	0.28		
<b>⊢-5</b>	+	ST-04	24"			Very soft to soft, gray CLAY with organic pockets and wood (CH)	107				
	-	ST-05	24"			Very soft, gray and dark gray CLAY with organic pockets (CH)	72			125	92
10	10	ST-06	24"			Modium groonish gray and light gray CLAV with silt pockets	-				
	-	ST-07	24"			Medium, greenish gray and light gray CLAY with silt pockets and ferrous nodules (CH)  [UU @ 5.2 psi]	33	91	0.55		
<b>−-15</b>	15-	ST-08	24"			Medium, light gray, greenish gray and tan CLAY with silt pockets and ferrous nodules (CH)	_				
	-	ST-09	24"			Stiff to very stiff, light gray, greenish gray and tan SILTY CLAY	27	101	0.93		
	20	ST-10	24"			with ferrous nodules (CL)  [UU @ 10.0 psi]	23	104	2.00		
<b>20</b>	-					Top CANDY Off Turish trace of also (All )					
- <b>-2</b> 5	25-	ST-11	24"			Tan SANDY SILT with trace of clay (ML)  [98.7% passing #200 sieve]	34				
Templa	te: 10001	431_LDNR	Proj ID	: 10001431_LDN	NR.GP	URS —				Printed:	7/31/13

#### Log of Boring B-13

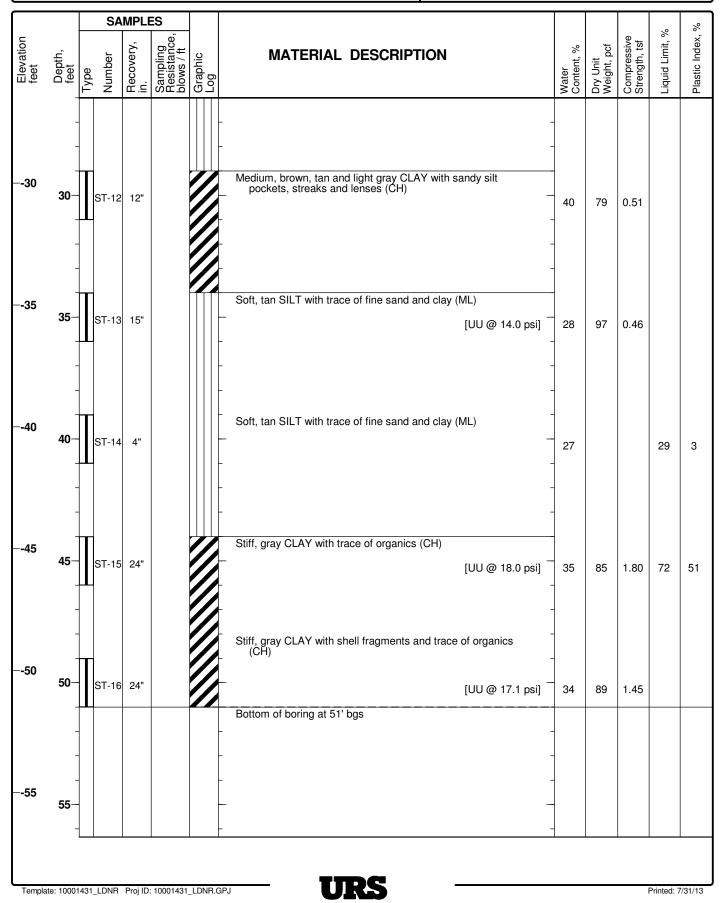


## Log of Boring B-14

Date(s) Drilled 8/30/07	Logged By	J. Murray	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 1/2" Wing Bit	Total Depth Drilled (feet) 51.0
Drill Rig Type Air Boat	Drilling Contractor	ESS	Sampler Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation -0.50 '
Location N-583283.70, E-3502344.35			Borehole Backfill Grout full depth

		SA	MPLI	ES						. 0	%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
		ST-01	24"								
		ST-02	24"			Soft, gray and dark gray CLAY with organic pockets (CH) Disturbed Sample [UU @ 1.2 psi]	96	45	0.28	116	82
<b>−-</b> 5	5-	ST-03	20"			Soft, gray and dark gray CLAY with organic pockets (CH)  Disturbed Sample [UU @ 2.0 psi]	95	45	0.26		
	+	$\dagger$				Very soft, gray and brown CLAY with organic pockets and organic clay pockets (CH to OH)					
		ST-04	15"			Disturbed Sample [UU @ 2.8 psi]	123	34	0.13		
10	10	ST-05	10"			_					
	-	ST-06	20"			Very soft, gray and dark gray CLAY with organic pockets and organic clay pockets (CH to OH)  [UU @ 4.4 psi]	123	37	0.18	50	27
	-	ST-07	21"								
<b>−-15</b>	15-	ST-08	24"			Medium, greenish gray and light gray CLAY with organic and silt pockets (CL)	26			44	23
	-	ST-09	24"				-				
- <b>-20</b>	20	ST-10	24"			Stiff, greenish gray and light gray CLAY with silt pockets and ferrous nodules (CH)  [UU @ 7.6 psi]	23	103	1.57		
- <b>-2</b> 5	25	ST-11	15"			Dense, tan SILT with sand and trace of clay (ML)  [96.8% passing #200 sieve]	29			32	4
Templa	ate: 10001	431_LDNR	Proj ID	: 10001431	LDNR.G	TIDC				Printed: 7	7/31/13

## Log of Boring B-14



#### **Log of Boring B-15**

Date(s) Drilled 8/30/07	Logged By	J. Murray	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 1/2" Wing Bit	Total Depth Drilled (feet) 51.0
Drill Rig Type Air Boat	Drilling Contractor	ESS	Sampler Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation -0.40 '
Location N-584082.59, E-3501143.64			Borehole Backfill Grout full depth

		SA	MPLI	ES						_	%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
		ST-01	15"			- - -					
	-	ST-02	12"			Very soft, gray CLAY with organic pockets (CH) [UU @ 1.2 psi]	130	37	0.23	170	110
<b>−-5</b>	5-	ST-03	11"			Very soft, gray CLAY with organic pockets (CH)  [UU @ 2.0 psi]	132	34	0.24		
	-	ST-04	6"			-					
<b>−-10</b>	10	ST-05	20"			Very soft, greenish gray and light gray CLAY with trace of organics (CH)  [UU @ 3.6 psi]	75	62	0.11	51	34
	10	ST-06	22"								
	-	ST-07	20"			Medium, greenish gray and light gray CLAY with trace of organic pockets (CH)	32			55	38
<b>−-15</b>	15-	ST-08	15"			- 					
	-	ST-09	20"			- - -					
- <b>-20</b>	20-	ST-10	24"			Medium, greenish gray and light gray CLAY with calcareous nodules (CH)  [UU @ 10.1 psi]	27	100	0.80	56	37
	-					- - -					
- <b>-25</b>	25-	ST-11	24"			Stiff, brown, tan and light gray CLAY with trace of silt and ferrous nodules (CH)  [UU @ 10.0 psi]	29	91	1.56		
Templa	ite: 10001	431_LDNR	Proi ID	: 10001431	LDNR	TRS —				Printed:	7/31/13

## Log of Boring B-15

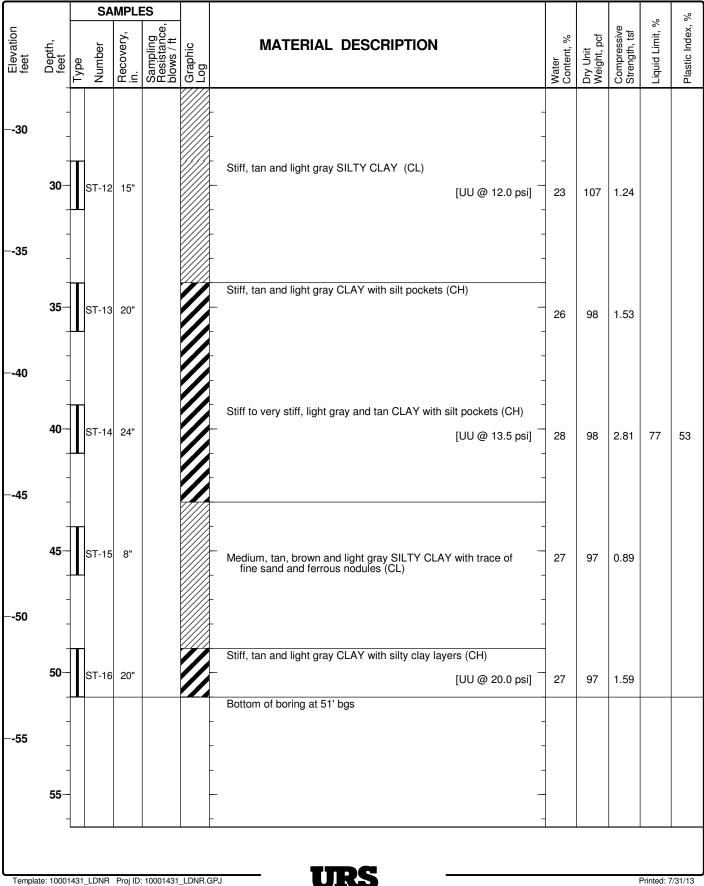
uo			AMPL				.0	*:	ive	it, %	ex, %
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Grapnic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
- <b>-3</b> 0	- - 30- -	ST-12	2 20"			Stiff, tan and light gray CLAY with silt pockets and ferrous nodules (CH)  [UU @ 12.0 psi]	28	91	1.31	68	44
<b>35</b>	- 35- -	ST-13	3 20"			- - - -					
- <b>-40</b>	40 -	ST-14	1 24"			Medium to stiff, light gray and tan CLAY with trace of silt pockets and ferrous nodules (CH)	39	80	1.04	80	55
- <b>-4</b> 5	- 45	ST-15	5 24"			Stiff, light gray and tan CLAY with trace of silt pockets and ferrous nodules (CH)	30	91	1.29		
<b>−-50</b>	50- -	ST-16	5 24"			Stiff, light gray and tan CLAY with trace of silt pockets and ferrous nodules (CH)  Bottom of boring at 51' bgs	34	85	1.45	85	55
<b>−-5</b> 5	- - <b>55</b>					- - -	-				
Templa	ite: 10001	1431_LDNR	Proj ID	: 10001431_L	DNR.G	URS			-	Printed: 7	7/31/13

## Log of Boring B-16

Date(s) Drilled 8/30/07	Logged By	J. Murray	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 1/2" Wing Bit	Total Depth Drilled (feet) <b>51.0</b>
Drill Rig Type Air Boat	Drilling Contractor	ESS	Sampler Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation -2.30 '
Location N-585264.10, E-3499959.56			Borehole Backfill Grout full depth

		SA	MPLI	ES						. 0	%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
	-	ST-01	5"			- -					
<b>−-</b> 5	-	ST-02	4"			Very soft, gray and dark gray CLAY with organics (CH to OH)	177			180	137
	5-	ST-03	NR			- Variable and did one OLAVe the arrain (OLA)	-				
<b>−-10</b>	-	ST-04	10"			Very soft, gray and dark gray CLAY with organics (CH)	54			55	37
	10	ST-05	20"			Soft, greenish gray and light gray CLAY with vertical root masses (CH)  [UU @ 3.6 psi]	48	69	0.36	62	44
	-	ST-06	15"			-					
<b>−-15</b>	-	ST-07	18"			Stiff, greenish gray and light gray SILTY CLAY with vertical root masses (CL)	25			44	28
	15-	ST-08	20"				_				
20	-	ST-09	6"				-				
	20	ST-10	24"			Medium, greenish gray and light gray CLAY with trace of organics (CH)  [UU @ 7.6 psi]	27	96	0.55	61	45
- <b>-2</b> 5	-					- -	_				
	25-	ST-11	10"			Stiff, brown and tan SILTY CLAY with trace of fine sand (CL)	30			33	10
Templa	ate: 10001	431_LDNR	Proj ID	: 10001431	LDNR.G	TRS —				Printed:	7/31/13

#### **Log of Boring B-16**

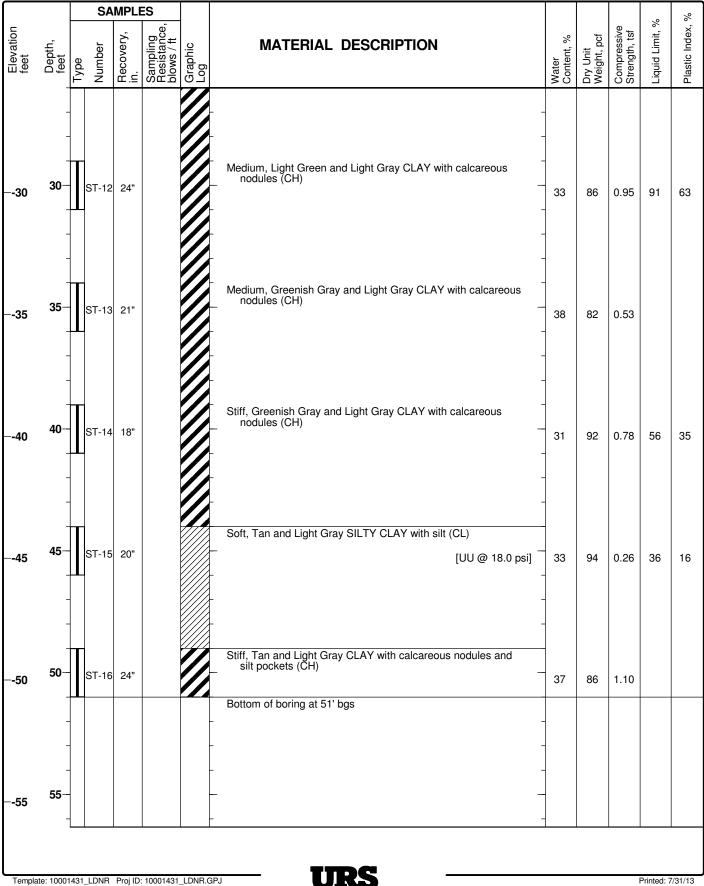


## Log of Boring B-17

Date(s)	Logged	J. Murray	Checked
Drilled 9/3/07	By		By M. Shewalla
Drilling	Drill Bit	3 1/2" Wing Bit	Total Depth
Method Rotary Mud	Size/Type		Drilled (feet) 51.0
Drill Rig	Drilling	ESS	Sampler
Type Air Boat	Contractor		Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation 0.30 '
Location N-586688.18, E-3499945.87			Borehole Backfill Grout full depth

		SA	MPLI	ES						. 0	%
Elevation feet		Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
-0	0-	П			12						
	4	ST-01	10"			-	1				
	+	Ħ				Very Soft, Greenish Gray and Brown CLAY with organics (CH					
	-	ST-02	11"			to OH)	177			184	137
	+	H				Very Soft, Gray and Dark Gray Muck CLAY with organics (CH	-				
<b>⊢-5</b>	5-	ST-03	12"			_ to OH)	190			203	151
	+	╂				-	1				
	-	ST-04	8"			-	-				
	+	H				-	1				
	4	ST-05	15"			-	1				
<b>−-10</b>	10	H				Very Soft, Organic CLAY with clay and organics (OH)	1				
-10	4	ST-06	14"			-	302			285	188
	+	┦				-	- 302			200	100
	-	ST-07	20"			-	1				
	1	<b></b>				-					
	15-	ST-08	21"				_				
<b>−-15</b>	-		-								
		ST-09	24"			Very Soft, Light Gray and Dark Gray SILTY CLAY with organic pockets (CL)					
		<b>1</b> 51-09	24"			[UU @ 6.8 psi]	47	79	0.18	45	28
		11				Stiff, Greenish Gray and Light Gray SILTY CLAY with clay (CL)					
		ST-10	24"			[UU @ 10.1 psi]	24	104	1.84		
<b>20</b>	20										
	_					<del>-</del>					
						-	1				
	1					-	1				
	†	T				Very stiff, Greenish Gray and Light Gray CLAY with trace of silt	1				
<b>−-25</b>	25-	ST-11	24"			pockets (CH)	22	107	2.16		
				<u> </u>							
Templa	ite: 10001	431_LDNR	Proj ID	: 10001431	_LDNR.G	TRS —				Printed:	7/31/13

#### Log of Boring B-17

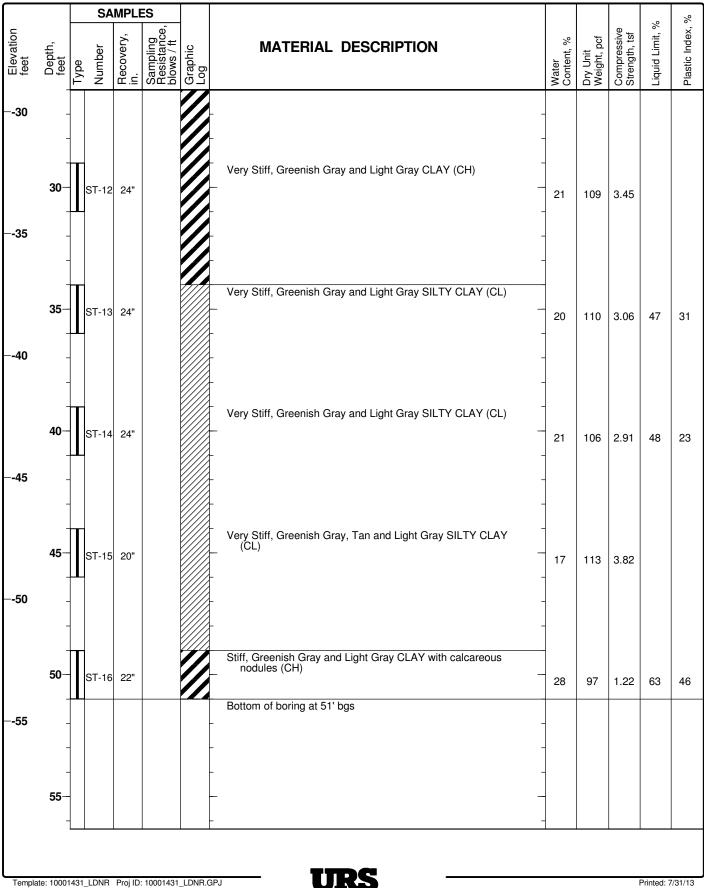


## Log of Boring B-18

Date(s)	Logged	J. Murray	Checked
Drilled 9/3/07	By		By M. Shewalla
Drilling	Drill Bit	3 1/2" Wing Bit	Total Depth
Method Rotary Mud	Size/Type		Drilled (feet) 51.0
Drill Rig	Drilling	ESS	Sampler
Type Air Boat	Contractor		Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation -3.10 '
Location N-588144.78, E-3499879.01			Borehole Backfill Grout full depth

		SA	MPLI	ES						<b>\0</b>	%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
<b>⊢-5</b>	0-	ST-01	10"			-					
	-	ST-02	8"			Very Soft, Gray and Black CLAY with organic pockets and organic clay (CH/OH)	217			146	110
	5-	ST-03	10"			Very Soft, Gray and Black CLAY with organic pockets and Organic CLAY (CH/OH)  [UU @ 2.0 psi]	173	19	0.18		
10	-	ST-04	12"			Very Soft, Gray and Black Organic CLAY and CLAY with organics (OH)	552			472	170
	10-	ST-05	21"			Very Soft, Brown and Black Organic CLAY with organics (OH)	408			575	300
<b>⊢-15</b>	-	ST-06	18"			-					
	-	ST-07	24"			Very Soft, Gray, Brown and Black Organic CLAY with organics (OH)  [UU @ 5.2 psi]	200	26	0.07		
	15-	ST-08	24"				-				
<b>−-20</b>	-	ST-09	24"			_	_				
	20-	ST-10	24"			Soft, Greenish Gray and Light Gray CLAY (CH)	40	84	0.43	70	51
<b>−-25</b>	-					- -	-				
	-					- Stiff Croonigh Croy and Light Croy CLAY (CLI)	-				
	25-	ST-11	24"			Stiff, Greenish Gray and Light Gray CLAY (CH)  -	23	104	1.82	53	32
Templa	ate: 10001	431_LDNR	Proj ID	: 10001431	LDNR.G	TRS —				Printed:	7/31/13

#### Log of Boring B-18



#### **Log of Boring B-19**

Date(s) Drilled 9/3/07	Logged By	J. Murray	Checked By M. Shewalla
Drilling Method Rotary Mud	Drill Bit Size/Type	3 1/2" Wing Bit	Total Depth Drilled (feet) 51.0
Drill Rig Type Air Boat	Drilling Contractor	ESS	Sampler Type(s) Shelby Tube
Groundwater Level and Date Measured Not Recorded	Hammer Data	140 lb Cat Head	Approximate Surface Elevation 0.00 '
Location N-589474.49, E-3499884.19			Borehole Backfill Grout full depth

		SA	MPLI	ES						_	%
Elevation feet	- 1	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, 6
-0	0-				1/						
	-	ST-01	12"			-					
	†	Ħ				Very Soft, Gray and Black Organic CLAY with organics (OH)					
		ST-02	5"			-	220			185	129
_	آ ہے	T .				Very Soft, Black and Brown Organic CLAY with organics (OH)					
<b>⊢-5</b>	5-	ST-03	15"				166	27	0.09	340	197
	]	Π				Very Soft, Gray CLAY and Black and Brown Organic Clay with organics (CH/OH)					
		ST-04	11"				157	32	0.10	293	254
		ST-05	00"								
<b>−-10</b>	10-	51-05	20"								
		ST-06	21"			Soft, Gray, Light Gray and Dark Gray CLAY with organics, silt pockets and vertical roots (CH)					
		131-00	21			[UU @ 4.4 psi]	38	87	0.33		
	_	ST-07	24"			Medium, Greenish Gray and Light Gray CLAY with silt and organics (CH)					
	1						35			58	42
<b>−-15</b>	15-	ST-08	24"								
	+	<b></b>				-	-				
	-	ST-09	21"			-					
	}	H				Very Soft Groonish Gray and Light Gray CLAV with silt and					
	-	ST-10	24"			Very Soft, Greenish Gray and Light Gray CLAY with silt and organic pockets (CH)  [UU @ 7.6 psi]	38	75	0.19		
20	20	Ц					- 50	/5	0.13		
	-					-	_				
	-					-					
	-					-	1				
	†	Т				Stiff, Greenish Gray and Light Gray CLAY with trace of silt (CH)	1				
<b>25</b>	25-	ST-11	24"			_	26	100	1.41	67	51
	1	Ш	<u> </u>								
Templa	te: 100014	431_LDNR	Proj ID	: 10001431	_LDNR.G	TRS —				Printed:	7/31/13

## Log of Boring B-19

		SA	MPLI	ES							
Elevation feet	Depth, feet	Type	Recovery, in.	Sampling Resistance, blows / ft	Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	Compressive Strength, tsf	Liquid Limit, %	Plastic Index, %
30	30-	ST-12	20"			Stiff, Greenish Gray and Light Gray CLAY with trace of silt and roots (CH)	26	100	1.58		
- <b>-3</b> 5	35-	ST-13	18"			Stiff, Greenish Gray and Light Gray SILTY CLAY with roots (CL)	23	100	1.72	50	33
<b>-40</b>	40-	ST-14	24"			Stiff, Greenish Gray and Light Gray SILTY CLAY with silt pockets (CL)	22	105	1.74	48	33
<b>−-45</b>	<b>45</b> —	ST-15	24"			Stiff, Greenish Gray and Light Gray CLAY with trace of silt (CL)	23	104	1.87	50	34
- <b>-50</b>	50-	ST-16	24"			Stiff, Greenish Gray and Light Gray CLAY with trace of silt (CH)  Bottom of boring at 51' bgs	22	107	1.85	60	43
- <b>-</b> 55	- 55—				-	- -	-				
Templa	ate: 10001	431_LDNR	Proj ID	: 10001431_L[	DNR.GF	URS —			ı	Printed: 1	7/31/13

Project Number: 19229541.00001

#### **Log of Boring B-1**

Date(s) Drilled 8/4/11 - 8/9/11	Logged By	Bukkapatnam, Ananth	Checked By Shewalla, Mahendra
Drilling Method Solid Stem/ Rotary Wash Drilling	Drill Bit Size/Type	5.5" Wing Bit	Total Depth Drilled (feet) 130.0
Drill Rig Type Diedrich D-50	Drilling Contractor	SESI	Sampler Type(s) Shelby Tube/ Split Spoon
Groundwater Level and Date Measured 8.0' after 15 min	Hammer Data	140 lbs, 30 inch drop	Approximate Surface Elevation 8.9'
Location N564749.90,E3499953.11			Borehole Backfill Cement Bentonite Grout

1			SA	MPLE	ES				(tsf)					%
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, 9
	0-	1					Loose; Tan SILTY SAND (SM)							
	_	$\mathbb{N}$		16	2		-			17				
		M					Soft; Tan and Brown LEAN CLAY (CL)							
<b>−</b> 5	_	$\Delta$		14	2		[94.8% passing #200 sieve]			30				
	5-			43			Brown SILT (ML) w/ clay strata and lenses			36				
	-	H					Gray and Tan SILT (ML) w/ clay pockets			35				
	-	Н								33			33	9
	_			43			Gray and Brown SILT (ML) w/ clay strata and lenses, sand strata and lenses	_	0.50	32 32				
<b>⊢</b> 0	10-							-	0.00	32				
	-						Gray SILT (ML) w/ clay pockets [95.7% passing #200 sieve]			38				
	-	Ш					-			34				
	-	Ш		26		3600	Cycly CILTY CAND (CNA)	-	0.50	29				
<b>−-5</b>	_	H					Gray SILTY SAND (SM) [43.7% passing #200 sieve] NO RECOVERY	-		29			NP	NP
	15						NO RECOVERY	-						
	-			48			Gray SILT (ML) w/ clay strata and lenses		0.50	31				
	-	Н								38				
<b>−-10</b>	-	H					NO RECOVERY			38				
	20-			46			Gray SILT (ML) w/ clay strata and lenses		0.75	31				
	_						O CAMPYOUT (MI)			30				
	-						Gray SANDY SILT (ML) [63.5% passing #200 sieve] NO RECOVERY			28				
<b>−-15</b>	-			45			Soft; Gray LEAN CLAY (CL)	_	0.50	20				
	25-						<ul> <li>Soft; Gray LEAN CLAY (CL) w/ silt lenses and pockets</li> </ul>	-	0.50	33 34				

Project Number: 19229541.00001

#### **Log of Boring B-1**

SAMPLES   SAMP	% 'Xerial Limit, % 'Xer
-20 3020 30	46 26
30	
30 Soft; Gray LEAN CLAY (CL) w/ silt lenses [3.1% Sand; 55.1% Silt; 41.8% Clay]  46 Soft; Gray CLAY (CH) w/ silt strata and lenses  Gray CLAY (CH) w/ organic pockets [2.4% Organic Content]  47 Medium; Dark Gray CLAY (CH) w/ organics [UU @ 22.6psi; 34.2psi and 35.9psi][9.6% Organic Content]  48 Medium; Gray CLAY (CH) w/ organics [UU @ 22.6psi; 34.2psi and 35.9psi][9.6% Organic Content]  Medium; Gray CLAY (CH) w/ organics [UU @ 22.6psi; 34.2psi and 35.9psi][9.6% Organic Content]  Medium; Gray CLAY (CH)	
Soft; Gray LEAN CLAY (CL) w/ silt lenses [3.1% Sand; 55.1% Silt; 41.8% Clay]  46  Soft; Gray CLAY (CH) w/ silt strata and lenses Gray CLAY (CH) w/ organic pockets [2.4% Organic Content]  44  45  Medium; Dark Gray CLAY (CH) w/ organics [UU @ 22.6psi; 34.2psi and 35.9psi][9.6% Organic Content]  Medium; Gray CLAY (CH)  Medium; Gray CLAY (CH)  Medium; Gray CLAY (CH)  Medium; Gray CLAY (CH)	
Soft; Gray CLAY (CH) w/ silt strata and lenses  Gray CLAY (CH) w/ organic pockets [2.4% Organic Content]  44  Medium; Dark Gray CLAY (CH) w/ organics [UU @ 22.6psi; 34.2psi and 35.9psi][9.6% Organic Content]  Medium; Gray CLAY (CH)  Medium; Gray CLAY (CH)  Medium; Gray CLAY (CH)	
Gray CLAY (CH) w/ organic pockets [2.4% Organic Content]  44  44  44  44  46  47  Medium; Dark Gray CLAY (CH) w/ organics [UU @ 22.6psi; 34.2psi and 35.9psi][9.6% Organic Content]  Medium; Gray CLAY (CH)  Medium; Gray CLAY (CH)  Medium; Gray CLAY (CH)	
35 44 56 62 0.75 111 111 111 111 111 111 111 96 46 0.94 Content][Consolidation Test]	
44  Medium; Dark Gray CLAY (CH) w/ organics [UU @ 22.6psi; 34.2psi and 35.9psi][9.6% Organic Content][Consolidation Test]  Medium; Gray CLAY (CH)  Medium; Gray CLAY (CH)	
Medium; Dark Gray CLAY (CH) w/ organics [UU @ 22.6psi; 34.2psi and 35.9psi][9.6% Organic Content][Consolidation Test]  Medium; Gray CLAY (CH)	
Medium; Dark Gray CLAY (CH) w/ organics [UU @ 22.6psi; 34.2psi and 35.9psi][9.6% Organic Content][Consolidation Test]  Medium; Gray CLAY (CH)  Medium; Gray CLAY (CH)	
40 Medium; Gray CLAY (CH)	161   118
Gray SILT (ML) w/ sand and clay pockets  [28.1% Sand; 64.1% Silt; 7.8% Clay]  38	NP NP
47 Stiff; Gray CLAY (CH)	
Medium; Gray CLAY (CH) w/ shell pockets	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
40 Stiff; Gray CLAY (CH)	
Gray CLAY (CH) w/ shell fragments	
[Consolidation Test] 52 37	86 62
39 1.75 25	
Stiff; Greenish Gray LEAN CLAY (CL) w/	
55	
Tampido 10000541 LAKEMAUDEDAS, Drai ID 10000544 LAKE MAUDEDAS ON TIPE	

Project Number: 19229541.00001

#### **Log of Boring B-1**

Sheet 3 of 5

-50 60 47 Greenish Gray and Light Gray LEAN CLAY (CL) wi silt [1,7% Sand: 65.5% Silt; 32.3% Clay] NO RECOVERY NO RECOVERY Silf; Gray LEAN CLAY (CL) Silf; Tan and Light Gray LEAN CLAY (CL) w/ sandy silt pockets [UU @ 32.2psi; 43.8psi and 45.5psi] 27 97 1.2 55 65 48 Silf; Tan and Light Gray and Brown CLAY (CH) Stiff; Tan and Light Gray CLAY (CH), jointed 48 Alternating Layers of Tan. Brown and Light Gray LEAN CLAY (CL) and SANDY SiLT (ML) NO RECOVERY  48 Very Stiff; Tan and Light Gray CLAY (CH) w/ silt pockets and streaks [UU @ 37.0psi; 48.6psi and 50.3psi] NO RECOVERY  48 Stiff; Tan, Brown and Light Gray CLAY (CH) 48 Stiff; Tan, Brown and Light Gray CLAY (CH) 48 Stiff; Tan, Brown and Light Gray CLAY (CH) 49 Stiff; Tan, Brown and Light Gray CLAY (CH) 40 31 32 33 34 34 35 36 37 42 36 37 42 38 38 38 40 2.55 39 40 2.50 26 39 40 2.50 26 39 40 2.50 26 39 40 2.50 26 39 40 2.50 26 39 40 2.50 39 40 2.50 36 31 36 37 38 38 40 39 40 39 40 39 40 39 40 39 40 39 40 30 30 30 30 30 30 30 30 30 30 30 30 30				SA	MPLE	ES				(tsf)				_	%
Greenish Gray and Light Gray LEAN CLAY (CL) Will (1/7% Sand; 65.5% Silt; 32.8% Clay) NO RECOVERY  80	Elevation feet	Depth, feet	Туре	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, 9
NO RECOVERY   Siff; Gray LEAN CLAY (CL)   Siff; Tan and Light Gray LEAN CLAY (CL)   w/ sandy silt pockets   Siff; Tan and Light Gray and Brown CLAY (CH)   1.25   37   37   37   42   83   1.2   30   30   30   30   30   30   30   3	- 50	-			24			w/ silt [1.7% Sand; 65.5% Silt; 32.8% Clay]		1.25				39	22
-55 65 65 65 65 65 65 65 65 65 65 65 65 6	-50	60-			47			Stiff; Gray LEAN CLAY (CL)		2.25	26				
Stiff; Tan, Light Gray and Brown CLAY (CH)		-						sandy silt pockets			27	97	1.26	40	23
-60 70  Alternating Layers of Tan, Brown and Light Gray LEAN CLAY (CL) and SANDY SILT (ML)  Very Stiff; Tan and Light Gray CLAY (CH) w/ silt pockets and streaks  [UU @ 37.0psi; 48.6psi and 50.3psi]  NO RECOVERY  47  Stiff; Tan, Brown and Light Gray CLAY (CH)  48  2.00 31 30 31 30 32 48  2.00 31 30 32 40 2.50 39 46 39 40 2.50 2.50 26 34  IUU @ 40.2psi; 51.8psi and 53.5psil  [UU @ 40.2psi; 51.8psi and 53.5psil 35 84 1.1	<b>⊢-55</b>	<b>65</b> —			48			_		1.25	37				
Alternating Layers of Tan, Brown and Light Gray LEAN CLAY (CL) and SANDY SILT (ML)  NO RECOVERY  48  Very Stiff; Tan and Light Gray CLAY (CH) w/ silt pockets and streaks  [UU @ 37.0psi; 48.6psi and 50.3psi]  NO RECOVERY  47  Stiff; Tan, Brown and Light Gray CLAY (CH)  48  Stiff; Tan, Brown and Light Gray CLAY (CH)  48  Stiff; Light Brown LEAN CLAY (CL) w/ sandy silt layers and pockets  [UU @ 40.2psi; 51.8psi and 53.5psi]  [UU @ 40.2psi; 51.8psi and 53.5psi]  42  43  34  35  84  1.1		_			40			Stiff; Tan and Light Gray CLAY (CH), jointed			30	83	1.20		
-65 75 48 Very Stiff; Tan and Light Gray CLAY (CH) w/ silt 30  [UU @ 37.0psi; 48.6psi and 50.3psi] NO RECOVERY  47 Stiff; Tan, Brown and Light Gray CLAY (CH)  48  Stiff; Light Brown LEAN CLAY (CL) w/ sandy silt layers and pockets  [UU @ 40.2psi; 51.8psi and 53.5psi]  31 2.00 31 30 32 33 86 2.50 39 40 2.50 2.50 26 34 35 84 1.1	<b>⊢-60</b>	<b>70</b> -			40			LEAN CLAY (CL) and SANDY SILT (ML)		2.25	34			35	15
[UU @ 37.0psi; 48.6psi and 50.3psi]  A7  Stiff; Tan, Brown and Light Gray CLAY (CH)  48  Stiff; Light Brown LEAN CLAY (CL) w/ sandy silt layers and pockets  [UU @ 40.2psi; 51.8psi and 53.5psi]  33  86  2.50  39  40  2.50  2.50  26  34  35  84  1.1	0.5	_			48			Verv Stiff: Tan and Light Gray CLAY (CH) w/ silt		2.00					
Stiff; Tan, Brown and Light Gray CLAY (CH)  48  Stiff; Light Brown LEAN CLAY (CL) w/ sandy silt layers and pockets  [UU @ 40.2psi; 51.8psi and 53.5psi]  12.50  46  39  40  2.50  26  34  35  84  1.1	<b></b> 65	75- -						[UU @ 37.0psi; 48.6psi and 50.3psi]				86	2.39		
80 40 2.50 26 34 Stiff; Light Brown LEAN CLAY (CL) w/ sandy silt layers and pockets 34 35 84 1.1		-	<b>+</b>		47			Stiff; Tan, Brown and Light Gray CLAY (CH)		2.50	46				
[UU @ 40.2psi; 51.8psi and 53.5psi] 34 35 84 1.1	<b>70</b>	80-			48					2.50	40				
		-						layers and pockets			35	84	1.15	48	31
Tan, SANDY SILT (ML)  Alternating Layers of Gray CLAY (CH) and SILT (ML)  2.00 27 33	<b>⊢-75</b>	85-			47					2.00	27				

Project Number: 19229541.00001

#### **Log of Boring B-1**

Sheet 4 of 5

			SA	MPLE					(tst)				. 0	%
Elevation feet	Depth, feet	Туре	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index,
	-	Ħ					Medium; Gray CLAY (CH) w/ silt lenses and layers [UU @ 42.2psi; 53.8psi and 55.5psi]			37 40	79	0.95	52	33
<b>⊢-80</b>	-			47			Gray LEAN CLAY (CL)		1.25	34				
	90-	Щ					- Alternating Layers of Cray CLAY (CH) and Tan			34				
	-	$\mathbb{H}$					Alternating Layers of Gray CLAY (CH) and Tan LEAN CLAY (CL)			35			77	55
	-	$\mathbf{H}$		48			Stiff; Gray CLAY (CH) w/ sand strata and lenses			42				
	-	H		40			-		1.50	38 48				
<b>⊢-85</b>	-	П					Stiff; Gray CLAY (CH) w/ silt lenses and pockets [UU @ 45.0psi; 56.6psi and 58.3psi]			38	85	1.09		
	95-						_			34				
	-	Щ		48			Stiff; Gray CLAY (CH) w/ sand strata and lenses		1.25	35				
	-	$\mathbb{H}$					Gray CLAY (CH) w/ silt lenses			36				
<b>−-90</b>	-	$\mathbf{H}$					Stiff; Gray CLAY (CH) w/ silt pockets and lenses			39			64	42
	100-	Н		48			[UU @ 47.0psi; 58.6psi and 82.0psi] Stiff; Gray CLAY (CH) w/ sand strata and lenses		4.05	36	84	1.67		
	-	Ħ					- -		1.25	39				
	-						Stiff; Gray CLAY (CH) w/ shells and silt lenses [UU @ 48.2psi; 59.8psi and 61.5psi]_			33	90	1.63		
<b>−-9</b> 5	-						_			34				
	105-	Ш		48			Stiff; Light Gray CLAY (CH) w/ silt strata and lenses		1.25	40				
	-	$\mathbb{H}$					Gray CLAY (CH)			37				
	-	$\mathbf{H}$					-			37			65	42
	-	$\mathbf{H}$		48			-		4.05	44				
<b>−-100</b>	-	Ħ					- -		1.25	47 46				
	110-						Stiff; Gray CLAY (CH)			47	74	1.69		
	_						-			49				
	-	Щ		48			Stiff; Gray CLAY (CH)		1.50	49				
<b>−-105</b>	-	$\mathbb{H}$					Gray CLAY (CH)			50				
	115-	$\mathbf{H}$								48			87	59
	-	H		47			-		1.05	48				
	-			<u> </u>	<u> </u>				1.25	48				<u> </u>
Templa	te: 192295	541-LA	KEMAI	JREPAS	Proj ID: 19	9229541 -	AKE MAUREPAS.GPJ URS						Printed:	8/2/13

B - 48

Project Number: 19229541.00001

#### Log of Boring B-1

Sheet 5 of 5

			SA	MPLE					(tsf)			)	\o	%
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
	-	$\blacksquare$					Stiff; Gray CLAY (CH)	-		46				
- <b>-110</b>	-	$oldsymbol{H}$					[UU @ 54.6psi; 66.2psi and 67.9p	si]_		47	76	1.68		
	120-	$\mathbf{H}$		47			Stiff; Gray CLAY (CH)	-	1.25	44				
	_								1.23	47				
	-						Gray CLAY (CH)			44			89	62
115	-	Н						-		36				
	125-	H		48				-	1.50	40				
	-	H					Very Stiff; Gray CLAY (CH) w/ silt pockets	-		42 44	79	2.13		
	_									44	/ 5	2.10		
120	-	Щ		48			Stiff; Gray CLAY (CH)  Gray CLAY (CH)	-	1.50	45				
	130-	Ц								45			82	54
	- - -	-				-	Bottom of Hole @ 130.0' bgs  Note 1: All samples were collected using a 48" long Piston Sampler  Note 2: Strength's shown are average values of 3-point UU test	-						
125	-					-		-						
	135-	-						-						
	-					-		-						
	_													
130	-							_						
	140-	$\left\{ \ \right\}$						-						
	-					-		-						
	-							-						
<b>−-135</b>	-													
	145-													
	-	$\left\{ \ \right $						-						
	-	Ш				-		_						

Project Number: 19229541.00001

#### **Log of Boring B-2A**

Date(s) Drilled	8/18/11 - 8/18/11	Logged By	Bukkapatnam, Ananth	Checked By Shewalla, Mahendra
Drilling Method	Solid Stem/ Rotary Wash Drilling	Drill Bit Size/Type	5.5" Wing Bit	Total Depth Drilled (feet) 40.0
Drill Rig Type	Diedrich D-50	Drilling Contractor	SESI	Sampler Type(s) Shelby Tube/ Split Spoon
Groundwa and Date I		Hammer Data	140 lbs, 30 inch drop	Approximate Surface Elevation 6.7'
Location	N565554.00, E3500470.89			Borehole Backfill Cement Bentonite Grout

			SA	MPLE	ES				(tsf)					%
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
	0-	П					Stiff; Brown and Gray CLAY (CH)			27				
<b>−</b> 5	_						-			27				
										27				
				46					2.25	34				
	5-						Stiff; Brown and Gray CLAY (CH)			43				
	J _	Ш					_			41				
<b>−</b> 0	_	Ш					Medium; Gray and Brown CLAY (CH) w/ ferrous nodules [1.7% Sand; 39.7% Silt; 58.6% Clay]_			41			64	45
	_			41			[2.4% Organic Content]		0.50	39				
	_	Ш					Soft; Gray CLAY (CH) w/ silt strata or lenses			36				
	10-	Ш					- M F 0 15 01 NV(01) : : : 1			35				
	_	Ш		36			Medium; Gray and Brown CLAY (CH), jointed [98.4% passing #200 sieve][3.0% Organic Content]		1.00	53	72	0.60		
<b>−-</b> 5	_	Ш					[11' - 12' - No Sample Recovered] -							
	_						Medium; Gray CLAY (CH) -			53				
	-	Ш					-			43				
	15-						Gray CLAY (CH), jointed [2.6% Organic Content]			46				
	-			48			-		1.25	53				
<b>−-10</b>	_	Н					Medium; Gray CLAY (CH) w/ organics			55				
	_	Н					- Madisum Curas CLAV (CU) initated			58				
	-	Ш					Medium; Gray CLAY (CH), jointed [UU @ 10.1psi; 21.8psi and 23.4psi]_			55	68	0.74	98	63
	20-			41			Coff. Crow and Dark Crow CLAV (CLI) w/ avagains		0.75	57				
	-						Soft; Gray and Dark Gray CLAY (CH) w/ organics			75				
<b>−-15</b>	-						Soft; Black and Dark Gray ORGANIC CLAY (OH)			93				
	_			35			w/ organics [17.7% Organic Content]_ [23' - 24' - No Sample Recovered]		0.50	150			231	145
	25-						Soft; Gray and Dark Gray CLAY (CH) w/ organics			77				
										63				

Project Number: 19229541.00001

#### **Log of Boring B-2A**

			SA	MPLE					(tst)				. 0	%
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
<b>−-20</b>	_			37			Soft; Dark Gray and Black CLAY (CH) w/ organics		0.50	97	47	0.45		
	-	Ш					[27' - 28' - No Sample Recovered]							
	-	lacksquare				-	Gray SANDY SILT (ML) w/ clay strata or lenses	-		39 36				
	30-	П					Soft; Gray CLAY (CH) w/ silt and shells			71			89	65
<b>−-2</b> 5	-	$\prod$		47			Soft; Gray CLAY (CH) w/ silt strata or lenses		0.50	70				
	-	Ш					Soft; Gray CLAY (CH) w/ shell fragments [UU @ 16.1psi; 27.8psi and 29.4psi]	-		71 74	57	0.36		
	-	Ш					[OO @ 10.1psi, 27.opsi and 29.4psi]	-		71	37	0.30		
	35-	П		47			- -		1.25	66				
<b>−-30</b>	-	П					Soft; Gray CLAY (CH) w/ silt strata or lenses		20	65				
	_									59				
	_	Ш					Stiff; Greenish Gray and Gray CLAY (CH) w/ silt strata or lenses			28				
	40-	Ш		45					1.25	25				
<b>3</b> 5	-	-				- - -	Bottom of Hole @ 40.0' bgs  Note 1: All samples were collected using a 48" long Piston Sampler  Note 2: Strength's shown are average values of 3-point UU test							
	45-					-	-	-						
<b>40</b>	_													
	_						_							
	-						_							
	50-						_							
	-					_								
<b>−-45</b>	-													
	-							-						
	-							-						
	55-	$\mid \mid$					-	-						
	-	Ш					-							
<u></u>		-4		IDES::			AKE MAUREPAS.GPJ URS						D.:	0/4/47
I empla	te: 19229	541-LA	KEMAl	JHEPAS	Proj ID: 19	1229541 - L	AKE MAUREPAS.GPJ						Printed:	ช/1/13

Project Number: 19229541.00001

#### **Log of Boring B-3A**

Date(s) Drilled	8/17/11 - 8/17/11	Logged By	Bukkapatnam, Ananth	Checked By Shewalla, Mahendra
Drilling Method	Solid Stem/ Rotary Wash Drilling	Drill Bit Size/Type	5.5" Wing Bit	Total Depth Drilled (feet) 40.0
Drill Rig Type	Diedrich D-50	Drilling Contractor	SESI	Sampler Type(s) Shelby Tube/ Split Spoon
Groundwa and Date N		Hammer Data	140 lbs, 30 inch drop	Approximate Surface Elevation 6.7'
Location	N565694.35, E3500536.99			Borehole Backfill Cement Bentonite Grout

			SA	MPLE	ES				tsf)					%
Elevation feet	Depth, feet	Туре	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, 9
	0-	$\prod$					Very Stiff; Brown and Light Gray CLAY (CH) w/ silt strata and streaks	-		21				
<b>−</b> 5	-						Stiff; Brown and Light Gray CLAY (CH) w/ silt strata and streaks			28 35				
	-			46			- Medium; Gray and Brown CLAY (CH) w/ wood	-	2.50	30				
	5-						<u>-</u>			53 51				
<b>−</b> 0	-	H		43			Light Gray and Tan CLAY (CH) w/ ferrous nodules, silt [2.6% Organic Content	l_	0.75	30 40				
	-						Soft; Gray CLAY (CH) w/ silt strata and streaks		0.75	42				
	10-						Soft; Gray CLAY (CH), jointed [8.1% Silt; 91.9% Clay][2.5% Organic Content	-		49 56	67	0.37	91	58
<b>−-5</b>	-			46			Soft; Gray CLAY (CH) w/ silt strata and streaks	-  -  -	1.00	53				
	-						-			50 52				
	15-			45			_	-	0.75	47				
<b>−-10</b>	-						Soft; Gray CLAY (CH) w/ silt strata and streaks, wood	-	0.75	48 59				
	-						Medium; Gray CLAY (CH), jointed [99.0%passing #200 sieve][2.8% Organic Content [UU @ 10.8psi; 22.5psi and 24.1psi	_		74 59	65	0.58	94	63
	20-			43			[UU @ 10.8psi; 22.5psi and 24.1psi Soft; Gray CLAY (CH) w/ organics		1.25	55		0.00		
<b>−-15</b>	_						-			78 272				
	_			47			Medium; Black and Dark Gray ORGANIC CLAY (OH) w/ clay strata and streaks PEAT and ORGANICS w/ clay [20.2% Organic Content		0.50	109			000	100
	- 25-			7/			Soft; Gray CLAY (CH) w/ organics, silt strata and streaks	<u> </u>  -	0.50	191 79			266	166
										81				

Project Number: 19229541.00001

#### **Log of Boring B-3A**

			SA	MPLE	S				(tsf)					%
Elevation feet	Depth, feet	Туре	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
<b>−-20</b>	_	Ш								51				
	-	Ш		46					0.50	56				
	-	Ш					Soft; Gray CLAY (CH) w/ silt strata and streaks, organics			56				
	30-	$\mathbb{H}$					Soft; Gray CLAY (CH) w/ silt lenses and layers [UU @ 15.2psi; 26.9psi and 28.5psi]			50	72	0.31		
	-	H		48			-			47				
<b>⊢-25</b>	-	Н		40			Soft; Gray CLAY (CH) w/ slickensides, silt strata and streaks		0.50	68				
	-	Ħ					and streaks  Very Soft; Gray CLAY (CH) w/ shell fragments			77 59	64	0.21	97	71
	05	П					-			43	04	0.21	37	, ,
	35-			48			_		0.50	65				
<b>30</b>	_	Ш					Soft; Gray CLAY (CH) w/ slickensides, silt strata and streaks			76				
	_	Ш								62				
	-	Ш					Stiff; Grennish Gray and Light Gray CLAY (CH) w/ silt strata and streaks			28				
	40-			47			Bottom of Hole @ 40.0' bgs		2.00	22				
<b>−-35</b>	- - -					- - -	Note 1: All samples were collected using a 48" - long Piston Sampler  Note 2: Strength's shown are average values of 3-point UU test							
	- 45-					-	- -							
<b>−-40</b>	-													
	_													
	_	-					_							
	50-						-							
	-					-	-							
<b>−-45</b>	-						-							
	-						-							
	-						-							
	55-						-							
	_										<u> </u>			
Templa	te: 192295	541-LA	KEMAL	JREPAS	Proj ID: 19	229541 - L	AKE MAUREPAS.GPJ URS						Printed:	8/1/13

Project Number: 19229541.00001

#### Log of Boring B-4A

Date(s) Drilled 8/16/11 - 8/16/11	Logged By	Bukkapatnam, Ananth	Checked By Shewalla, Mahendra
Drilling Method Solid Stem/ Rotary Wash Drilling	Drill Bit Size/Type	5.5" Wing Bit	Total Depth Drilled (feet) 41.0
Drill Rig Type <b>Diedrich D-50</b>	Drilling Contractor	SESI	Sampler Type(s) Shelby Tube/ Split Spoon
Groundwater Level and Date Measured 14.2' after 15 min	Hammer Data	140 lbs, 30 inch drop	Approximate Surface Elevation 6.6'
Location N565864.08, E3500618.35			Borehole Backfill Cement Bentonite Grout

		SAMPLES			ES				(tsf)					%
Elevation feet	Depth, feet	Туре	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, <sup>e</sup>
_	-						Stiff; Brown and Gray CLAY (CH) w/ sand strata and lenses, silt strata and lenses			23				
<b>−</b> 5	-	H					_			22				
	-	$\mathbf{H}$		42			<u>-</u>		4.25	19 25				
	5-						Stiff; Gray and Brown CLAY (CH) w/ sand strata and lenses, silt strata and lenses		4.25	41				
	_	Н					- Gray and Brown CLAY (CH) w/ silt, ferrous			31				
<b>⊢</b> 0	_	Н		48			nodules [2.9% Organic Content]_			28				
	-	Н		40			Stiff; Gray CLAY (CH) w/ silt strata and lenses		1.25	32 41				
	-						Stiff; Gray and Tan CLAY (CH) w/ silt strata and lenses			38				
	10-						Medium; Light Gray and Brown CLAY (CH), jointed [2.5% Organic Content]_			55	70	0.54		
<b>−-5</b>	-	Ш		42			_		0.75	52				
	-	Н					Stiff; Gray CLAY (CH) w/ silt strata and lenses			52				
	-	H								49				
	15-	Н		42			Gray CLAY (CH), jointed [16.6% Silt; 83.4% Clay][2.3% Organic Content]			49			77	52
<b>−-10</b>	_	$\mathbf{H}$		72			Soft; Gray CLAY (CH) w/ silt strata and lenses, organics		0.75	57 62				
	-						Ç		0.75	56				
	_						<ul> <li>Medium; Gray CLAY (CH) w/ organic pockets, calcium nodules, jointed [99.1% passing #200 sieve]</li> <li>[UU @ 13.4psi; 25.0psi and 26.7psi]</li> </ul>			61	64	0.65	97	66
	20-	Ш		43					0.75	60				
	-	Н					Medium; Gray and Dark Gray ORGANIC CLAY (OH) w/ organics ORGANICS w/ clay			188				
<b>−-15</b>	-	$\mathbb{H}$					Medium; Gray CLAY (CH) w/ silt strata and lenses			142			216	171
	-	$\mathbf{H}$		43					0.50	57				
	-						Soft; Gray CLAY (CH) w/ silt strata and lenses		0.50	60 61				
	25-						Soft; Gray CLAY (CH) w/ organic pockets			67	61	0.51		

Project Number: 19229541.00001

#### **Log of Boring B-4A**

			SA	MPLE	ES				(tst)				_	%
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
<b>−-20</b>	_	Ш								62				
	_	Щ		47			Gray SANDY SILT (ML)		0.25	25				
	-	X		24	WOH		Medium; Gray CLAY (CH) w/ silt strata and lenses			52				
	30-						_			52				
0.5	-	H					-			65				
<b>−-25</b>	-	H					-			69				
	-	$\mathbf{H}$		47			-			61				
	-	$\mathbf{H}$		7,			Soft; Gray CLAY (CH) w/ slickensides		0.50	50 43				
	35-	H					Soft; Gray ORGANIC CLAY (OH) w/ organics			69				
<b>−-30</b>	-	П					Medium; Gray CLAY (CH) w/ silt pockets [UU @ 20.6psi; 32.2psi and 33.9psi]			34	89	0.62	52	35
	_			48			[60 € 20060, 02.260 3.10 00060]		1.25	28				
	_						Stiff; Gray CLAY (CH) w/ silt strata and lenses			29				
	40-						_			22				
	-	Ш		35					2.50	22				
<b>−-35</b>	-	-				-	Bottom of Hole @ 41.0' bgs  Note 1: All samples were collected using a 48" - long Piston Sampler  Note 2: Strength's shown are average values of 3-point UU test							
	45						- - -							
<b>40</b>	_						-							
	-					-	-							
	-	-				-	-							
	50-	-					-							
<b>−-4</b> 5	-	$\mid \mid$					-							
	-						-							
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	-						-							
	55-	$] \mid$												
						<u>Ι Γ</u>			<u> </u>	<u> </u>	<u> </u>	<u> </u>		<u> </u>
Templa	te: 192295	541-L/	AKEMAL	JREPAS	Proj ID: 19	9229541 - L	AKE MAUREPAS.GPJ URS						Printed:	8/1/13

Project Location: Garyvile, LA Project Number: 10001863

#### **Log of Boring KCS-01**

Date(s) Drilled 1/22/13 - 1/24/13	Logged By	M. Shewalla	Checked By A. Bukkapatnam
Drilling Auger 0' to 10'; Rotary Wash 10' to Method 155'	Drill Bit Size/Type	4" Auger	Total Depth Drilled (feet) <b>155.0</b>
Drill Rig Type SIMCO 2800	Drilling Contractor	APS Testing	Sampler Type(s) Shelby Tube/Split Spoon
Groundwater Level and Date Measured 7.0' at 0 Mins; 5.0' after 15 Mins	Hammer Data	140 lbs	Approximate Surface Elevation 2.6'
Location N572379.75, E3504323.37			Borehole Backfill Cement and Betonite Grout

ſ			SA	MPLE	ES				tsf)					%
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
	-		ST-1	20			Light Gray CLAY (CH) w/ Fe Nodules	_	1.00	43				
-0	_		ST-2	17			Light Gray to Brown CLAY (CH) w/ Fe Nodules		0.50	45			100	62
	5-		ST-3	14			_ Medium, Light Gray to Brow CLAY (CH) w/ Fe Nodules	_	0.50	49	72	0.58		
<b>⊢-5</b>	-		ST-4	22			Medium, Light Gray to Brow CLAY (CH) w/ Fe Nodules [UU @5.0psi		0.25	39	82	0.52	58	30
	10-		ST-5	10			Gray CLAY (CH) w/ Fe Nodules and tree roots	_	1.25	44			82	49
	-						-	_						
<b>-1</b> 0	- 15-		ST-6	22			- Soft, Gray CLAY (CH) w/ organic content [UU @7.8psi -	_	0.25	99	46	0.32		
<b>−-15</b>	20-		ST-7	23			- - Soft, Gray CLAY (CH) w/ organic content [UU @9.8psi	- - -	0.25	61	64	0.40	93	61
- <b>-2</b> 0	- - - 25—		ST-8	17			- - Gray CLAY (CH) w/ organic content	-	0.25	50			70	43
Templa	ate: 10001	863_	LNDR-I	KCS P	roj ID: 1000	1863-KC	S-LMAUREPAS.GPJ URS						Printed:	8/2/13

Project Location: Garyvile, LA Project Number: 10001863

#### **Log of Boring KCS-01**

		SA	MPLI					(tsf)				. 0	%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
<b>25</b>	- - 30-	ST-9	22			Medium, Light Gray CLAY (CH)  -		0.50	30	92	0.61		
30	35—	ST-10	23			Gray SILTY CLAY (CL) w/ sand lenses	-	1.50	21			36	17
- <b>-3</b> 5	- - 40-	ST-11	20			Stiff, Tan and Light Gray to Brown CLAY (CH)  -	-	1.75	37	84	1.15		
40	- - 45	ST-12	2 16			Stiff, Light Gray to Brown CLAY (CH) w/ sand pockets [UU @19.8psi]	-	1.25	29	96	1.75		
<b>4</b> 5	- - 50-	SS-13	3 17	32		Very Firm, Brown SILT (ML) w/ sand	-		26				
- <b>-50</b>	- - 55—	SS-14	. 12	24	-	Firm, Brown SILT (ML) w/ sand	-		29				
Templa	ate: 10001	863_LNDR-	KCS P	roj ID: 1000	11863-KCS	-LMAUREPAS.GPJ URS	•	•				Printed:	8/2/13

Project Location: Garyvile, LA Project Number: 10001863

## Log of Boring KCS-01

Sheet 3 of 6

		SA	MPLI	ES				(tsf)					%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
<b>55</b>	- - 60-	SS-15	5 14	13		Stiff, Light Gray to Brown SILTY CLAY (CL)			28			49	28
60	- - 65-	ST-16	3 16			Stiff, Gray to Brown CLAY (CH) w/ silt pockets		1.25	28	96	1.16		
<b>-6</b> 5	- - 70-	ST-17	13			Gray to Brown CLAY (CH) w/ silt/sand lenses		1.00	35			51	30
- <b>-70</b>	- - 75-	ST-18	3 14			Gray to Brown CLAY (CH) w/ shells		1.00	31			51	32
<b>−-75</b>	- 80-	ST-19	18			Gray to Brown CLAY (CH)		1.00	43			79	52
- <b>-80</b>	- - 85-	ST-20	20			Stiff, Gray CLAY (CH)  [UU @35.8psi]		1.25	52	71	1.40		
Templa	ate: 10001		KCS P	roj ID: 1000	11863-KC	S-LMAUREPAS.GPJ URS	-	ļ		I		Printed:	8/2/13

Project Location: Garyvile, LA Project Number: 10001863

#### **Log of Boring KCS-01**

Sheet 4 of 6

		SA	MPLE	ES				(tst)					%
Elevation feet	Depth, feet	Type Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
- <b>-8</b> 5	90	ST-21	21			Gray CLAY (CH)		1.00	48			95	66
90	95	ST-22	20			Very Stiff, Gray CLAY (CH) w/ sand lenses [UU @39.8psi]		1.25	46	74	2.27		
<b>9</b> 5	100	SS-23	13	17		Firm, Gray SANDY SILT (ML)			22				
<b>105</b>	105	ST-24	19			Stiff, Gray CLAY (CH) w/ sand lenses		1.50	30	95	1.42		
105	110	ST-25	13		-	Firm, Gray SILT (ML) w/ clay [UU @45.8psi]		1.00	31	93	1.63	31	5
<b>−-110</b>	115-	SS-26	14	36	-	Very Firm, SILT (ML) w/ sand			32				
Templa	te: 10001	, 863_LNDR-	KCS P	roj ID: 1000	1863-KCS	S-LMAUREPAS.GPJ URS	+	+				Printed:	8/2/13

Project Location: Garyvile, LA Project Number: 10001863

## Log of Boring KCS-01

Sheet 5 of 6

			SA	MPL						(tst)				%	%
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic	Fog	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
115	_														
	- 120-		SS-27	15	17		_	Firm, Gray SILT (ML) [92.7% passing #200 Sieve]			31				
<b>−-120</b>	-							-	-						
-120	- 125-		SS-28	13	14			Stiff, Gray SILTY CLAY (CL) w/ shells			30			48	23
<b>−-125</b>	-	-							_						
-125	130-		ST-29	16				Gray CLAY (CH) w/ shells	-	1.00	47				
<b>−-130</b>	-	-													
	135- -		ST-30	21				Very Stiff, CLAY (CH) w/ silt lenses		1.50	47	75	2.02	78	45
<b>−-135</b>	-							-	-						
-100	- 140-		ST-31	7				Gray SAND (SP) w/ silt [5.2% passing #200 Sieve]			26				
<b>−-140</b>	- - 145—		SS-32	14	45			Very Dense, Gray SILTY SAND (SM)							
	-							S-LMAUREPÄS.GPJ <b>URS</b>						Printed:	

Project Location: Garyvile, LA Project Number: 10001863

#### **Log of Boring KCS-01**

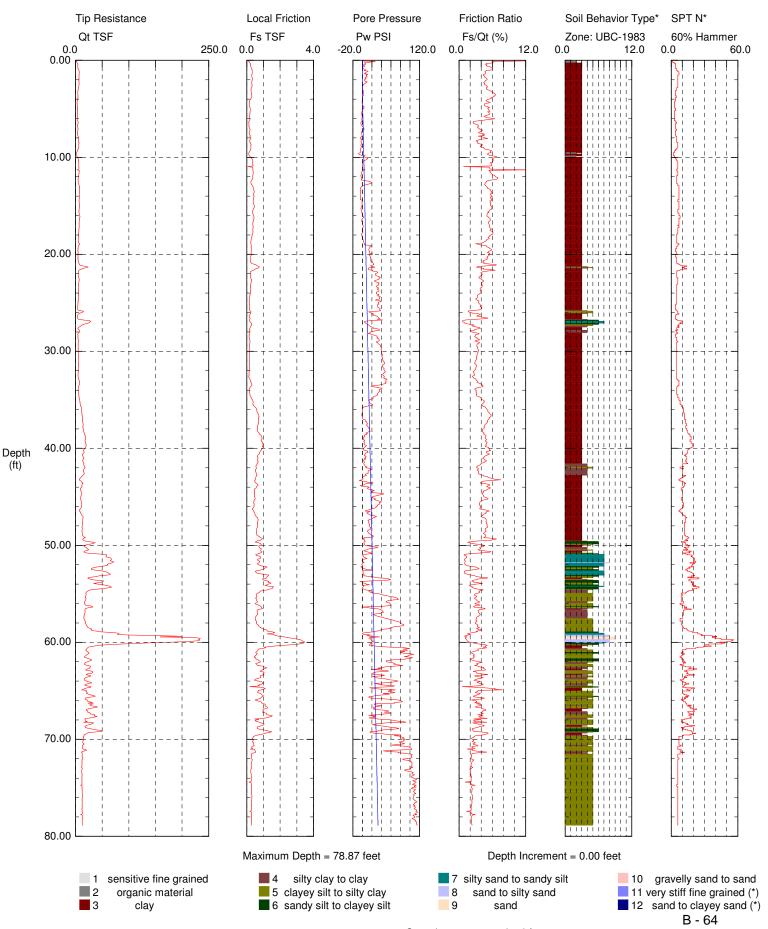
Sheet 6 of 6

			SA	MPLE	ES				tsf)					,o
Elevation feet	Depth, feet	Type	Number	Recovery, in.	Sampling Resistance, blows / ft	Graphic Log	MATERIAL DESCRIPTION	Torvane (tsf)	Hand Penetrometer (tsf)	Water Content, %	Dry Unit Weight, pcf	Cohesion (ksf)	Liquid Limit, %	Plastic Index, %
<b>−-145</b>	150-	X	SS-33	9	43		Dense, Gray SILTY SAND to SAND (SP-SM)			28				
<b>−-150</b>	-	-					- -							
	155-	0:	SS-34	NR	>50		Very Dense, Gray SILTY SAND to SAND (SP-SM)							
	-	-					Bottom of Hole - 155' bgs							
<b>−-155</b>	-	-					- -							
	160-	-												
1 <b>60</b>	-	-					- - -							
	165-	-					_ -							
<b>−-165</b>	-	-					- -							
	170-	-					- - -							
<b>−-170</b>	-	-				-	- -							
	175-	-					- -							
<b>−-175</b>	-	-												
Templat	e: 1000°	1863	LNDR-	KCS P	roj ID: 1000	1863-KC	S-LMAUREPAS.GPJ URS						Printed:	8/2/13

# APPENDIX II EXISTING GEOTECHNICAL CPTS

Operator: TTC CPT Date/Time: 9/14/2007 10:43:49 AM

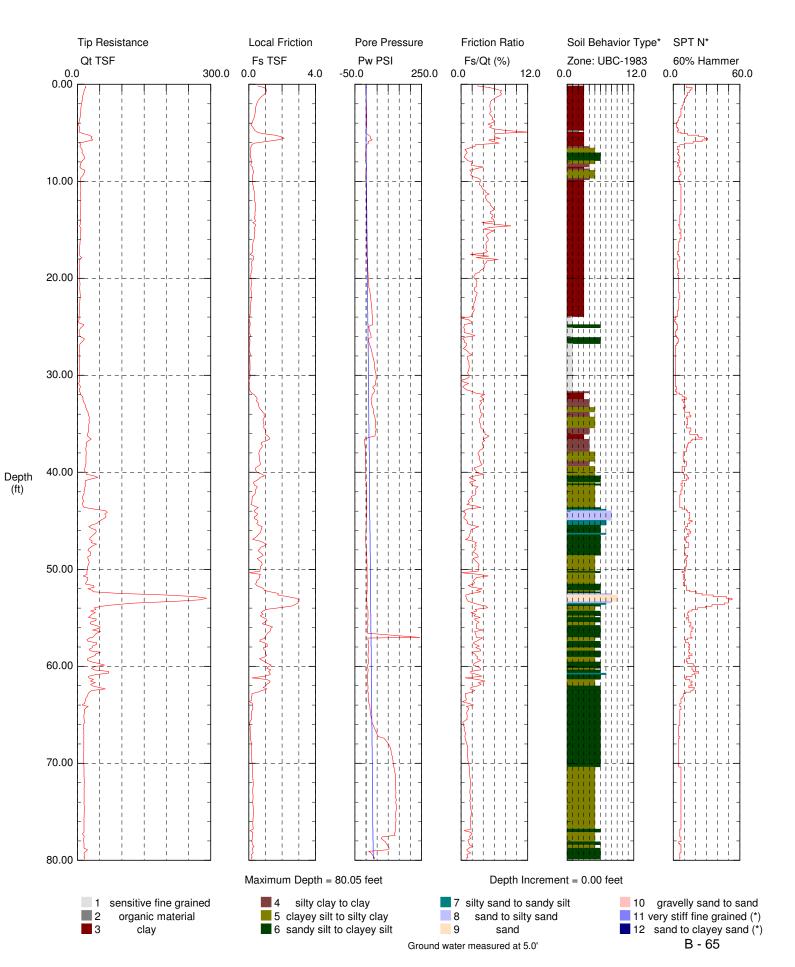
Sounding: CPT-03 ELEV. 6.00 ft Location: LAKE MAUREPAS Cone Used: DDG0899 Job Number: B07-163



Operator: Mike Wright
Sounding: CPT-04 ELEV. 5.00 ft

Cone Used: DSG0780

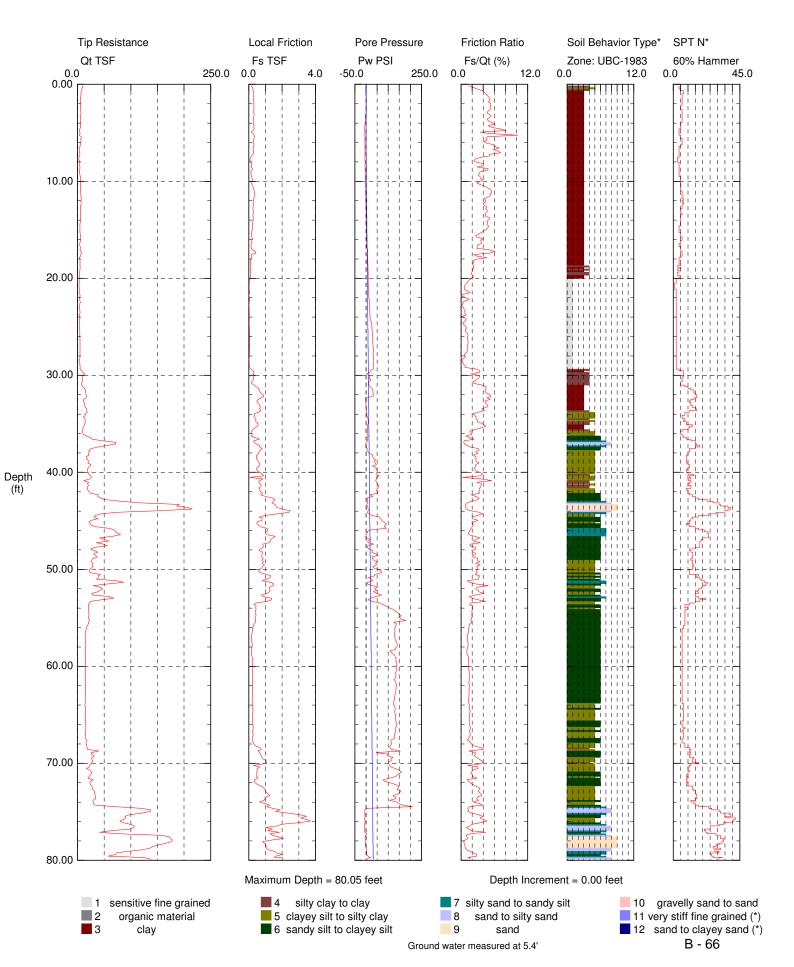
CPT Date/Time: 8/16/2007 3:37:40 PM Location: DNR Maurepas Diversion



Operator: Mike Wright
Sounding: CPT-05 ELEV. 6.30 ft

Cone Used: DSG0780

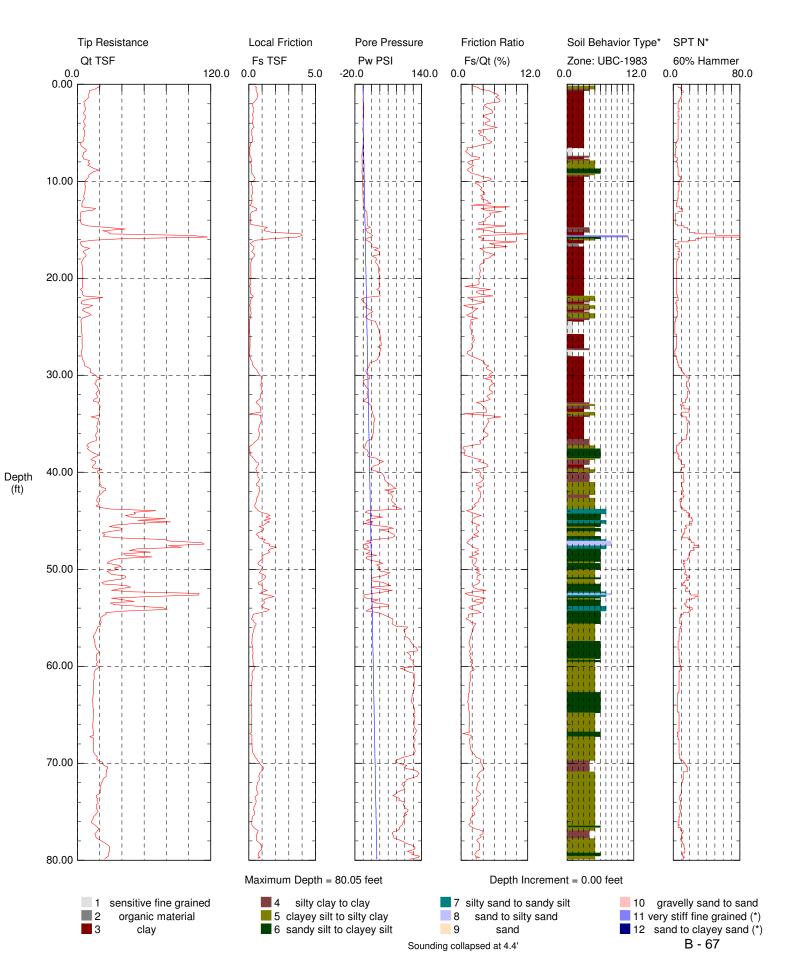
CPT Date/Time: 8/17/2007 7:55:26 AM Location: DNR Maurepas Diversion



Operator: Mike Wright
Sounding: CPT-06 ELEV. 4.50 ft

Cone Used: DSG0780

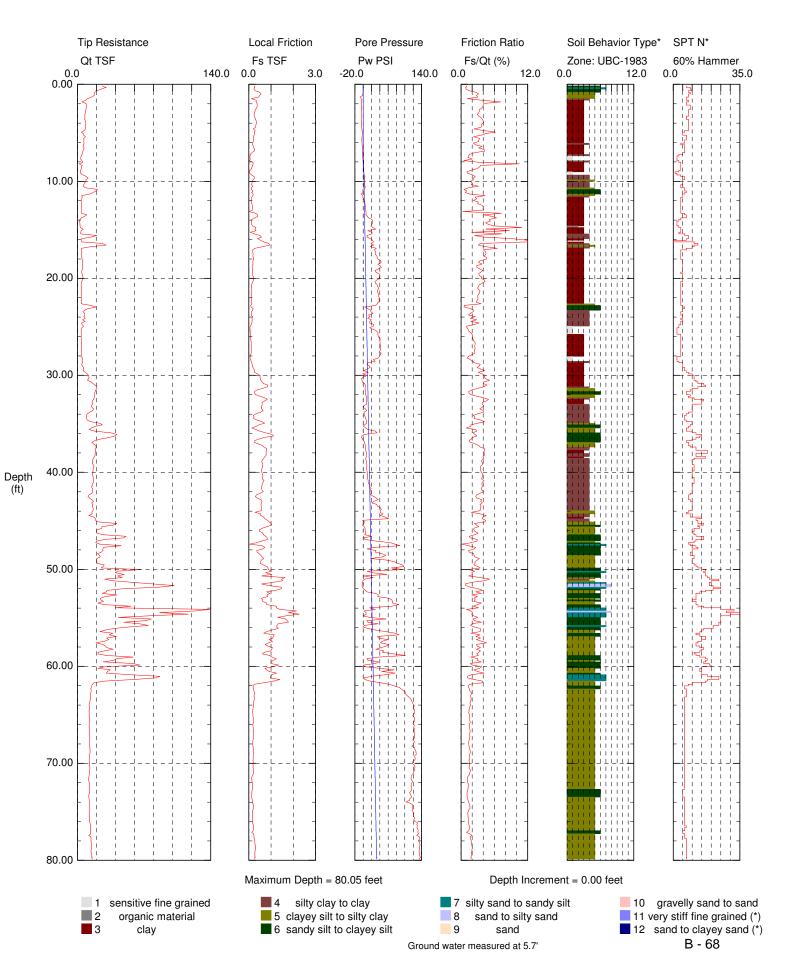
CPT Date/Time: 8/17/2007 8:52:37 AM Location: DNR Maurepas Diversion



Operator: Mike Wright Sounding: CPT-07 ELEV. 5.10 ft

Cone Used: DSG0780

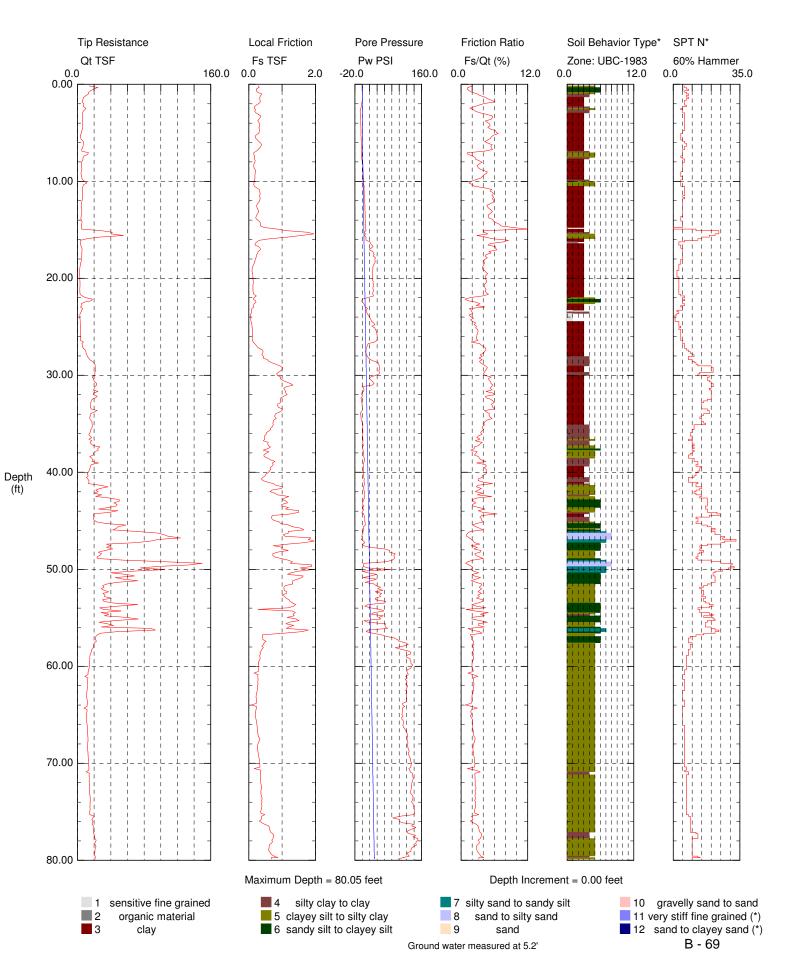
CPT Date/Time: 8/16/2007 9:54:21 AM Location: DNR Maurepas Diversion



Operator: Mike Wright Sounding: CPT-08 ELEV. 4.40 ft

Cone Used: DSG0780

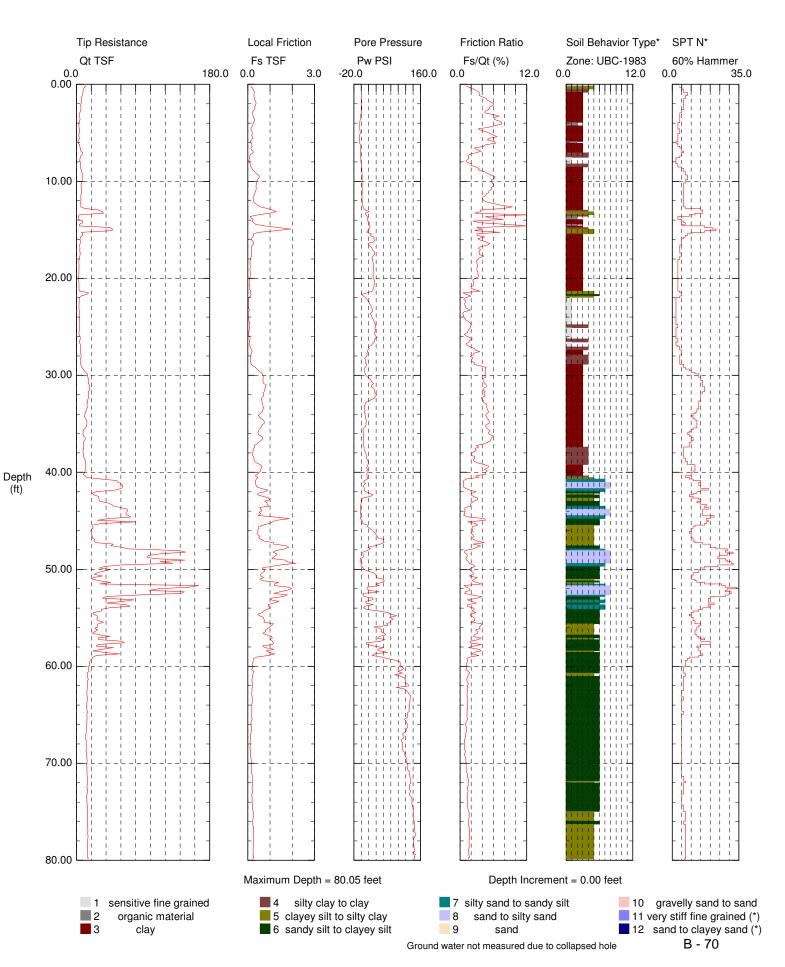
CPT Date/Time: 8/16/2007 8:52:55 AM Location: DNR Maurepas Diversion



Operator: Mike Wright Sounding: CPT-09 ELEV. 3.40 ft

Cone Used: DSG0780

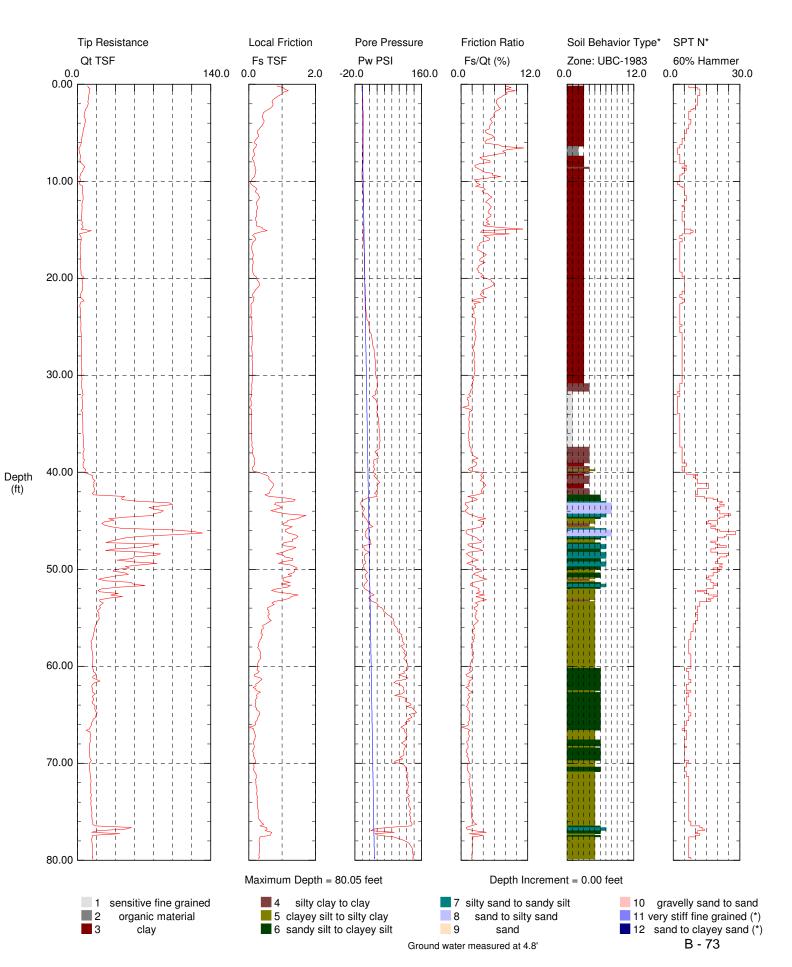
CPT Date/Time: 8/16/2007 7:34:08 AM Location: DNR Maurepas Diversion



Operator: Mike Wright Sounding: CPT-12 ELEV. 4.30 ft

Cone Used: DSG0780

CPT Date/Time: 8/21/2007 9:10:11 AM Location: DNR Maurepas Diversion



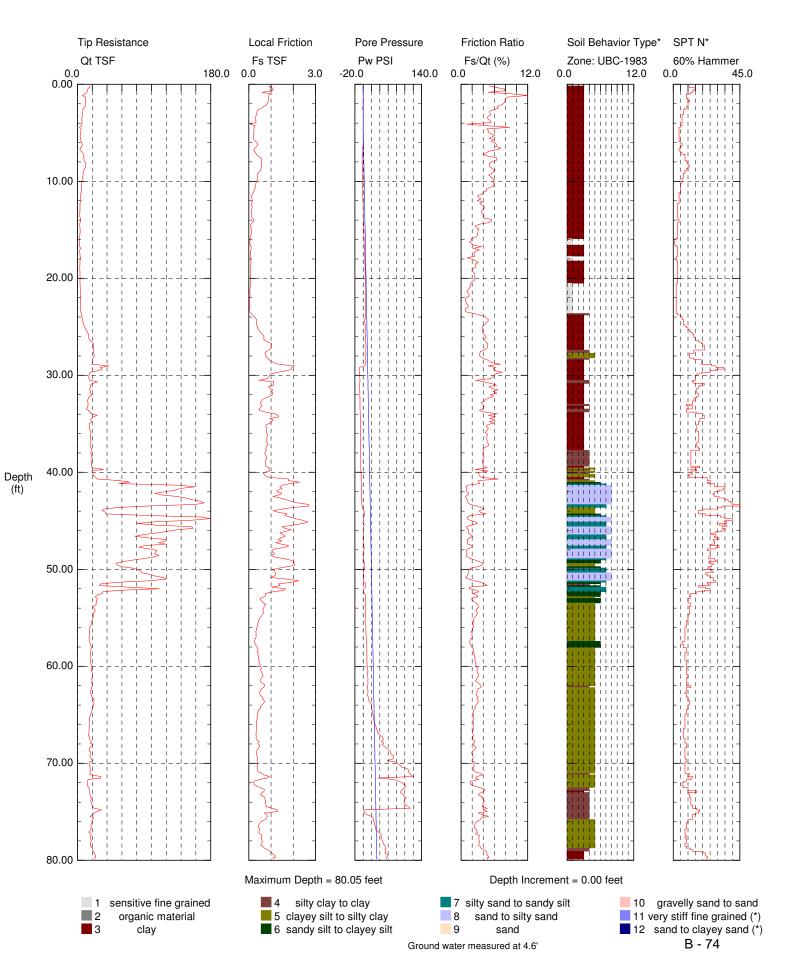
### Southern Earth Sciences, Inc

Operator: Mike Wright Sounding: CPT-13 ELEV. 3.40 ft

Cone Used: DSG0780

CPT Date/Time: 8/21/2007 3:39:55 PM Location: DNR Maurepas Diversion

Job Number: B07-163



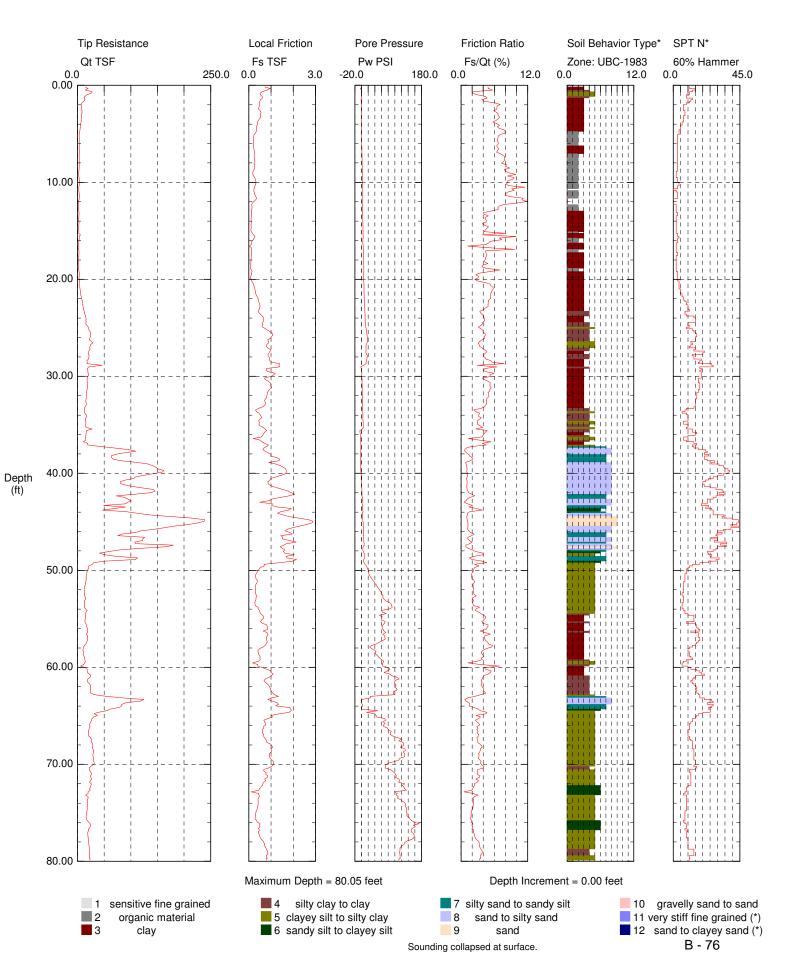
### Southern Earth Sciences, Inc

Operator: Mike Wright
Sounding: CPT-15 ELEV. 2.10 ft

Cone Used: DSG0780

CPT Date/Time: 8/21/2007 2:10:50 PM Location: DNR Maurepas Diversion

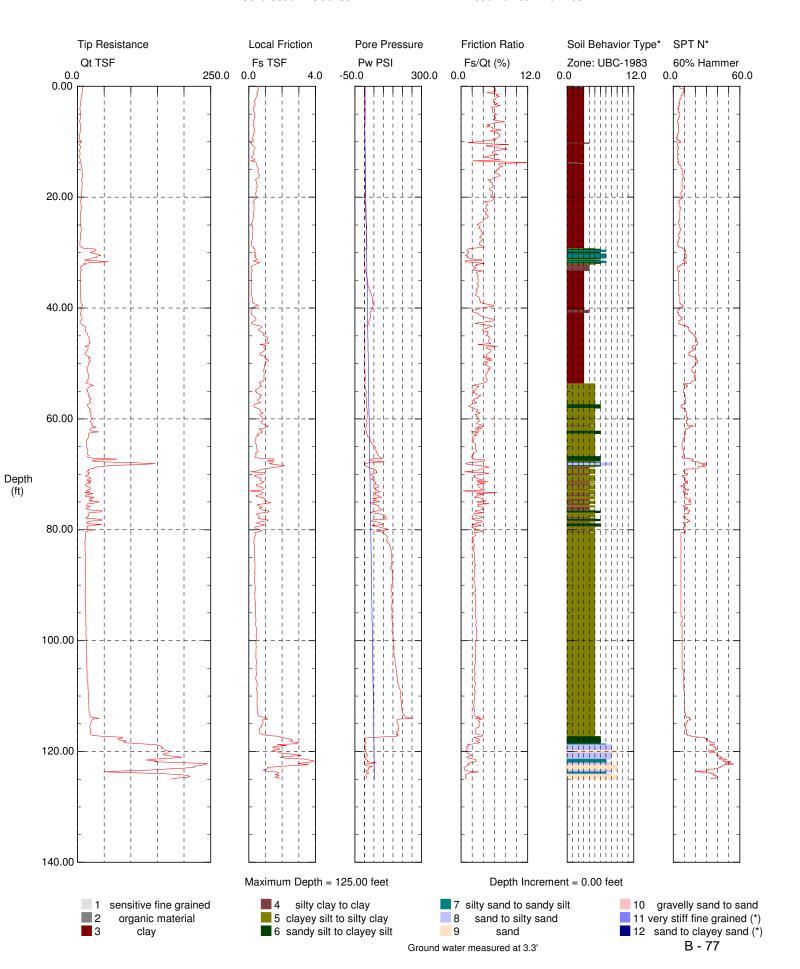
Job Number: B07-163



### Southern Earth Sciences, Inc

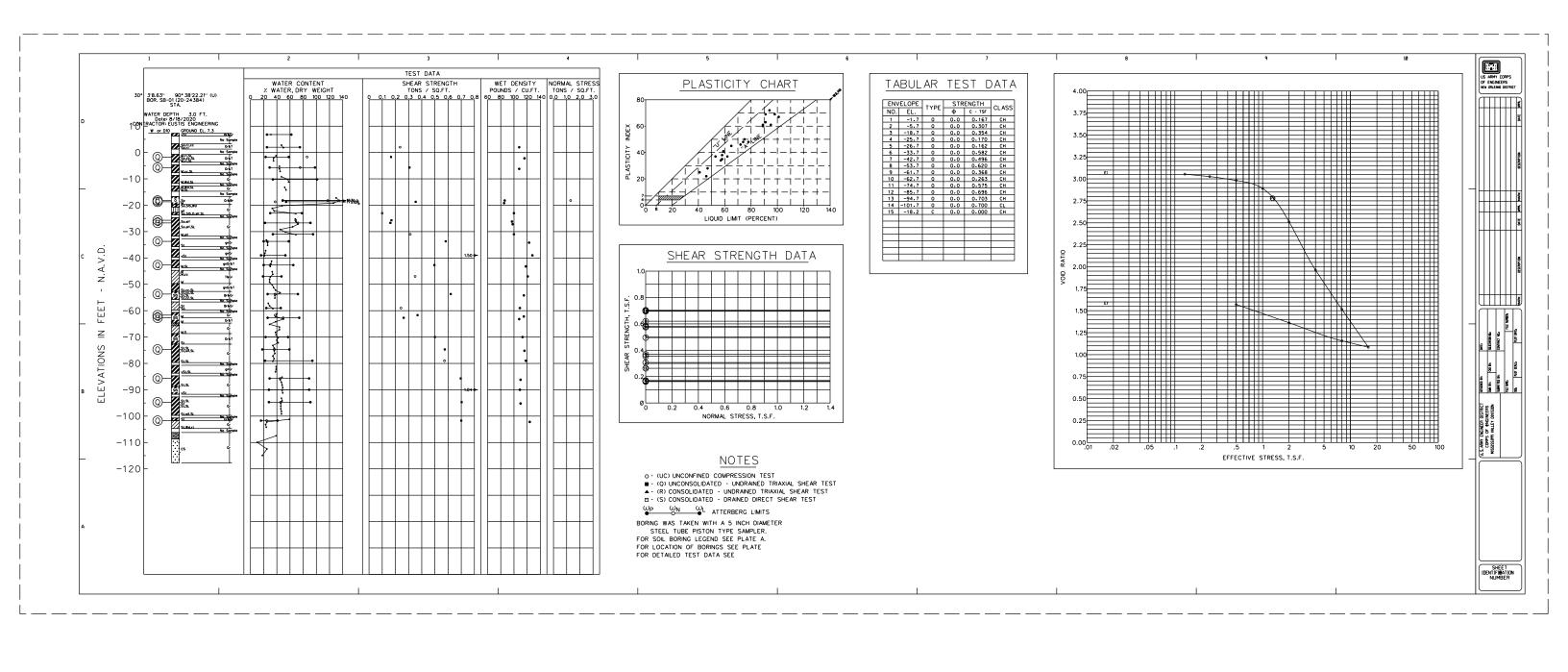
Operator: Mike Wright CPT Date/Time: 9/14/2007 1:52:25 PM

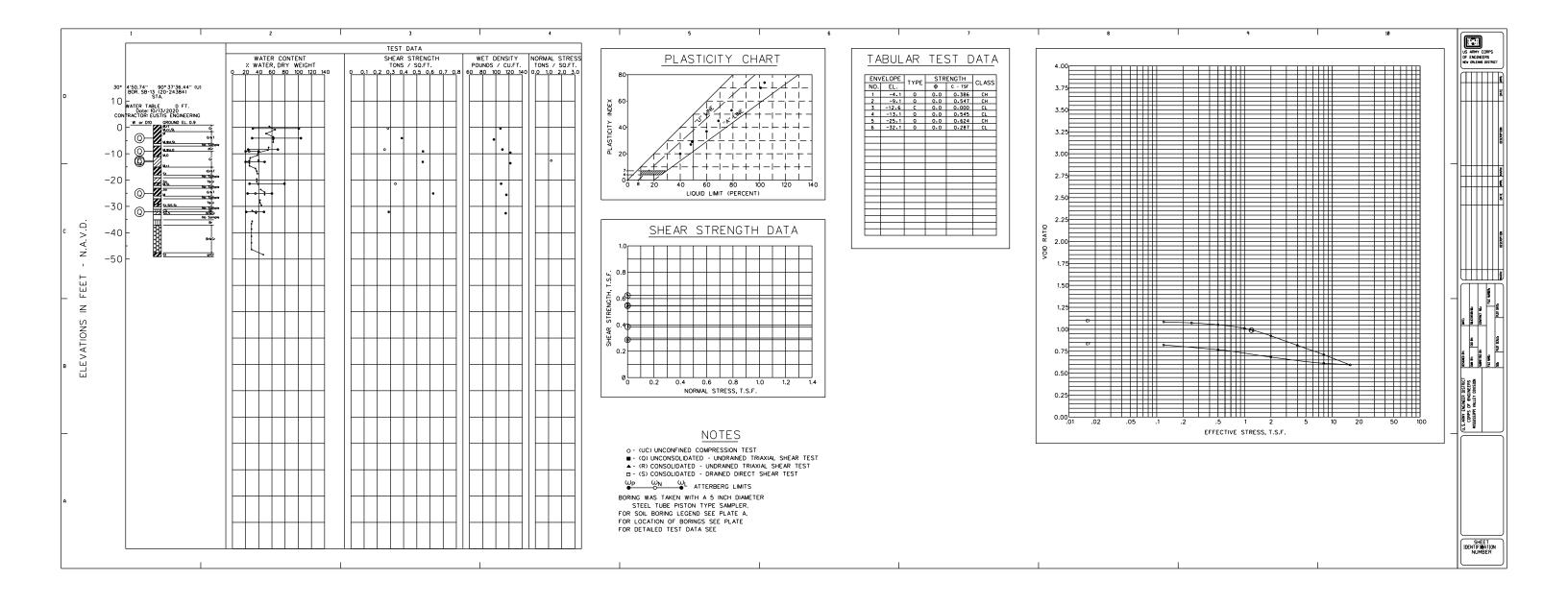
Sounding: CPT-16 ELEV. 8.01 ft Location: Lake Maurepas Cone Used: DSG0780 Job Number: B07-163

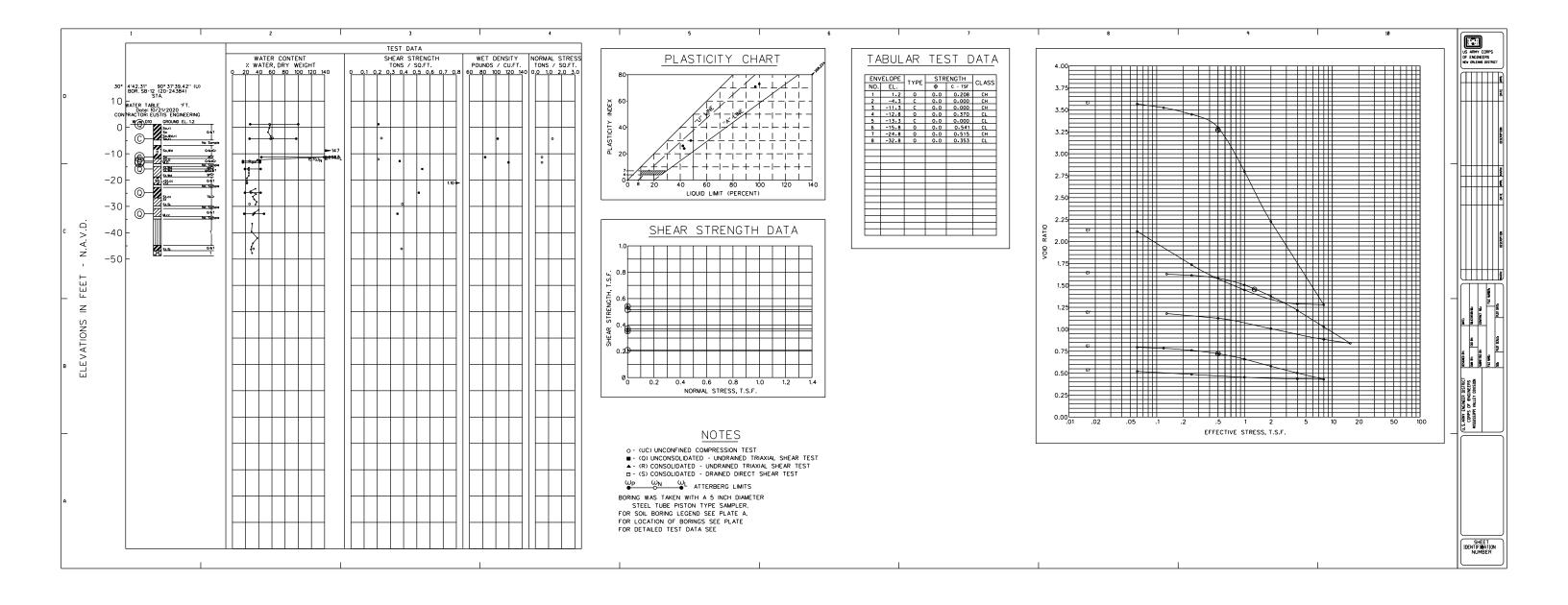


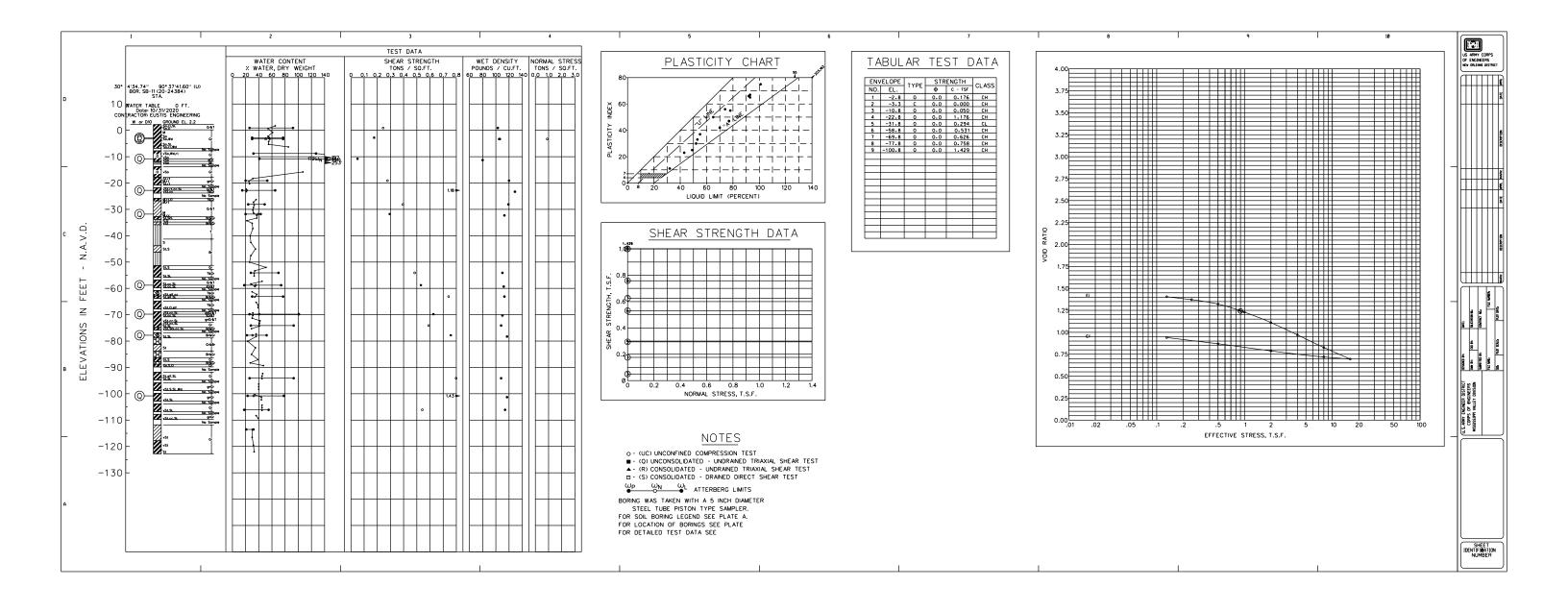
#### APPENDIX III

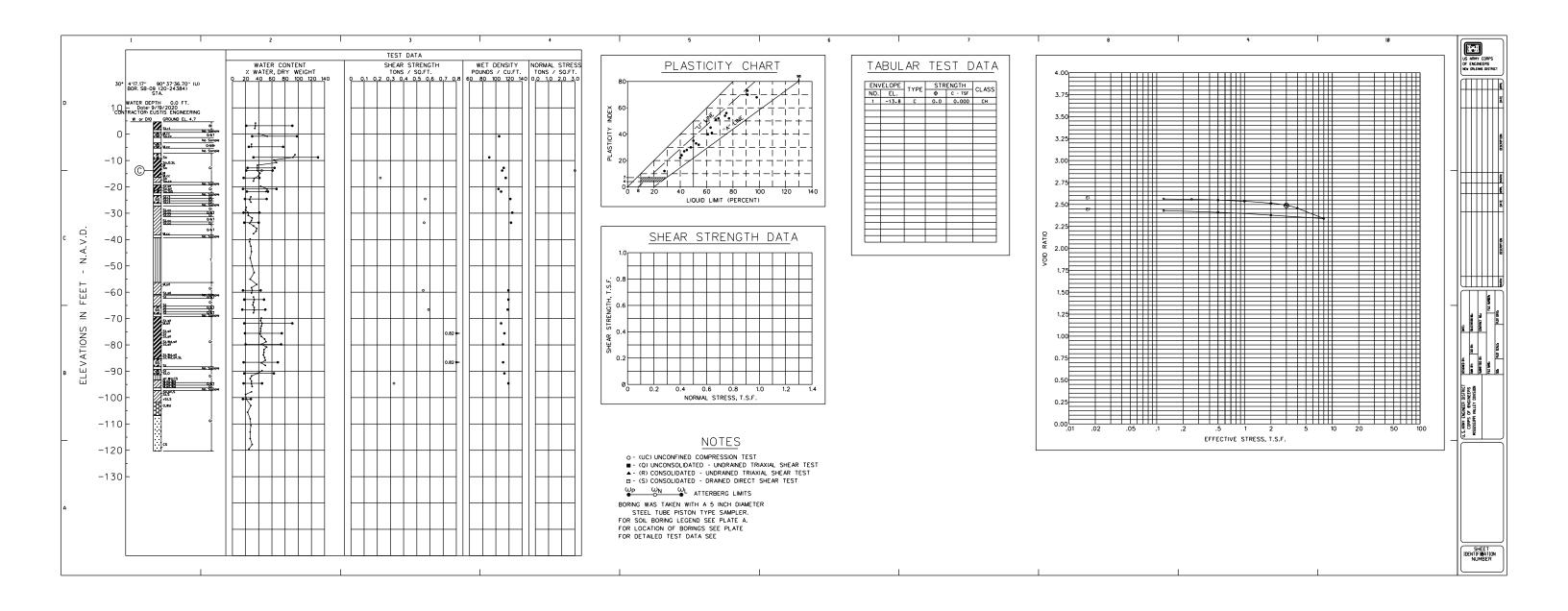
GEOTECHNICAL BORINGS – CURRENT EXPLORATION

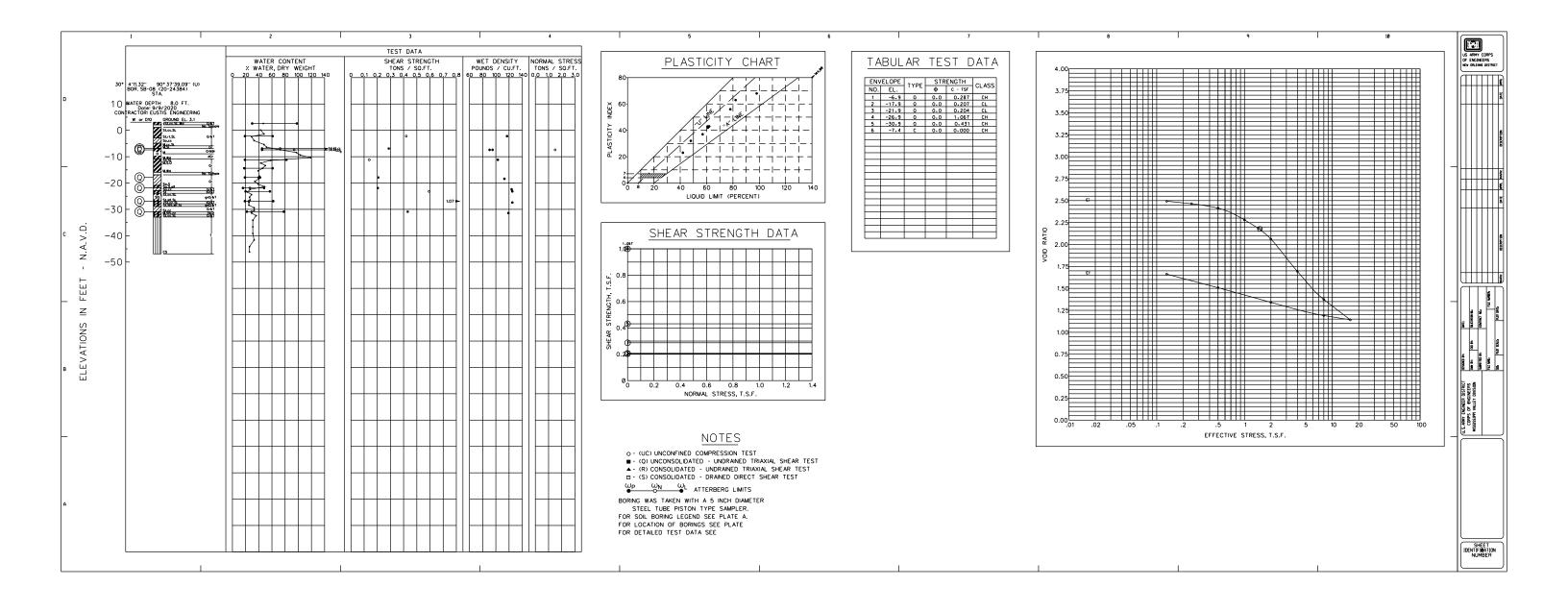


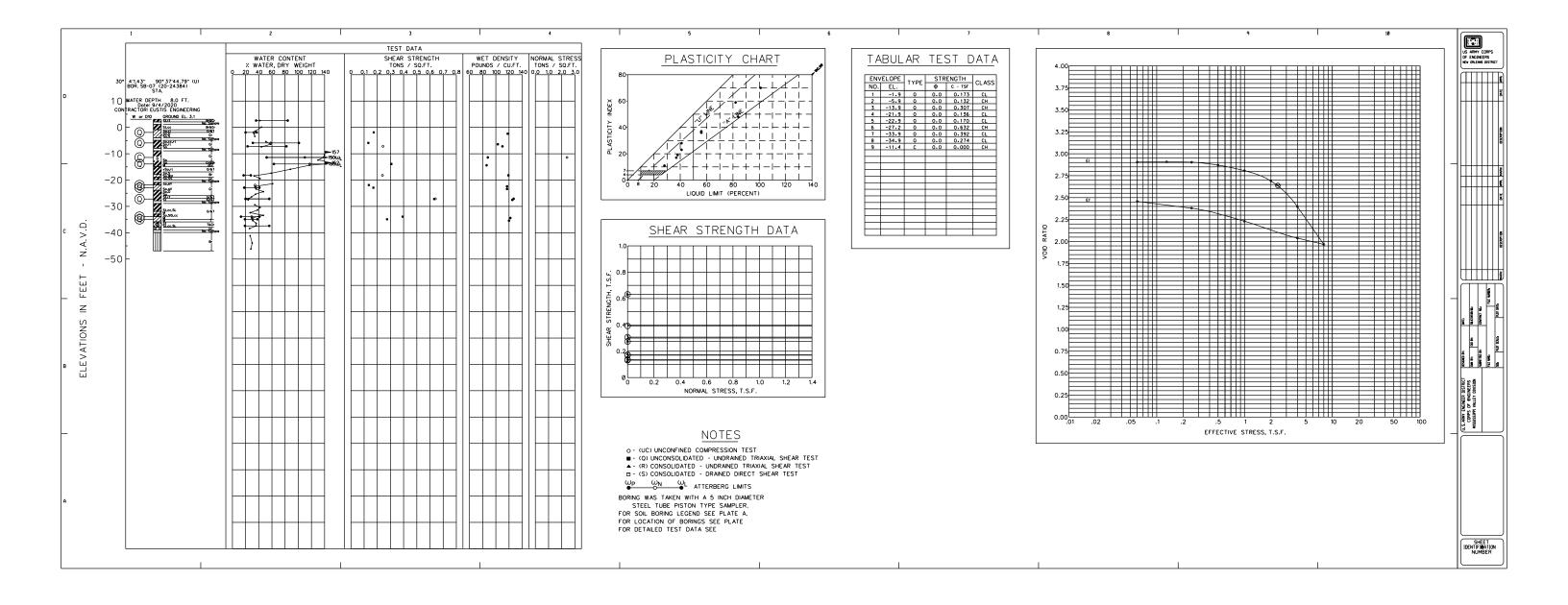


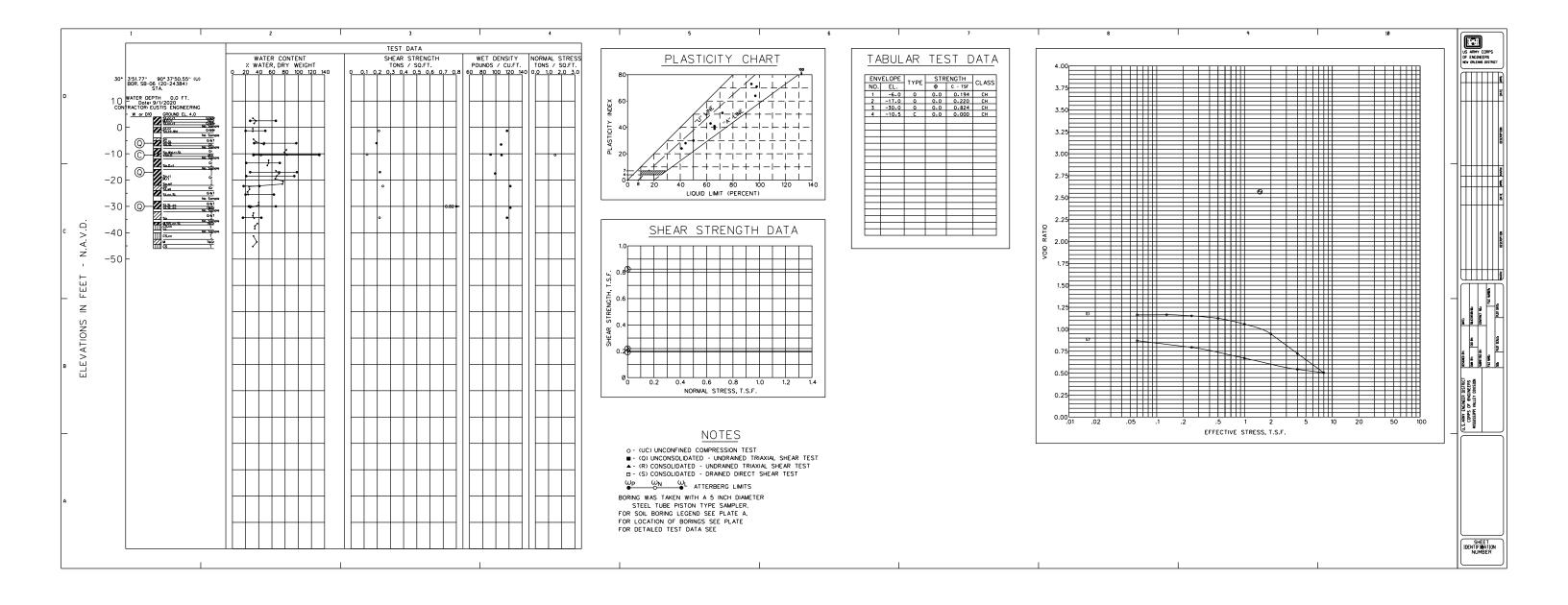


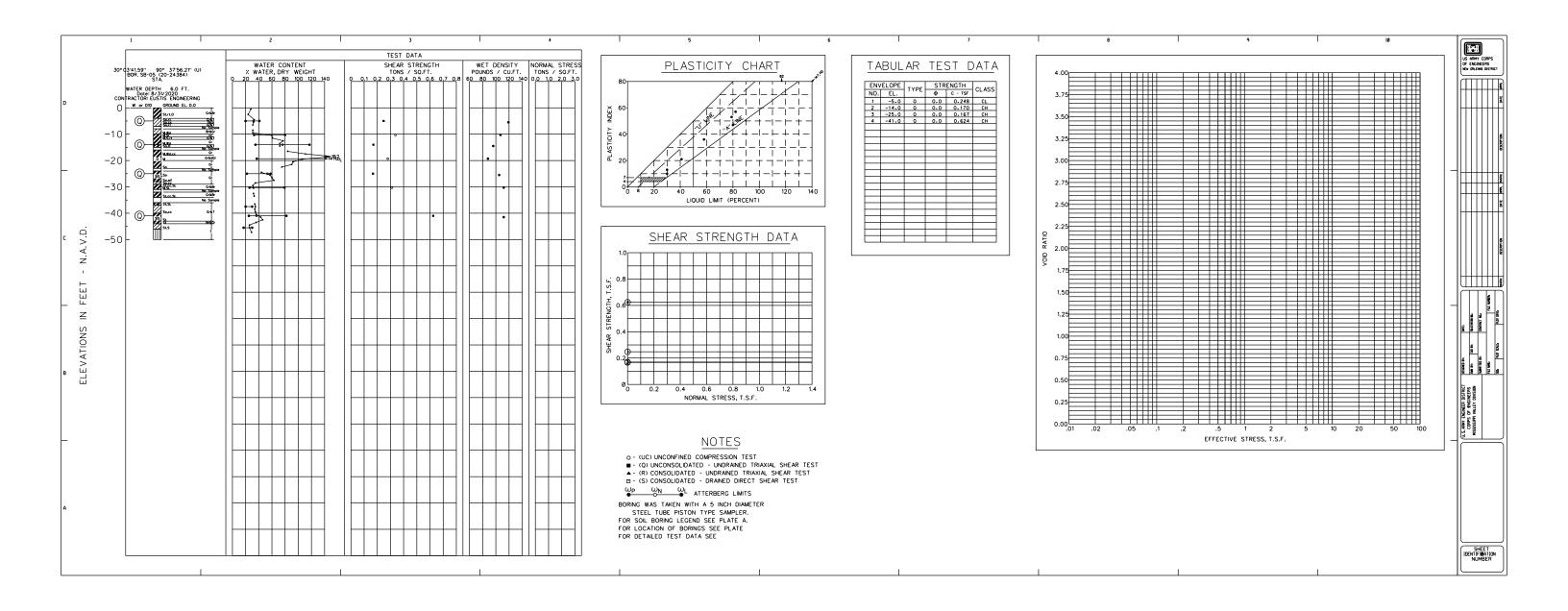


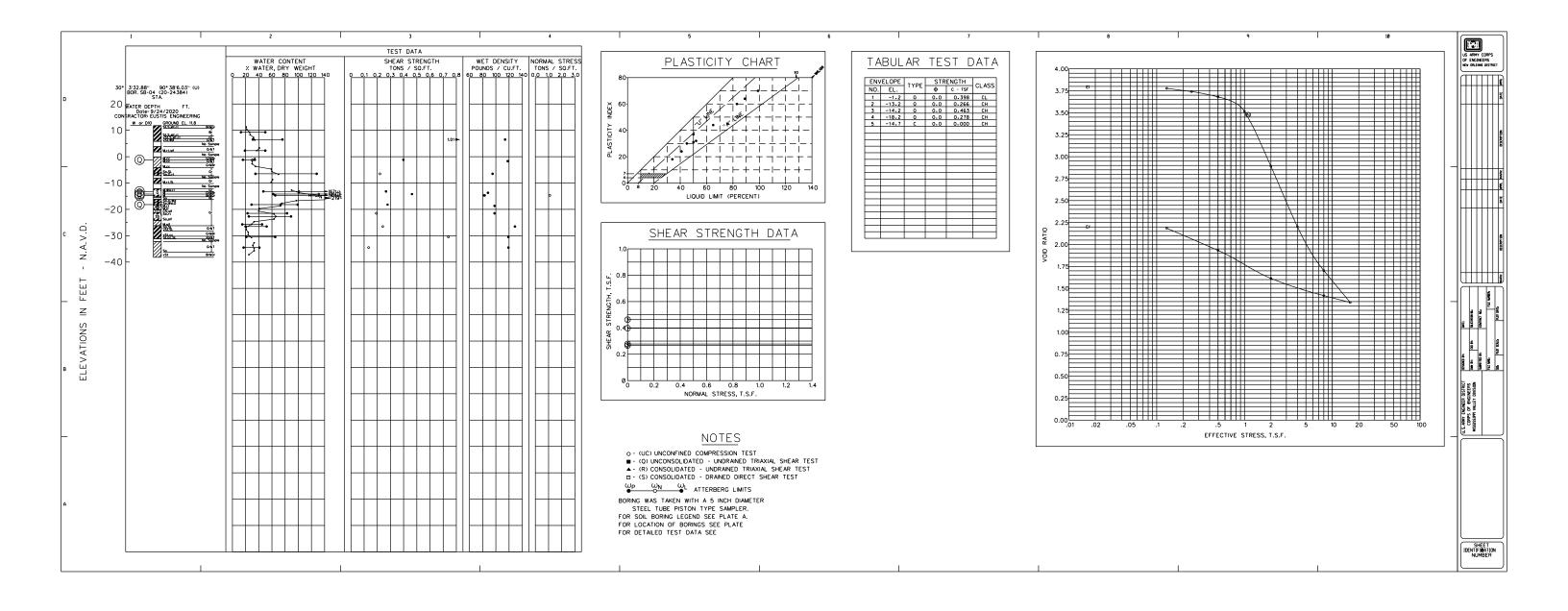


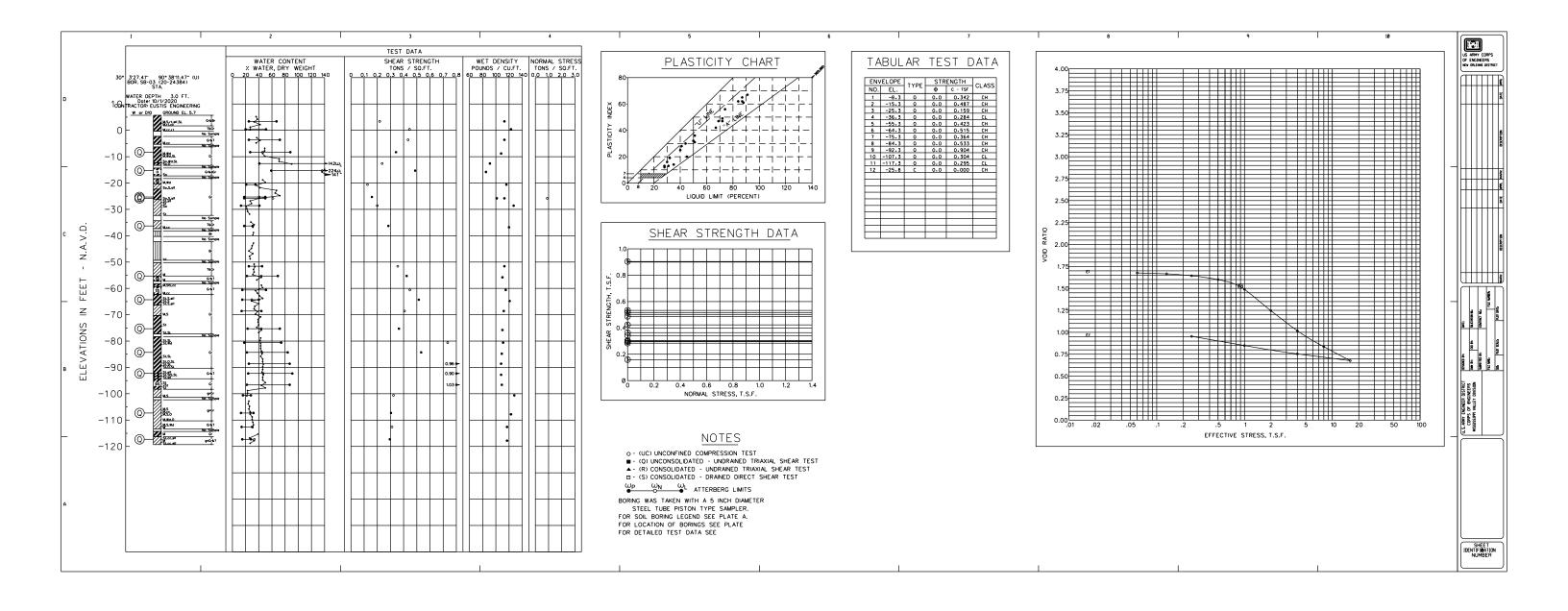


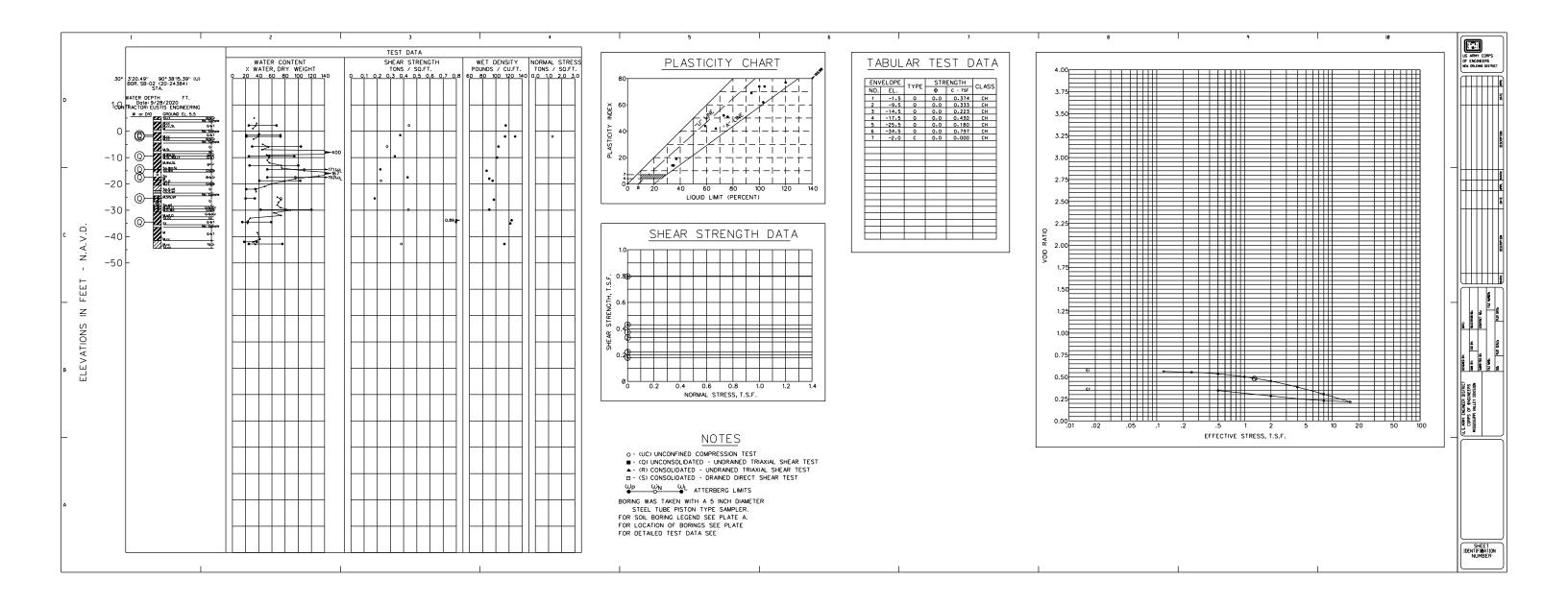












#### APPENDIX IV

GEOTECHNICAL CPTS – CURRENT EXPLORATION



### CONE PENETRATION SOIL BEHAVIOR TYPE LEGEND



\*OVERCONSOLIDATED OR CEMENTED

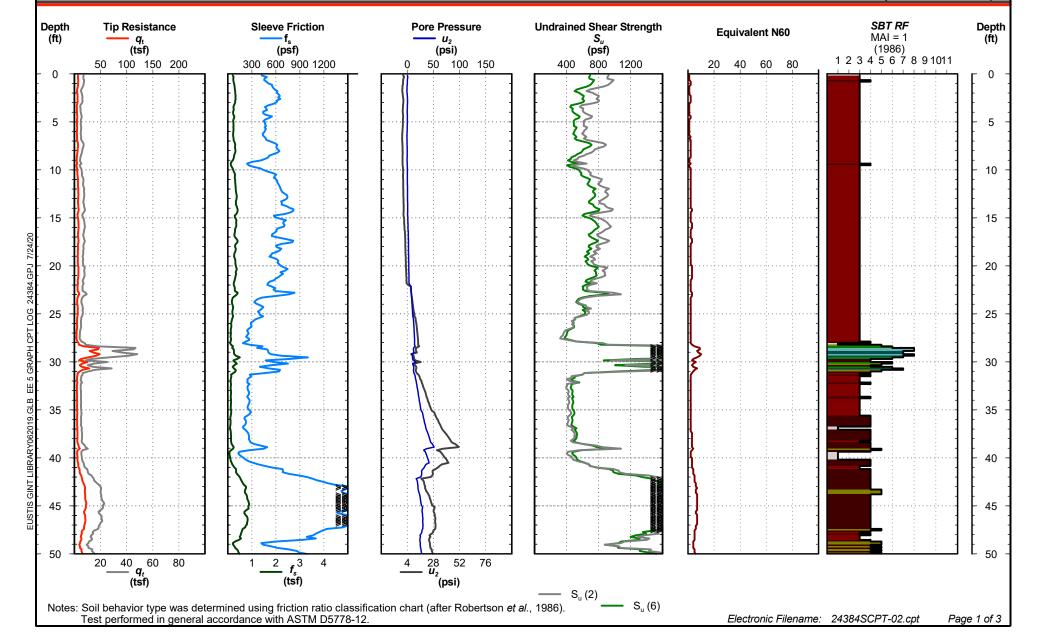


#### **CONE PENETRATION TEST**

SCPT-02

Project No: 24384 Date: 07/23/2020 Latitude:

Longitude: CPT ID: DSA1082 Est. Water Depth: 0.0 ft Total Depth: 113.9 ft Operator: G. Reitmeyer





#### **CONE PENETRATION TEST**

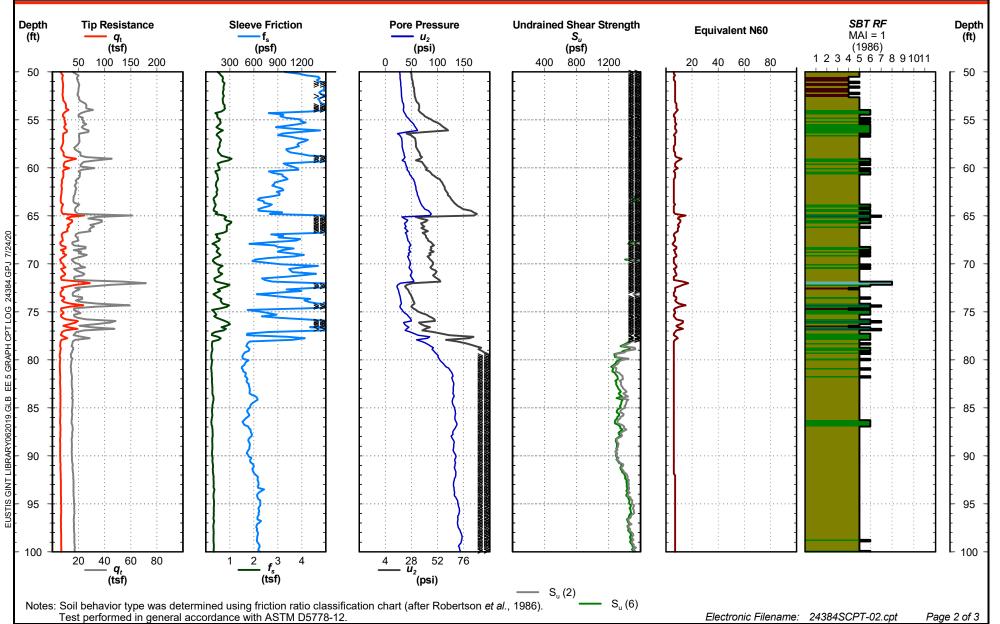
SCPT-02

Project No: 24384 Date: 07/23/2020 Latitude: Longitude:

CPT ID: DSA1082

**Total Depth:** 113.9 ft **Operator:** G. Reitmeyer

Est. Water Depth: 0.0 ft





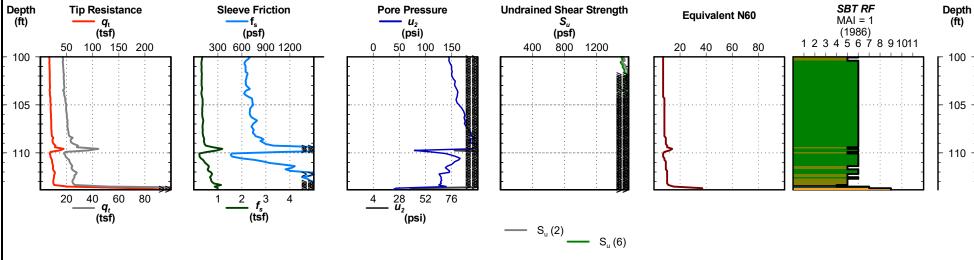
#### **CONE PENETRATION TEST**

SCPT-02

Project No: 24384 Date: 07/23/2020 Latitude: Longitude:

CPT ID: DSA1082

Est. Water Depth: 0.0 ft Total Depth: 113.9 ft Operator: G. Reitmeyer





State of Louisiana Coastal Protection and Restoration

**Authority (CPRA)** 

15% Design Phase

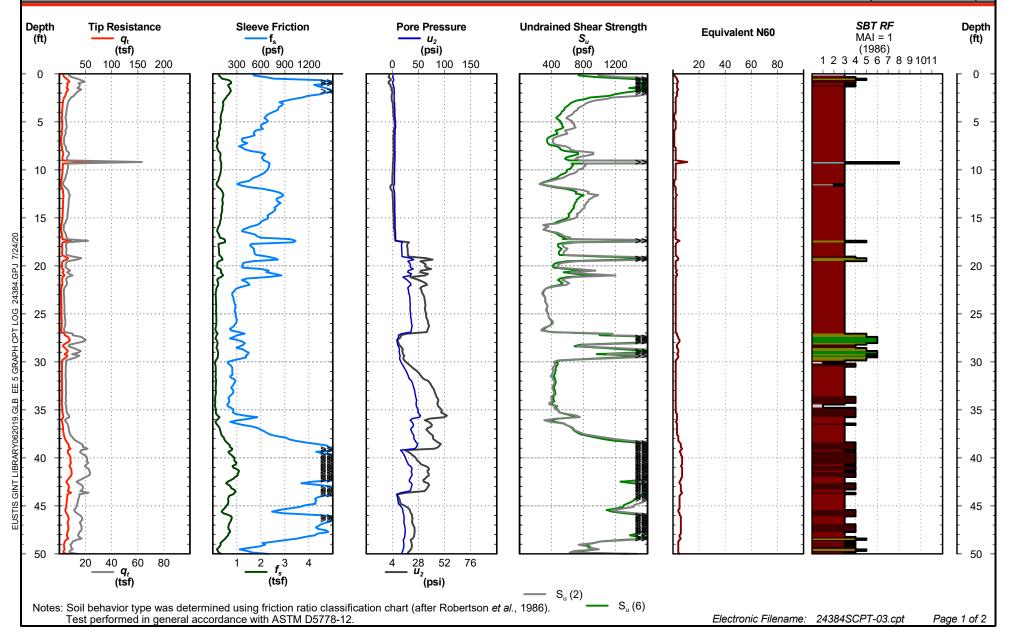
St. John the Baptist Parish, Louisiana

#### **CONE PENETRATION TEST**

SCPT-03

Project No: 24384 Date: 07/23/2020 Latitude:

Longitude: CPT ID: DSA1082 Est. Water Depth: 0.0 ft Total Depth: 60.2 ft Operator: G. Reitmeyer



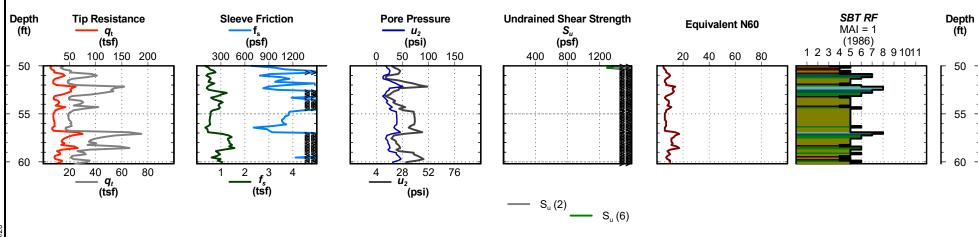
#### **CONE PENETRATION TEST**

SCPT-03

Project No: 24384 Date: 07/23/2020 Latitude:

Longitude: CPT ID: DSA1082

Est. Water Depth: 0.0 ft Total Depth: 60.2 ft Operator: G. Reitmeyer



### EUSTIS ENGINEERING SINCE 1946

## State of Louisiana Coastal Protection and Restoration Authority (CPRA) 15% Design Phase St. John the Baptist Parish, Louisiana

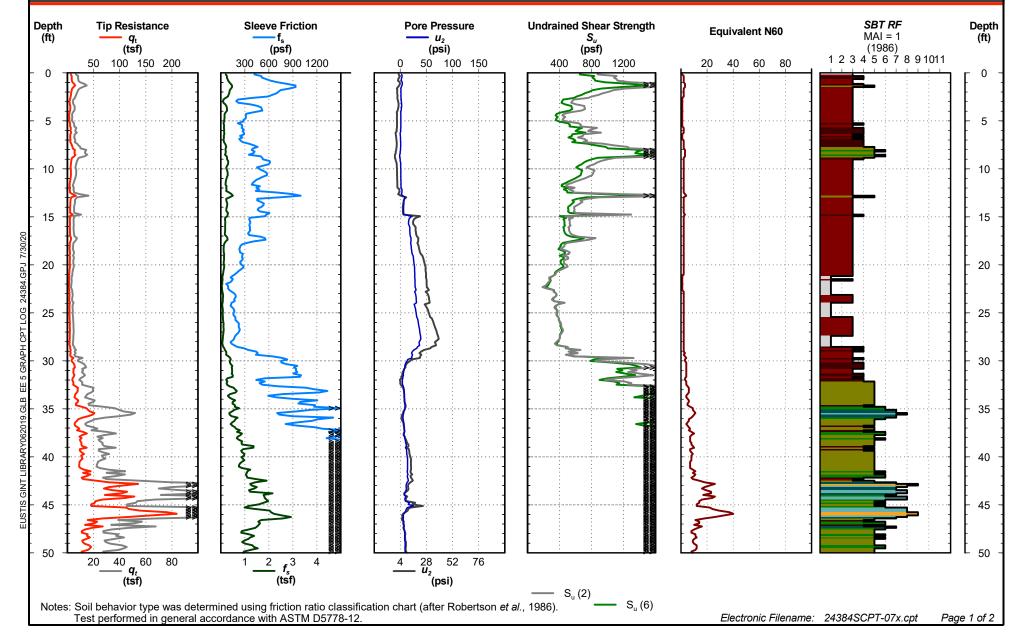
#### **CONE PENETRATION TEST**

SCPT-07x

Project No: 24384 Date: 07/28/2020 Latitude:

Longitude: CPT ID: DSA1082

Est. Water Depth: 0.0 ft Total Depth: 50.0 ft Operator: G. Reitmeyer



#### State of Louisiana **Coastal Protection and Restoration**

**Authority (CPRA)** 15% Design Phase St. John the Baptist Parish, Louisiana

**CONE PENETRATION TEST** 

SCPT-07x

Project No: 24384 Date: 07/28/2020

**Equivalent N60** 

20 40 60 80

Est. Water Depth: 0.0 ft Total Depth: 50.0 ft Operator: G. Reitmeyer

Depth

(ft)

50

SBT RF

MAI = 1

(1986)

1 2 3 4 5 6 7 8 9 1011

Latitude: Longitude: CPT ID: DSA1082

Depth (ft)	Tip Resistance —— q <sub>t</sub> (tsf)				
- 50 <del>-</del>	50	100	150	200	
- 50 -	20_	40 — (ts	60 (t s <b>f</b> )	80	

Undrained Shear Strength 
$$S_u$$
 (psf)  $400 800 1200$ 

### EUSTIS ENGINEERING SINCE 1946

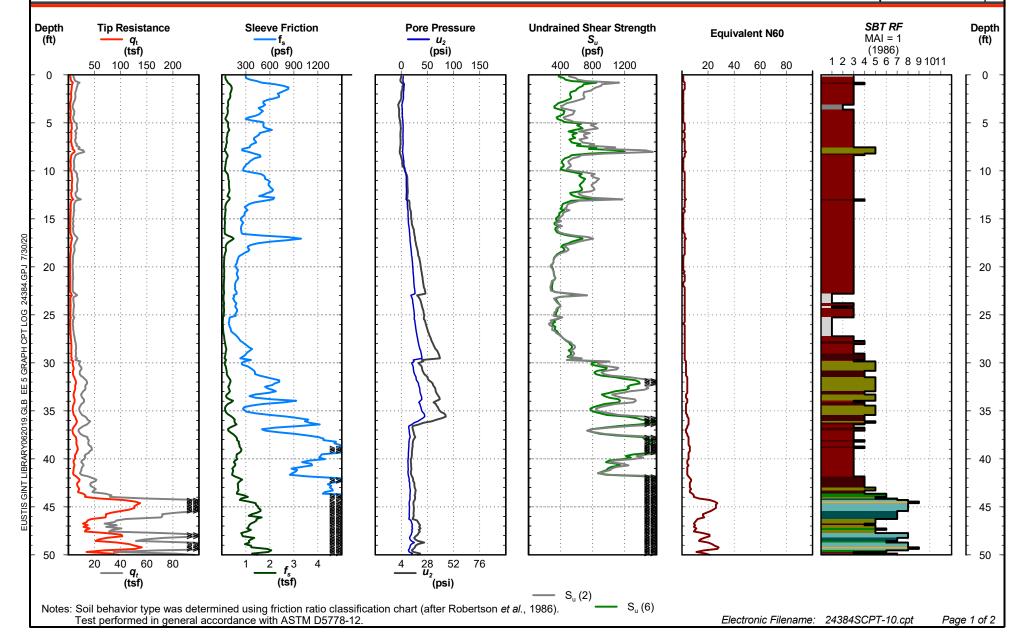
## State of Louisiana Coastal Protection and Restoration Authority (CPRA) 15% Design Phase St. John the Baptist Parish, Louisiana

#### **CONE PENETRATION TEST**

SCPT-10

Project No: 24384 Date: 07/28/2020 Latitude:

Longitude: CPT ID: DSA1082 Est. Water Depth: 0.0 ft Total Depth: 50.4 ft Operator: G. Reitmeyer



#### **CONE PENETRATION TEST**

SCPT-10

Project No: 24384 Date: 07/28/2020 Latitude:

Longitude: CPT ID: DSA1082 Est. Water Depth: 0.0 ft Total Depth: 50.4 ft Operator: G. Reitmeyer

Depth

(ft)

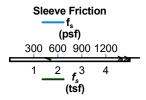
SBT RF

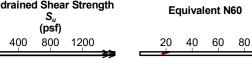
MAI = 1

(1986)

1 2 3 4 5 6 7 8 9 1011

Depth (ft)	Tip Resistance q <sub>t</sub> (tsf)					
- 50 <del>-</del> r	50 100 150 200					
30 <b>L</b>	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$					





— S<sub>u</sub> (2) — S<sub>u</sub> (6)

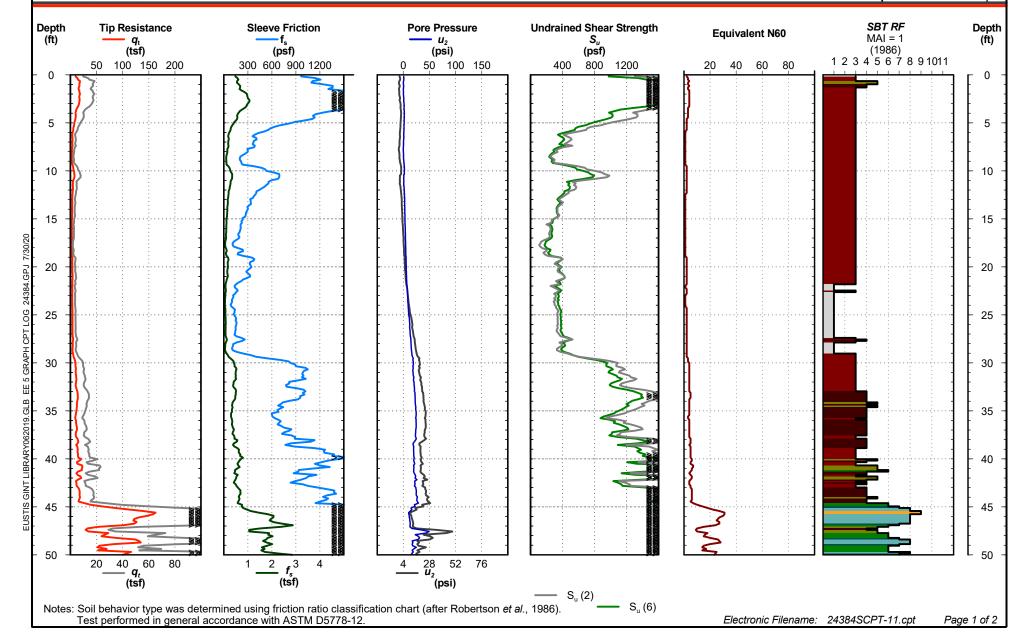


#### **CONE PENETRATION TEST**

SCPT-11

Project No: 24384 Date: 07/29/2020 Latitude:

Longitude: CPT ID: DSA1082 Est. Water Depth: 0.0 ft Total Depth: 99.9 ft Operator: G. Reitmeyer





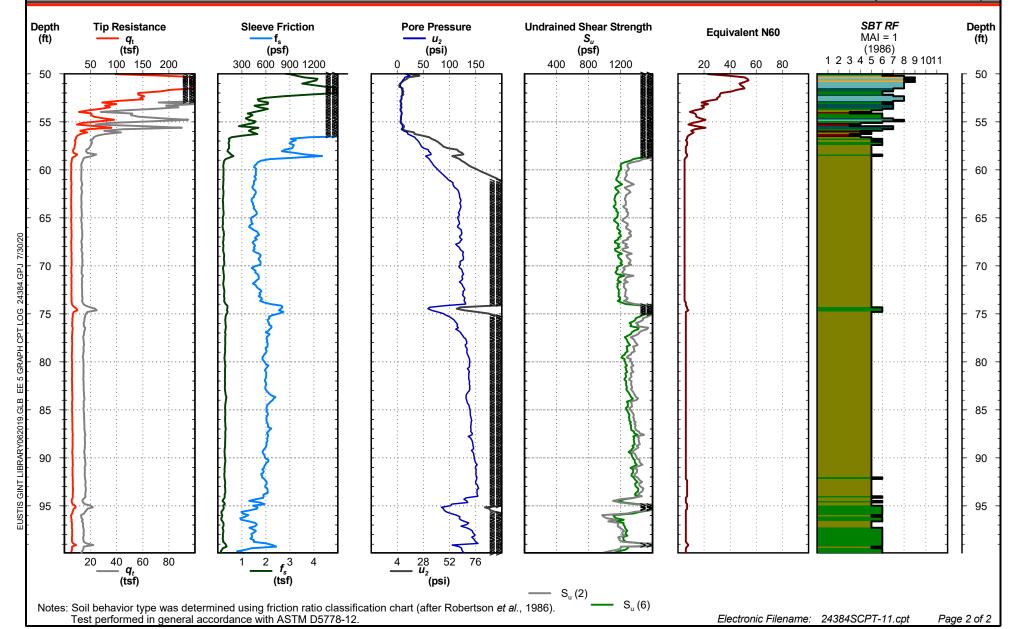
#### **CONE PENETRATION TEST**

SCPT-11

Project No: 24384 Date: 07/29/2020 Latitude: Longitude:

CPT ID: DSA1082

Est. Water Depth: 0.0 ft Total Depth: 99.9 ft Operator: G. Reitmeyer





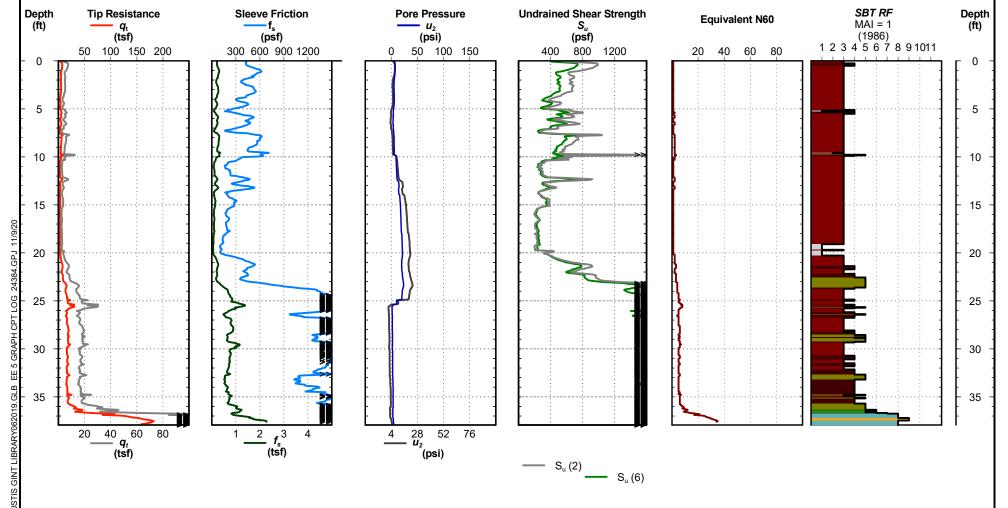
#### **CONE PENETRATION TEST**

SCPT-12

Project No: 24384 Date: 11/05/2020 Latitude: 30.07351° Longitude: -90.62723°

Longitude: -90.62723° Total Depth: 37.9 ft
CPT ID: 5389 Operator: GRR

Est. Water Depth: 0.0 ft





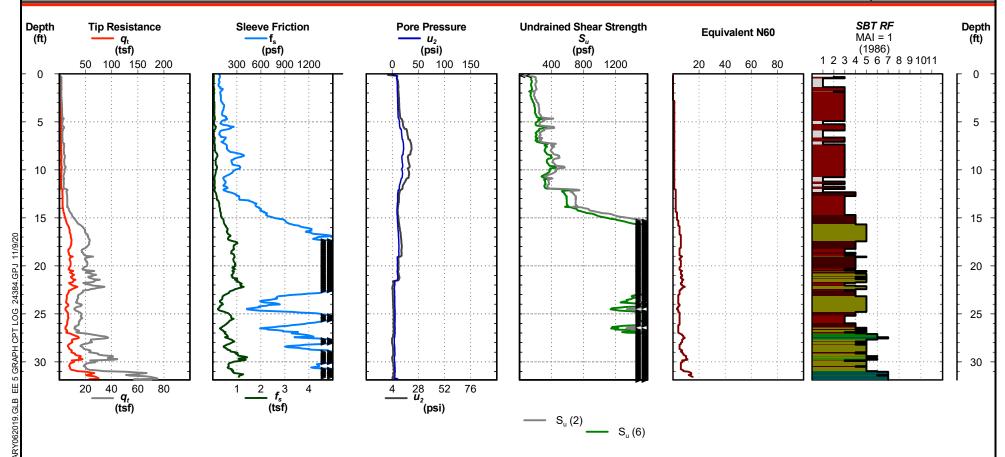
#### **CONE PENETRATION TEST**

SCPT-13

Project No: 24384 Date: 11/04/2020 Latitude: 30.07725° Longitude: -90.62805°

**CPT ID:** 5389

Est. Water Depth: 0.0 ft Total Depth: 31.9 ft Operator: GRR



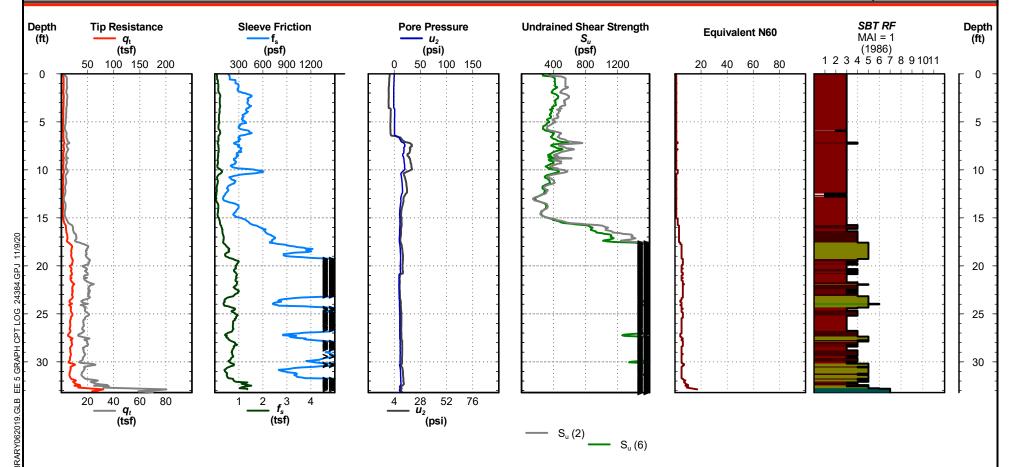


#### **CONE PENETRATION TEST**

SCPT-14

Project No: 24384 Date: 11/04/2020 Latitude: 30.07977° Longitude: -90.62709° CPT ID: 5389

Est. Water Depth: 0.0 ft Total Depth: 33.2 ft Operator: GRR



#### **CPT Correlations**

References are in parenthesis next to the appropriate equation.

#### General

p<sub>a</sub>=atmospheric pressure (for unit normalization)

q<sub>t</sub>=corrected cone tip resistance (tsf)

f<sub>s</sub>=friction sleeve resistance (tsf)

 $R_f = 100\% \cdot (f_s/q_t)$ 

u<sub>2</sub>=pore pressure behind cone tip (tsf)

u₀=hydrostatic pressure

$$\begin{split} \mathsf{B}_{\mathsf{q}} &= (\mathsf{u}_2\text{-}\mathsf{u}_0)/(\ \mathsf{q}_t\text{-}\sigma_{\mathsf{vo}}) \\ \mathsf{Q}_{\mathsf{t}} &= (\mathsf{q}_t\text{-}\sigma_{\mathsf{vo}})/\ \sigma'_{\mathsf{vo}} \\ \mathsf{F}_{\mathsf{r}} &= 100\% \cdot \mathsf{f}_{\mathsf{s}}/(\ \mathsf{q}_t\text{-}\sigma_{\mathsf{vo}}) \\ \mathsf{I}_{\mathsf{c}} &= ((3.47\text{-}\log\mathsf{Q}_{\mathsf{t}})^2 + (\log\mathsf{F}_{\mathsf{r}} + 1.22)^2)^{0.5} \\ \mathsf{I}_{\mathsf{SBT}} &= ((3.47\text{-}\log(\mathsf{q}_{\mathsf{c}}/\mathsf{p}_{\mathsf{a}}))^2 + (\log\mathsf{F}_{\mathsf{r}} + 1.22)^2)^{0.5} \\ \mathsf{I}_{\mathsf{c}\,\mathsf{J\&D}} &= \sqrt{\left\{3 - \log\left(Q_t \cdot (1 - B_q)\right\}^2 + \left[1.5 + 1.3 \cdot \log\left(F_r\right)\right]^2} \\ \mathsf{I}_{\mathsf{c}\,\mathsf{J\&B}} &= \sqrt{\left\{3 - \log\left(Q_t \cdot (1 - B_q) + 1\right\}^2 + \left[1.5 + 1.3 \cdot \log\left(F_r\right)\right]^2} \\ \end{split}$$

$$\overline{K_0}$$
 (1)  $K_0 = (1-\sin\phi)OCR^{\sin\phi}$ 

$$K_0(2)$$
  $K_0 = 0.1(Q_t)$ 

#### Stress History

OCR = 
$$\sigma_p'/\sigma'_{vo}$$

$$\begin{array}{lll} \text{OCR (1)} & \sigma_{p}{'} = 0.33(q_{t} - \sigma_{vo}) \text{ - clays} & 8 \\ \text{OCR (2)} & \sigma_{p}{'} = 0.53(u_{2} - u_{o}) \text{ - clays} & 9 \\ \text{OCR (3)} & \sigma_{p}{'} = 0.60(q_{t} - u_{2}) \text{ - clays} & 9 \\ \text{OCR (4)} & \text{OCR = 0.25 } Q_{t}^{1.25} \text{ - clays} & 37 \\ \end{array}$$

1

35

OCR (4) OCR = 
$$0.25 Q_t^{1.25}$$
 – clays

OCR (5) 
$$\text{OCR} = \left[ \frac{0.192*(q_t/p_a)^{0.22}}{(1-\sin(\phi')\cdot(\sigma'_{v_0}/p_a^{0.31})]} \right]^{\frac{1}{\sin(\phi'-0.27)}} - \text{ sands}$$
 35 
$$\text{OCR (6)} \quad \sigma_{p'} = .101 \cdot p_a^{0.102} \cdot G_{max}^{0.478} \cdot \sigma'_{v_0}^{0.420} - \text{ all soils}$$
 36

#### N-Value

OCR (5)

$$\overline{N_{60}} = (q_t/p_a)/[8.5(1-l_c/4.6)]$$

#### **Undrained Shear Strength**

S <sub>u</sub> (1)	$S_u = (u_2 - u_o)/N_u$	where $7 \le N_u \le 9$	10
S <sub>u</sub> (2)	$S_u = (q_t - \sigma_{vo})/N_{kT}$	where $15 \le N_{kT} \le 20$	11
S <sub>u</sub> (3)	$S_u = 0.091 * ((\sigma'_{vo}^{0.2}) * (q_t - \sigma_{vo})^{0.8}$		21
S <sub>u</sub> (4)	$S_u = (q_c - \sigma_{vo})/N_k$	where $15 \le N_k \le 20$	11
S <sub>u</sub> (5)	$S_u = q_t/N_c$	where $XXX \le N_c \le YYY$	
S <sub>u</sub> (6)	$S_u = q_c/N_c$	where $XXX \le N_c \le YYY$	

#### **Effective Cohesion**

c' = $0.02 * \sigma_p$ Drained Frict $\phi'(1)$ $\phi'(2)$ $\phi'(3)$ $\phi'(4)$	tion Angle $φ' = 17.6 + 11.0 \text{Log}[q_t/(σ_{vo}')^{0.5}]$ $φ' = \arctan[0.1 + 0.38 \text{Log}(q_t/σ_{vo}')]$ $φ' = 30.8 \text{Log}[(f_s/σ_{vo}') + 1.26]$ (for clays or sands)	38 1 13 14 24		
$\frac{\text{Unit Weight}}{\rho = \gamma/\gamma_w}$ $\rho = 0.8 \text{Log}(V_s)$	) V <sub>s</sub> in m/sec	17		
D <sub>R</sub> (1) D <sub>R</sub> (2) D <sub>R</sub> (3) D <sub>R</sub> (4)	sity and Void Ratio $ D_R = 100(q_{c1}/305)^{1/2} \qquad \text{where, } q_{c1} = q_c/(\sigma_{vo}/305)^{1/2} \\ D_R = -1.292 + 0.268ln(q_c \cdot (\sigma_{vo}'^{-0.5})) \\ D_R = (1/2.41) \cdot ln(q_{c1}/15.7) \\ D_R = 1/2.91 * ln((q_c/(61*\sigma'_{vo}^{0.71}))*100 \\ D_R = 100*(0.268*ln((q_t/p_a)/(\sigma'_{vo}/p_a)^{0.5}) - 0.675) $	1) <sup>1/2</sup> 1 18 3 20 34		
$e_0 = 1.099 - 0$	0.204log(q <sub>c1</sub> )	1		
$E_D = 5 q_t$	$I_D = 2.0 - 0.14(R_f)$ $K_D = E_D/(34.7 \cdot I_D \cdot \sigma_{vo}')$			
<u>Compressibility</u> $M(1) = R_m E_D$ where $R_m$ = function( $I_D$ , $K_D$ ) see the following table 22				
$I_D \le 0.6$ $I_D \ge 3$ $0.6 < I_D < 3$ $K_D > 10$ $R_M < 0.85$				
M (2) M (3) M (4)	$\begin{aligned} \mathbf{M} &= \mathbf{q_c} \cdot 10^{(1.09\text{-}0.0075D} \mathbf{R})  \textit{sands} \\ \mathbf{M} &= 8.25 \; (\mathbf{q_t} - \sigma_{vo}) \qquad \textit{clays} \\ M &= \alpha \cdot \textit{G}_{max} \; \textit{where} \; 0.02 < \alpha < 2 \; \text{and} \; \textit{G}_{max} \; \textit{is} \; \textit{from} \; \textit{Vs} \end{aligned}$	1 1 33		
Rigidity Index $I_R = exp\left[\left(\frac{1.5}{M} + 2.925\right) \cdot \left(\frac{q_t - \sigma_{vo}}{q_t - u_2}\right) - 2.925\right] \text{ where } M = 6\sin\phi'/(3 - \sin\phi')  39$				
Sensitivity St (1) St (2)	$S_t = 7.5/R_f$ $S_t = (q_t - \sigma_{vo})/(15 \cdot f_s)$	2 2		
Fines Conter FC = [(3.58-lo FC = [5.31(I <sub>cfs</sub>	$\frac{\text{nt}}{\text{g}(q_t))^2 + (1.43 + \log(R_f))^2]^{1.8}}$ $(1.95 - \log(Q_t))^2 + (\log(R_f))^2]^{0.5}$ $(1.95 - \log(Q_t))^2 + (\log(R_f))^2]^{0.5}$	4		

#### **Shear Wave Velocity**

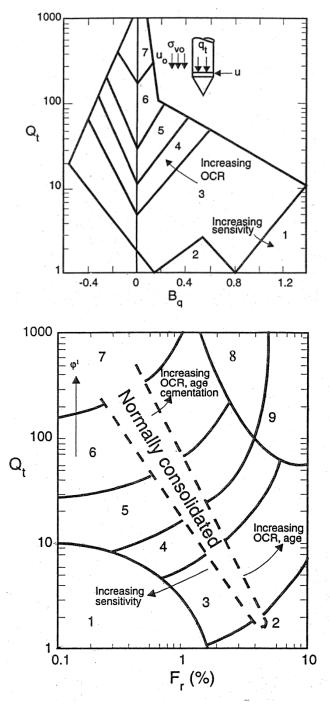
$$V_s(1) = 277 \cdot q_t^{0.13} \cdot \sigma_{vo}^{\prime 0.27}$$
 (sands) - m/s and MPa

$$V_s(2) = 1.75 \cdot q_t^{0.627}$$
 (clays) - m/s and kPa 30

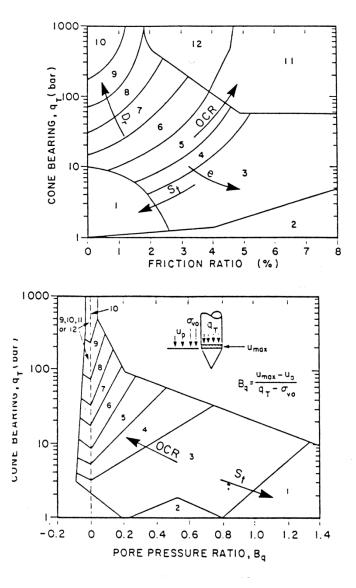
$$V_s(3) = (10.1 \cdot \log q_t - 11.4)^{1.67} \cdot (\frac{f_s}{q_t} \cdot 100)^{0.3} \qquad \text{(all soils) - m/s and kPa} \qquad 31$$
 
$$V_s(4) = 118.8 \cdot \log f_s + 18.5 \qquad \qquad \text{(all soils) - m/s and kPa} \qquad 32$$
 
$$G_{max} = \rho V_s^2$$

#### **Hydraulic Conductivity**

### Normalized Soil Behavior Types - Robertson & Campanella (1990)



# Non-Normalized Soil Behavior Types – Robertson & Campanella (1986)



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## APPENDIX V

COMPILATION OF LABORATORY TESTING REPORT SUMMARIES (SDR-A, SDR-B, SDR-C)

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-01

1A	Sample Number	Depth	Visual Classification	USCS	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
2A	1A	0	VST BR & GR CH3	СНЗ		30.8	-							62	25	37		
S	NS	1																
NS 6 NO SAMPLE 3A 8 M GR & T CH2 W/ ARS ML, RT, SL 3B 9 SO GR & T CH2 W/ ARS ML, RT, SL 3CH2 3CH2 3CH2 3CH2 3CH2 3CH2 3CH2 3CH2	1																	
NS 6 NO SAMPLE	2B	5	SO GR & T CH3 W/ ARS ML, RT	CH3	9	49.9	72.0	108	100	UC		471	942	75	25	50	0.375	
3A 8 M GR & T CH2 W ARS ML, RT, SL CH2 CH2 9 35.2 85.9 116 99 UU 0 334 738 58 23 35 0.200 ORG%. 3C 10 M GR & T CH2 W ARS ML, RT, SL CH2 34.8 NS 11 NO SAMPLE NS 11 NO SAMPLE NS 15 NO SAMPLE N	NS	6	NO SAMPLE	NIS														5.5%
38 9 SO GR & T CH2 W / ARS ML, CC, SL CH2 9 3.5.2 85.9 116 99 UU 0 33.4 73.8 58 23 35 0.200 ORG% 2.69%  3C 10 M GR & T CH2 W / ARS ML, RT, SL NO SAMPLE NS NS 11 NO SAMPLE NS NS 15 NO SAMPLE NS NS 15 NO SAMPLE NS NS 16 M GR CH4 W / ARS ML, WD, SL CH4 S4.3 NS NS 29 NO GR & BR CHOB CHOB 129.7 TA 24 SO GR & BR CHOB CHOB 135.4 NS SO GR & BR CHOB CHOB SNS NS 22 NO SAMPLE NS NS SO GR & BR CHOB CHOB SNS NS 22 NO SAMPLE NS NS NS 20 NG & BR CHOB CHOB CHOB SNS NS 20 NG SABR CHOB CHOB CHOB SNS NS 20 NG SAMPLE NS NS NS 20 NG SABR CHOB CHOB CHOB SNS NS 20 NG SAMPLE NS NS NS NS 20 NG CH4 W / ARS ML, WIJF NL NG NS NS 20 NG CH4 W / ARS ML, SIF NS NS 20 NG CH4 W / ARS ML, SIF CH4 67.6 NS 20 NG NG CH4 W / ARS ML, SIF CH4 67.6 NS 20 NG NG NG W / ARS ML, SIF CH4 3 8.87 57.5 97.0 96 UU 0 340 UU 0 7.50 150 150 150 150 150 150 150 150 150 1				-														
3C 10 M GR & T CH2 W / ARS ML, RT, SL NS 11 NO SAMPLE NS	_	1			9		85.9	116	99	UU	0	334	738	58	23	35	0.200	ORG% =
NS					•				**									2.69%
4A 12 M GR & T CH4 W/ARS ML, CC, SL CH4 L4 44.4						34.8												
AB	_	1		-														
4C 14 M GR & T CH4 W/ ARS ML, CC, SL NS 15 NO SAMPLE NS 15 NO SAMPLE NS 15 NO SAMPLE NS 16 M GR CH4 W/ ARS ML, WD, SL CH4 46.5 SB 17 M GR CH4 W/ ARS ML, WD, SL CH4 56.5 SC 18 M GR CH4 W/ ARS ML, WD, SL CH4 56.5 SC NS 19 NO SAMPLE NS NS 19 NO SAMPLE NS NS 22 NO SAMPLE NS 20 GR & BR CHOB CH0B 129.7 TA 24 SO GR & BR CHOB CHOB 135.4 SO GR & BR CHOB CH0B 135.4 SO GR CH3 W/ ARS & LNS ML, WD CH3 ARS & LNS ML, WD CH4 GRAD ARS ML, SIF C		1									_							
4C 14 M GR & T CH4 W / ARS ML, CC, SL NS 15 NO SAMPLE NS CH4 NS 15 NO SAMPLE NS CH4 W / ARS ML, WD, SL CH4 S5.5 17 M GR CH4 W / ARS ML, WD, SL CH4 S6.5 NS 19 NO SAMPLE NS CH4 W / ARS ML, WD, SL CH4 S6.5 NS 19 NO SAMPLE NS CH4 W / ARS ML, WD, SL CH4 S6.5 NS 19 NO SAMPLE NS CH4 W / ARS ML, WD, SL CH4 S6.3 NS 22 NO SAMPLE NS CH4 W / ARS ML, WD, SL CH4 S6.3 NS 22 NO SAMPLE NS CH0B CH0B 129.7 A 24 SO GR & BR CH0B CH0B 135.4 SO GR CH3 W / ARS & LNS ML, WD CH3 A 33.1 NO SAMPLE NS CH3 SO GR CH4 W / ARS & LNS ML, WD CH3 NS SV SV SO GR CH4 W / ARS & LNS SM, O, SIF, SL NS ML, SIF CH4 66 69.1 So.9 99.6 100 UC 200 401 78 30 48 0.110 NS AND SAMPLE NS CH4 W / ARS ML, SIF CH4 67.6 So.9 SO GR CH4 W / ARS ML, SIF CH4 67.5 So.9 SO GR CH4 W / ARS ML, SIF CH4 67.5 So.9 So.0 GR CH4 W / ARS ML, SIF CH4 67.5 So.9 So.0 So. CH4 W / ARS ML, SIF CH4 67.5 So.9 So.0 So. CH4 W / ARS ML, SIF CH4 67.5 So.9 So.0 So. CH4 W / ARS ML, SIF CH4 67.5 So.9 So.0 So. CH4 W / ARS ML, SIF CH4 67.5 So.9 So.0 So. CH4 W / ARS ML, SIF CH4 67.5 So.9 So.0 So. CH4 W / ARS ML, SIF CH4 67.5 So.9 So.0 So.0 So.0 So.0 So.0 So.0 So.0 So.0	4B	13	M GR & T CH4 W/ ARS ML, CC, SL	CH4	14	43.8	75.4	108	95	UU	0	614		98	29	69	0.375	
NS	10	14	M GR & T CH4 W/ ARS ML CC SI	СНЛ		117												3.72%
5A 16 M GR CH4 W/ ARS ML, WD, SL CH4 CH4 55.5	1					l												
5B	_					46.5												
SC	5B			CH4		56.5								101	34	67		ORG% =
NS																		4.33%
6A 20 M GR CH4 W/ ARS ML, SL CH4 CH4 M GR CH4 W/ ARS ML, WD, SL CH4 CH4 NS 22 NO SAMPLE NS 22 NO SAMPLE NS SO GR & BR CHOB CHOB 129.7  7A 24 SO GR & BR CHOB CHOB 135.4  7C 26 M GR & BR CHOB CHOB CHOB 4 126.2 37.4 84.6 98 UU 0 707 172 54 118 0.250  7D 27 SO GR CH3 W/ ARS & LNS ML, WD CH3 AS & LNS ML, WD CH3 AS & LNS ML, WD SIF ML 33.1 ML 33.1 ML 35.6 8C 30 SO GR CH4 W/ ARS & LNS SM, O, SIF, SL CH4 6 69.1 NS 31 NO SAMPLE NS SO GR CH4 W/ ARS ML, SIF CH4 67.6 98 UU 0 340 0 340 0 0.150																		
6B	1	1																
NS	1 -		·			-												
7A         24         SO GR & BR CHOB         CHOB         129.7           7B         25         SO GR & BR CHOB         CHOB         129.7           7C         26         M GR & BR CHOB         CHOB         4           7C         26         M GR & BR CHOB         CHOB         4           7D         27         SO GR CH3 W/ ARS & LNS ML, WD         CH3           8A         28         GR ML W/ SIF         ML         33.1           8C         30         SO GR CH4 W/ ARS & LNS SM, O, SIF, SL         CH4         6         69.1         58.9         99.6         100         UC          200         401         78         30         48         0.110           NS         31         NO SAMPLE         NS          CH4         67.6         67.6         67.6         67.6         67.6         67.6         67.6         67.6         67.6         67.5         97.0         96         UU         0         340         0.150         0.150						54.5												
TR	1	1				129 7												
7C		1				_								163	49	114		CONS,
7C         26         M GR & BR CHOB         CHOB         4         126.2         37.4         84.6         98         UU         0         707         172         54         118         0.250           7D         27         SO GR CH3 W/ ARS & LNS ML, WD         CH3         47.7         47.7         47.7         47.7         88         28         GR ML W/ SIF         ML         33.1         33.1         35.6         80         98         UU         0         707		-																ORG =
7D         27         SO GR CH3 W/ ARS & LNS ML, WD         CH3         47.7         SO GR CH3 W/ ARS & LNS ML, WD         CH3         47.7         SO GR CH3 W/ ARS & LNS ML, WD         SO GR CH3 W/ ARS & LNS ML, SIF         ML         33.1         SO GR CH3 W/ ARS & LNS SM, O, SIF, SL         ML         35.6         SO GR CH4 W/ ARS & LNS SM, O, SIF, SL         CH4         6 69.1         58.9         99.6         100         UC          200         401         78         30         48         0.110           NS         31         NO SAMPLE         NS          67.6         67.6         67.6         67.6         98         UU         0         340         0.150         0.150																		20.7%
8A 28 GR ML W/ SIF 8B 29 GR ML W/ SIF 8C 30 SO GR CH4 W/ ARS & LNS SM, O, SIF, SL NS 31 NO SAMPLE 9A 32 SO GR CH4 W/ ARS ML, SIF 9B 33 SO GR CH3 W/ ARS ML, SIF  NL 33.1		1			4		37.4	84.6	98	UU	0	707		172	54	118	0.250	
8B		1	· ·															
8C   30   SO GR CH4 W/ ARS & LNS SM, O, SIF, SL   CH4   6   69.1   58.9   99.6   100   UC     200   401   78   30   48   0.110   NS   31   NO SAMPLE   SO GR CH4 W/ ARS ML, SIF   CH4   67.6   CH4   67.6   CH3   3   68.7   57.5   97.0   96   UU   0   340   0   0.150   0.150	_	-																SV
NS 31 NO SAMPLE		1					50 O	00.6	100	шс		200	401	70	20	10	0.110	
9A 32 SO GR CH4 W/ ARS ML, SIF CH4 67.6 9B 33 SO GR CH3 W/ ARS ML, SIF CH3 3 68.7 57.5 97.0 96 UU 0 340 0.150	1				"		30.9	99.0	100			200	401	'0	30	40	0.110	
9B   33   SO GR CH3 W/ ARS ML, SIF   CH3   3   68.7   57.5   97.0   96   UU   0   340     0.150	_																	
	-				3		57.5	97.0	96	UU	0	340					0.150	
9C   34  SO GR CH4 W/ ARS ML, SIF, SL	9C	34	SO GR CH4 W/ ARS ML, SIF, SL	CH4	3	72.0	57.0	98.0	99	ŪŪ	0	323		91	22	69	0.200	

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-01

Sample Number	Depth	Visual Classification	USCS	E(f)		Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	ΡI	TORVANE (tsf)	Other Tests
9D	35	SO GR CH4 W/ ARS ML, SIF, SL	CH4		63.8												
10A	36	M GR CH4 W/ ARS ML, SIF	CH4		44.5												
10B	37	M GR CH4 W/ ARS ML, SIF	CH4		52.7												
10C	38	M GR CH4 W/ ARS ML, SIF	CH4	3	68.9	59.1	99.8	100	UC		626	1253	95	34	61	0.250	
NS	39	NO SAMPLE	NS														
11A	40	ST GNG CH2 W/ ARS ML	CH2		23.9												
11B	41	ST GNG CH2 W/ ARS ML	CH2	15	25.6	98.0	123	96	UU	0	1164		58	20	38	0.500	
11C	42	ST GNG CH2 W/ ARS ML	CH2		26.1												
NS	43	NO SAMPLE	NS														
12A	44	VST GNG CH2 W/ ARS ML	CH2		23.1												
12B	45	VST GNG CH2 W/ ARS ML	CH2		22.1												
12C	46	VST GNG CH2 W/ ARS ML	CH2	15	21.4	106	128	97	UC		2991	5982	53	16	37	0.875	
NS	47	NO SAMPLE	NS														
13A	48	M GNG & T CH3 W/ ARS ML, SL	CH3		32.3												
13B	49	M GNG & T CH3 W/ ARS ML, SL	CH3		31.3												
13C		M GNG & T CH3 W/ ARS ML, SL	CH3	9	29.6	90.8	118	93	UU	0	992		65	20	45	1.000	
NS	51	NS	NS														
14A	52	M T & GR CL6	CL6		33.3												
14B		M T & GR CL6 W/ CC	CL6		34.3												
14C	54	M T & GR CL6	CL6	3	30.0	93.0	121	100	UC		699	1398	47	19	28	0.625	
14D	55	M T & GR CL6	CL6		29.3												
15A	56	M T & GR CL6	CL6		29.1												
15B	57	ST GNG & T CH3 W/ ARS ML, CC, SL	CH3		32.5												
15C	58	ST GNG & T CH3 W/ ARS ML, CC, SL	CH3		35.0												
15D	59	ST GNG & T CH3 W/ ARS ML, CC, SL	CH3		38.1												
16A	60	ST BR & GR CH3 W/ ARS & LNS ML, CC, SL	CH3		45.1												
16B	61	ST BR & GR CH3 W/ ARS & LNS ML, SL	CH3	5	37.3	83.7	115	99	UU	0	1240		72	26	46	1.250	
16C	62	ST BR & GR CH3 W/ ARS & LNS ML, CC, SL	CH3		37.2												
NS	63	NS	NS														
17A	64	ST BR & GR CL4	CL4		26.7												
17B	65	ST BR & GR CL4	CL4		28.6												
17C	66	SO GR & BR CL6	CL6	3	34.2	75.6	109	97	UC		485	970	46	24	22	0.625	
NS	67	NS	NS														
18A	68	M GR CH2 W/ ARS & LNS ML	CH2		39.8												
18B	69	M GR CH2 W/ ARS & LNS ML	CH2	12	37.5	83.4	115	99	UU	0	736					0.250	
18C	70	M GR & T CH3 W/ ARS & LNS ML	CH3	4	49.6	72.3	108	100	UU	0	526		74	26	48	0.375	
18D	71	M GR & T CH3 W/ ARS & LNS ML	CH3		38.1												

Checked by: RR

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C. File Name: 24384

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-01

Sample Number	Depth	Visual Classification	USCS	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	ΡI	TORVANE (tsf)	Other Tests
19A	72	GR SC	SC		29.4	(1)	(1)										
19B	73	M GR CL6 W/ ARS SP	CL6		44.3												
19C	74	M GR CL6 W/ ARS SP	CL6		43.7												
19D	75	M GR CL6 W/ ARS SP	CL6		38.0												
20A	76	ST GR & T CH3 W/ ARS & LNS ML	CH3		38.5												
20B	77	ST GR & T CH3 W/ ARS & LNS ML	CH3	7	41.5	79.8	113	100	UC		1001	2002	57	23	34	0.375	
20C	78	ST GR & T CH3 W/ ARS & LNS ML	CH3		40.2												
20D	79	ST GR & T CH3 W/ ARS & LNS ML	CH3		42.0												
21A	80	ST GR CH3 W/ ARS ML, SL	CH3		33.8												
21B	81	ST GR CH3 W/ ARS ML, SL	CH3		36.0												
21C	82	ST GR CH3 W/ ARS ML, SIF, SL	CH3	4	36.3	85.1	116	99	UU	0	1150		59	18	41	0.500	
21D	83	ST GR CH3 W/ ARS ML, SL	CH3		37.2												
22A	84	ST GR CH4 W/ ARS ML, SL	CH4		34.1												
22B	85	ST GR CH4 W/ ARS ML, SL	CH4		36.3												
22C	86	ST GR CH4 W/ ARS ML, SL	CH4	4	34.3	87.8	118	100	UC		1145	2289	94	22	72	0.400	
NS	87	NS	NS														
23A	88	VST GNG CH3 W/ ARS ML, SL	CH3		38.6												
23B	89	VST GNG CH3 W/ ARS ML, SL	CH3		38.6												
23C	90	VST GNG CH3 W/ ARS ML, SL	CH3		37.6												
NS	91	NS	NS														
24A	92	ST GR CH4 W/ ARS ML, SL	CH4		47.7												
24B	93	ST GR CH4 W/ ARS ML, SL	CH4	2	45.0	76.1	110	99	UU	0	1392		89	29	60	0.400	
24C	94	ST GR CH4 W/ ARS ML, SL	CH4		47.4												
24D	95	ST GR CH4 W/ ARS ML, SL	CH4		48.1												
25A	96	VST GR CH4 W/ ARS & LNS ML	CH4		49.0												
25B	97	VST GR CH4 W/ ARS & LNS ML	CH4	3	48.2	73.5	109	100	UC		2086	4172	89	28	61	0.500	
25C	98	VST GR CH4 W/ ARS & LNS ML	CH4		49.4												
NS	99	NS	NS														
26A	100	ST GR CH4 W/ SL	CH4		46.7												
26B	101	ST GR CH4 W/ SL	CH4		47.7												
26C	102	ST GR CH4 W/ ARS ML	CH4	2	45.7	75.7	110	100	UU	0	1405		91	28	63	0.400	
26D	103	ST GR CH4 W/ SL	CH4		45.6												
27A	104	ST GR CH4 W/ SIF, SL	CH4		46.2												
27B	105	ST GR CH4 W/ SIF, SL	CH4		47.4												
27C	106	ST GR CH4 W/ SIF, SL	CH4		46.9												
NS	107	NS	NS														
28A	108	ST GR & BR CH4	CH4		59.1												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C. File Name: 24384

Checked by: RR

Project:	LOUISIANA,	STATE	OF -	COASTAL	PROTECTION	AND	RESTORATION	Assigned By:
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Project Number: 24384 Current Date: 12/31/2020

Boring: SB-01

28B 28C 28D NS PB-29 PB-30 PB-31 PB-32	110 111 112 113.5 116 118.5 121	Visual Classification ST GR CL6 W/ WD, RT ST GR CL6 W/ WD, RT ST GR CL6 W/ WD, RT NO SAMPLE GR SC GR SP GR SP W/ ARS CH GR SP GR SP	USCS CL6 CL6 CL6 NS SC SP SP SP SP	E(f)	w% 24.5 23.2 24.0 39.1 10.0 24.5 17.6 19.8	Dry Dens (pcf) 100	Wet Dens (pcf) 124	<b>Sat</b> % 95	Shear Test Type UU	Angle O	Cohesion (psf) 1399	Unconf. Comp. Str.	LL 41	PL 16	PI 25	TORVANE (tsf) 0.500	Other Tests

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Checked by: RR

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-02

Sample Number	Depth	Visual Classification	uscs	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	ΡI	TORVANE (tsf)	Other Tests
1A	0	ST BR & GR CH3 W/ ARS ML, RT	CH3		33.0												
NS	1	NS	NS														1
2A	2	M GR & T CH3 W/ ARS ML, CC	CH3		37.2												1
2B	3	M GR & T CH3 W/ ARS ML, CC, SL	CH3	15	35.1	85.4	115	97	UC		882		67	25	42	0.750	1
2C	4	M GR & T CH2 W/ ARS ML	CH2		30.5												1
NS	5	NS	NS		44.0												1
3A	6	M GR & T CH3 W/ ARS ML, CC	CH3	1 44	41.3	04.6	444	00			747		70	24	F0	0.005	4200
3B	7	M GR & T CH3 W/ ARS ML, CC	CH3	14	39.5	81.6	114	99	UU	0	747		73	21	52	0.625	-#200 = 98.5% ORG% = 5.05%
3C	8	M BR & GR CH2 W/ ARS ML, CC	CH2		32.0												1
NS	9	NS	NS														1
4A	10	M GR CH4 W/ ARS ML, SL	CH4		46.1												1
4B	11	M GR CH4 W/ ARS ML, SL	CH4	2	55.4	66.8	104	98	UC		549	1098	104	30	74	0.625	-#200 = 98.9%
4C	12	M GR CH4 W/ ARS ML, SL	CH4		45.4												
PB-4B	13	BR WD	WD		400.3												1
5A	14	M GNG CH4 W/ ARS ML, WD, SL	CH4		52.3												1
5B	15	M GR & T CH4 W/ ARS ML, SL, WD, RT	CH4	5	55.8	65.8	102	96	UU	0	665		94	25	69	0.375	-#200 = 98.6% ORG% = 7.20%
5C	16	M GNG CH4 W/ ARS ML, WD, SL	CH4		52.6												1
5D	17	M GNG CH4 W/ ARS ML, WD, SL	CH4		56.2												1
6A	18	SO GNG CH4 W/ ARS ML, WD, SL	CH4		74.3								100	26	74		-#200 = 93.3% ORG% = 10.69%
6B	19	SO GNG CH4 W/ ARS ML, WD, SL	CH4		75.0												
6C	20	SO GR & BR CHOB W/ WD	СНОВ	5	109	41.2	86.2	95	UU	0	445		171	53	118	0.625	ORG% = 13.27%
6D	21	SO BR & GR CHOC	снос		167.4												13.2. /3
7A	22	SO BR & GR CHOB	СНОВ		131.0												
7B	23	M BR & GR CHOA	CHOA	5	95.6	45.9	89.7	96	UU	0	860		152	53	99	0.625	ORG% = 12.83%
7C	24	SO BR & GR CH4 W/ ARS ML, O	CH4	5	79.2	52.8	94.7	97	UC		456	912	103	41	62	0.500	12.03%
7D	25	M GR & BR CH4 W/ ARS ML, RT	CH4		59.4												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Checked by: RR

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-02

Sample Number	Depth	Visual Classification	uscs	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
8A	26	SO GR CL4 W/ ARS SP, SIF	CL4		48.9								25		4.4		
8B 8C	27 28	SO GR CL4 W/ ARS SP, SIF GR ML W/ ARS CH, SP, SIF	CL4 ML		35.2 35.9								35 NP	21 NP	14		MINI
00	20	GIVINE W/ AIXS OI I, SF, SII	IVIL		33.8								INF				VANE =
																	517 TSF
NS	29	NS	NS														
9A	30	M GR CH4 W/ ARS & LNS ML, SIF	CH4		68.3												
9B	31	SO GR CH3 W/ ARS ML, SIF	CH3	2	73.6	56.0	97.2	98	UU	0	360		76	25	51	0.200	
9C	32	M GR CH4 W/ ARS & LNS ML, SIF	CH4		70.0												
9D	33	M GR CH4 W/ ARS & LNS ML, SIF	CH4		68.4												
10A		M GR & DGR CH4 W/ ARS ML, SIF, O	CH4		73.0												
10B		M GR & BR CHOA W/ WD	CHOA	4	87.9	47.9	89.9	94	UC		872	1744	120	43	77	0.750	
10C		M GR & DGR CH4 W/ ARS ML, SIF, O	CH4		63.4												
10D		M GR & DGR CH4 W/ ARS ML, SIF, O	CH4		74.3												
11A	38	ST GR CH3 W/ ARS ML, CC	CH3		27.3												
11B	39	ST GR & T CH3 W/ ARS ML	CH3	14	23.8	100	124	93	UC		1735	3569				1.000	
11C	40	ST GR & T CH3 W/ ARS ML	CH3	14	23.0	99.3	122	88	UU	0	1594		59	15	44	0.875	
NS	41	NS	NS														
PB-12	42	M GR & T CH3 W/ ARS ML	CH3		37.8												
13A		M GR & T CH3 W/ ARS ML, CC	CH3		42.3									ا ا			
13B	47	M T & GR CL4 W/ CC	CL4		30.8								37	18	19		-#200 = 97.1%
13C	48	M T & GR CL4 W/ CC	CL4	4	34.8	83.6	113	93	UC		765	1530	34	20	14	0.150	97.1%
13D	49	ST T & GR CL4 W/ CC	CL4		32.6												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN Checked by: RR

EUSTIS ENGINEERING L.L.C. File Name: 24384

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-03

Sample Number	Depth	Visual Classification	USCS	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A 1B 1C	1	M GR & BR CH3 W/ ARS SP, RT, SIF, SL M GR & BR CH3 W/ ARS SP, RT, SIF, SL M GR & BR CH3 W/ ARS SP, RT, SIF, SL	CH3 CH3 CH3	3	36.1 37.6 32.9	85.2	113	90	UC		437	874	67	25	42	0.375	-#200 =
1D	3	M CD 9 DD CH2 W/ ADC MI DT CC	CH3		42.2												87.6%; ORG = 4.5%
2A		M GR & BR CH3 W/ ARS ML, RT, CC M T & GR CH2 W/ CC, RT	CH3		26.6												
2B		M T & GR CH2 W/ CC, RT	CH2	12	27.4	96.4	123	98	UC		889	1778	51	20	31	0.500	ORG = 2.8%
NS	6	NS	NS														070
3A		M GR & T CH3 W/ ARS ML, CC	CH3 CH3	_	37.3	70.4	440	100	UC		007	1734	70	0.5	47	0.500	
3B	9	M GR & T CH3 W/ ARS ML, CC	CH3	7	41.9	79.4	113	100			867	1/34	72	25	47	0.500	ORG = 8.2%
3C		M GR & T CH3 W/ ARS ML, CC	СНЗ		36.1												
NS		NS	NS														
4A 4B		M GR CH4 W/ ARS ML, WD M GR CH4 W/ ARS ML, WD	CH4 CH4		48.5 48.7												
4B 4C	ı	M GR CH4 W/ ARS ML, WD	CH4	9	45.9	73.9	108	96	lυυ	0	683		88	27	61	0.625	-#200 =
		,															90.7%; ORG = 4.7%
4D	15	M GR CH4 W/ ARS ML, WD, SL	CH4		49.4												4.770
5A		SO GR CH4 W/ ARS ML, WD, SL	CH4		71.4												
5B	17	SO GR CH4 W/ ARS ML, WD, SL	CH4		71.4												
5C	18	SO GR CHOA	CHOA	3	89.8	48.2	91.4	97	UC		473	946	142	41	101	0.375	ORG = 4.4%
NS	19	NS	NS														
6A		M GR & DGR CHOC	CHOC		140.3												
6B	21	M GR & DGR CHOC	CHOC	6	136.4	35.8	84.6	99	UU	0	974		224	59	165	0.375	ORG = 19.0%
6C	22	M GR & DGR CHOC	СНОС		140.6												
NS	23	NS	NS														
7A		M GR CH3 W/ ARS & LNS SP, WD	CH3		46.4												
7B		M GR CH3 W/ ARS & LNS SP, WD	CH3	۱.,	49.7	05.0	440	100			054	F 0.1				0.000	
7C 7D	26 27	SO GR CL4 W/ ARS SP, SIF SO GR CL6 W/ ARS SP, SIF	CL4 CL6	11	36.3 42.1	85.2	116	100	UC		251	501	35	21	14	0.200	
10		GN GLU W/ AND DP, DIF	CL6		42.1												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-03

Sample Number	Depth	Visual Classification	USCS	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
8A	28	SO GR CH3 W/ ARS SP, SIF	CH3		67.0												
8B	29	SO GR CH3 W/ ARS SP, SIF	CH3		65.1												
8C	30	SO GR CH3 W/ ARS SP, SIF	CH3		72.1												
8D	31	SO GR CH2 W/ ARS SP, SIF	CH2	6	40.6	80.7	113	100	UU	0	317		50	18	32	0.150	
9A	32	ST GNG CL6 W/ SIF	CH3		27.1												
9B	33	ST GNG CL6	CL6		28.6												
9C	34	SO GNG CL6	CL6	3	20.6	105	127	92	UC		400	800	41	13	28	0.150	
9D	35	ST GNG CL6	CL6		21.6												
10A	36	ST GNG CL6	CL6		22.5												
10B	37	ST GNG CL6	CL6		25.9												
NS	38	NS	NS														
11A	_	M T & GR CL4 W/ CC	CL4		28.3												
11B	41	M T & GR CL4 W/ CC	CL4		33.1												
11C	42	M T & GR CL4 W/ CC	CL4	14	30.6	91.9	120	99	UU	0	567		31	18	13	0.150	
NS	43	NS	NS														
12A	44	BR ML	ML		32.2												
12B	45	BR ML	ML		30.7												-#200 =
NS	46	NS	NS		l <u></u>												90.5%
13A	48	BR ML W/ CC	ML		31.9												
13B	49	BR ML W/ CC	ML		29.8												
13C	50	BR ML W/ CC	ML		29.4												-#200 =
130	30	BIC WE W/ GO	IVIL		23.4												67.0%
13D	51	BR ML W/ CC	ML		25.2												
14A	52	BR ML W/ CC	ML		31.1												
14B	53	BR ML W/ CC	ML		26.4												
14C	54	BR ML W/ CC	ML		27.1												
NS	55	NS	NS														
15A		M T & GR CL6	CL6		32.6												
15B	57	M T & GR CL6	CL6	5	33.9	84.5	113	92	UC		710	1420	45	25	20	0.500	
15C	58	M T & GR CL6	CL6	-	31.9										-		
15D		M T & GR CL6	CL6		33.2												
16A		M T & GR CL6	CL6		31.0												
16B	61	M GR & T CH3 W/ ARS & LNS SM	CH3	15	43.7	76.3	110	97	UU	0	845		69	22	47	0.625	
16C	_	M GR & T CH3 W/ LNS & LYS SM	CH3	-	37.4										•		
NS	63	NS	NS														
17A	64	M GR & T CH3 W/ ARS & LNS ML, CC	CH3		44.0												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-03

Sample Number	Depth	Visual Classification	uscs	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests	
17B		M GR & T CH3 W/ ARS & LNS ML, CC	CH3		45.6													ı
17C	66	M GR & T CH3 W/ ARS & LNS ML, CC	CH3	3	35.0	85.5	115	97	UC		894	1788	51	15	36	0.500		ı
17D	67	M GR & T CH3 W/ ARS & LNS ML, CC	CH3		38.4													ı
18A	68	ST GR CH3 W/ ARS SM, SIF	CH3		41.4													ı
18B	69	ST GR CH3 W/ ARS SM, SIF	CH3		46.0													ı
18C	70	ST GR CL6 W/ ARS SM	CL6	8	29.4	93.2	121	97	UU	0	1030		40	15	25	0.625		ı
18D	71	ST GR CL6 W/ ARS SM, SIF	CL6		39.7													ı
19A	ı	M GR CL6 W/ ARS SM	CL6		34.4													ı
19B	73	M GR CL6 W/ ARS SM	CL6		33.1													ı
19C	ı	M GR CL6 W/ ARS SM	CL6	3	38.1	81.2	112	95	UC		814	1629	44	14	30	0.700		ı
19D	ı	M GR CL6 W/ ARS SM	CL6		35.1													ı
20A	76	ST GR CH3 W/ ARS ML	CH3		43.2													ı
20B	77	ST GR CH3 W/ ARS ML	CH3		39.3													ı
20C	78	ST GR CH3 W/ ARS ML	CH3		39.4													ı
20D	79	ST GR CH3 W/ ARS ML	CH3		39.7													ı
21A	80	ST GR CH3 W/ ARS ML, SL	CH3		37.1													ı
21B	81	ST GR CH3 W/ ARS ML, SL	CH3	2	43.7	76.6	110	98	UU	0	728		72	23	49	0.700		ı
21C	82	ST GR CH3 W/ ARS ML, SL	CH3		40.4													ı
NS	83	NS	NS															ı
22A	84	ST GR CH4 W/ ARS ML, SL	CH4		43.3													ı
22B	85	ST GR CH4 W/ ARS ML, SL	CH4		42.4													ı
22C	86	ST GR CH4 W/ ARS ML, WD	CH4	2	42.7	77.5	111	98	UC		1468	2936	74	18	56	0.625		ı
22D	87	ST GR CH4 W/ ARS ML, SL	CH4		43.1													ı
23A	88	ST GR CH4 W/ ARS ML, SL	CH4		43.0													ı
23B	89	ST GR CH4 W/ ARS ML, SL	CH4		43.7													ı
23C	90	ST GR CH4 W/ ARS ML, SL	CH4	1	44.9	75.1	109	97	UU	0	1065		84	22	62	0.625		ı
23D	91	ST GR CH4 W/ ARS ML, SL	CH4		43.2													ı
24A	92	ST GR CH4 W/ ARS ML, O, SL	CH4		45.8													ı
24B	93	ST GR CH4 W/ ARS ML, O, SL	CH4		45.8													ı
24C	94	ST GR CH4 W/ ARS ML, SL	CH4	3	47.3	73.1	108	97	UC		1929	3859	87	25	62	0.625		ı
24D	95	ST GR CH4 W/ ARS ML, O, SL	CH4		45.6													ı
25A	96	ST GR & T CH4 W/ ARS ML, SIF	CH4		46.1													ı
25B	97	ST GR & T CH4 W/ ARS ML, SIF	CH4		44.8													ı
25C	98	ST GR CH4 W/ ARS & LNS ML, SL	CH4	2	46.7	73.4	108	97	UU	0	1808		91	24	67	0.625		ı
25D	99	ST GR & T CH4 W/ ARS ML, SIF	CH4		45.5													ı
26A	100	ST GR CH4 W/ ARS & LNS ML	CH4		45.8													ı
26B	101	ST GR CH4 W/ ARS & LNS ML	CH4		50.0													ı

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-03

Sample Number	Depth	Visual Classification	uscs	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp.	LL	PL	ΡI	TORVANE (tsf)	Other Tests
26C	102	VST GR CH4 W/ ARS & LNS ML	CH4	4	44.3	75.3	109	96	ÜC		2069	4138	87	22	65	0.750	
26D	103	ST GR CH3 W/ ARS & LNS ML	CH3		49.7												
27A	104	M GNG CL4 W/ ARS SP	CL4		22.6												
27B	105	M GNG CL4 W/ ARS SP	CL4		22.8												
27C	106	M GNG CL4 W/ ARS SP	CL4	6	21.6	106	128	98	UC		646	1292	28	16	12	0.500	
NS	107	NS	NS														
28A		M GNG CL4 W/ ARS SP	CL4		24.5												
28B	109	M GNG CL4 W/ ARS SP	CL4		24.4												
28C	110	M GNG CL4 W/ ARS SP	CL4		25.9												
28D	111	M GNG CL4 W/ ARS SP	CL4		25.4												
29A		M GNG CL4 W/ CC	CL4		27.2												
29B		M GNG CL4 W/ ASR SP, O	CL4	13	27.4	96.8	123	100	UU	0	608		32	13	19	0.375	
29C		M GNG CL4 W/ WD, O	CL4		29.8												
29D		M GNG CL4 W/ WD, O	CL4		30.2												
30A		M GR & T CL4 W/ ARS SP, WD	CL4		31.4												
30B		M GR & T CL4 W/ ARS SP, WD	CL4		32.0												
30C	118	M GR & T CL4	CL4	4	30.8	89.7	117	95	UC		623	1245	30	14	16	0.625	-#200 =
																	95.7%
NS	119	NS	NS														
PB-31		M GR CH2 W/ ARS ML	CH2		39.1												
32A	122	ST GNG & T CL4 W/ CC, SIF	CL4		25.9												
32B	_	M GNG & T CL4	CL4	3	33.3	88.0	117	99	UU	0	590		28	15	13	0.375	
32C	124	ST GNG & T CL4 W/ CC, SIF	CL4		26.5												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

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Checked by: RR

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-04

Sample Number	Depth	Visual Classification	uscs	E(f)	<b>W</b> %	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A 1B 1C	1	ST BR & GR CH2 W/ ARS SP, SIF, RT ST GR CH2 W/ ARS SP, SIF, RT ST GR CH2 W/ ARS SP, SIF, RT	CH2 CH2 CH2		21.2 22.1 25.9								50	13	37		-#200 = 91.7% ORG% = 3.91%
1D 2A 2B	3 4 5	ST GR CH2 W/ ARS SP, SIF, RT VST GR CH3 W/ ARS SP, SIF, RT VST GR & T CH3 W/ ARS ML, WD	CH2 CH3 CH3	10	27.1 32.0 33.8	85.0	114	92	UC		2024	4047	76	31	45	0.750	-#200 = 89.7% ORG% = 7.31%
NS 3A 3B	8	NS M GR & T CH2 W/ ARS ML, RT, SIF M GR & T CH2 W/ ARS ML, RT, SIF	NS CH2 CH2		 40.5 36.6								50	19	31		-#200 = 95.2% ORG% = 3.52%
NS 4A 4B	12	NS M GR & BR CL4 W/ CC M GR & T CL4 W/ CC	NS CL4 CL4	14	33.6 30.3	90.3	118	95	UU	0	796		34	16	18	0.200	-#200 = 90.7%
4C 4D 5A 5B 5C	15	M GR & BR CL4 W/ CC M GR & BR CL4 W/ CC SO GR CH4 W/ ARS ML, SL SO GR CH4 W/ ARS ML, SL SO GR CHOA W/ RT	CL4 CL4 CH4 CH4 CHOA	3	30.1 35.0 59.1 59.7 70.3	55.9	95.1	94	UC		440	881	128	35	93	0.200	-#200 = 97.5%
NS 6A 6B NS	20 21 22	NS M GR CH4 W/ ARS ML, RT, SL M GR CH4 W/ ARS ML, RT, SL NS	NS CH4 CH4 NS		 60.8 58.9												ORG% = 9.47%
7A 7B 7C	25	M GR CHOA W/ WD, RT M GR & BR CHOB M BR & GR CHOC	CHOA CHOB	4	90.4 101.0 131.1		87.9 81.7	96 94	UU UU	0	532 926		167 195	47 62	120 133	0.150 0.200	ORG% = 15.14% ORG% =

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

File Name: 24384

Checked by: RR

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-04

Sample Number	Depth	Visual Classification	uscs	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests 21.72%
7D	27	SO BR PT	PT CHOA		218.7												21.72/0
8A 8B	28 29	SO GR CHOA W/ WD SO GR CH4 W/ ARS ML, WD, O	CHOA CH4		96.5 75.4												
8C	_	M GR CH4 W/ ARS & LNS ML, RT	CH4	6	73.4	56.6	97.9	99	UU	0	555		99	29	70	0.200	
8D		M GR CH4 W/ ARS & LNS ML, O	CH4	~	41.3	00.0	07.0	00			000				,,	0.200	
9A	32	GR ML W/ ARS CH, SIF	ML		39.6												
9B	33	SO GR CH4 W/ ARS & LNS SP, RT	CH4	3	63.1	60.1	98.0	94	UC		384	768	83	23	60	0.200	
9C	34	SO GR CH4 W/ ARS & LNS SP, SIF	CH4		65.7			-					89	25			
9D	35	SO GR CH4 W/ ARS & LNS SP, SIF	CH4		62.9												
10A	36	M GR CL6 W/ SIF	CL6		33.2												
10B	37	M GR CL6 W/ SIF	CL6		29.1								45	15	30		-#200 =
10C	38	SO GR & T CH2 W/ ARS ML, SL	CH2	2	20.7	107	129	95	UC			965	52	20	32	0.750	99.7%
10D	39	VST GR & T CH3 W/ ARS ML, SL	CH3	-	21.3		0						"-		-	000	
11A	40	VST GR & BR CH3 W/ ARS ML, CC	CH3		24.0												
11B	41	VST GR & BR CH3 W/ ARS ML, CC	CH3		24.2												
11C	42	ST LGR & T CH3 W/ ARS ML, CC, SL	CH3	11	28.8	92.5	119	94	UC		1479	2958	65	21	44	1.000	
NS	43	NS	NS														
12A	44	SO GR & T CL6	CL6		33.1												
12B	45	SO GR & T CL6	CL6		31.3												
12C	46	SO GR & T CL6	CL6	4	32.9	89.3	119	100	UC		268	537	41	17	24	0.625	
12D	47	SO GR & T CL6	CL6		32.9												
PB-13	48	VST BR & GR CL6	CL6		25.0												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

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Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-05

Sample Number	Depth	Visual Classification	USCS	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A PB-1A 2A	0 1 4	ST GR & BR CH2 W/ ARS ML, RT, O ST GR & BR CH2 W/ ARS ML, RT, O ST GR & BR CH2 W/ ARS ML, CC	CH2 CH2 CH2		27.6 24.0 30.8								51	16	35		ORG = 3.1%
2B 2C NS	5 6 7	SO T & GR CL4 W/ CC SO T & GR CL4 W/ CC NS	CL4 CL4 NS	15	32.1 30.9 	90.2	119	100	UU	0	495		41	20	21	0.625	3.176
3A 3B 3C	8 9 10	M BR & GR CL6 W/ WD M BR & GR CL6 W/ WD M T & GR CH4 W/ ARS ML, CC	CL6 CL6 CH4	7	31.3 30.9 42.4	72.6	107	97	UC		677	1353	80	33	47	0.750	ORG = 3.7%
3D 4A 4B 4C	ı	M GR & T CH4 W/ ARS SP, RTS M GR CH4 W/ ARS ML, WD M GR CH4 W/ ARS ML, WD SO GR & T CHOA	CH4 CH4 CH4 CHOA	11	61.1 77.2 71.8 68.0	54.3	95.7	98	UU	0	340		117	35	82	0.500	ORG =
NS 5A	15 16	NS M GR CHOA W/ WD, CC	NS CHOA		 84.1	0 10	00.1				0.10					0.000	7.1%
5B 5C 5D	17 18 19	M GR CHOA W/ WD, CC M GR & DGR CHOA M GR & DGR CHOA	CHOA CHOA	8	111.0 151.9 114.3	41.3	88.4	100	UC		559	1119	132	32	100	0.625	ORG = 21.5% ORG =
6A 6B 6C	20 21 22	SO GR CHOA SO GR CHOA SO GR CHOA	CHOA CHOA CHOA		91.7 90.2 75.3												14.3%
NS 7A 7B 7C 7D	23 24 25 26 27	NS SO GR CH2 W/ ARS & LNS ML SO GR CH3 W/ ARS & LNS ML SO GR CH3 W/ ARS & LNS ML, SIF	NS CH2 CH3 CH3 CH3	3	 43.9 53.2 59.8 63.3	68.8	105	98	UU	0	334		58	22	36	0.375	
8A 8B 8C NS	28 29 30 31	SO GR CH3 W/ ARS & LNS ML, SIF SO GR CL6 W/ CC ST GR & BR CH3 W/ ARS ML, CC, SL M GR & BR CH3 W/ ARS ML, SL NS	CL6 CH3 CH3 NS	2	35.4 30.9 34.9	82.8	112	90	UC		620	1240	79	26	53	0.500	
9A 9B	32 33	ST GR & BR CH3 W/ ARS ML, CC, SL ST GR & BR CH3 W/ ARS ML, CC, SL	CH3 CH3		32.1 33.3												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project:	LOUISIANA,	STATE	OF -	COASTAL	PROTECTION	AND	RESTORATION	Assigned 1	By:
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Project Number: 24384 Current Date: 12/31/2020

Boring: SB-05

Sample Number NS	Depth 34	Visual Classification	uscs NS	E(f)	<b>W</b> %	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
10A 10B	36 37	ST GR & T CH3 W/ ARS ML, SL SO GR & T CL4 W/ CC	CH3 CL4		33.8 34.8								30	20	10		-#200 = 95.4%
10C 10D 11A 11B 11C 11D	43 44	SO GR & T CL4 W/ CC SO GR & T CL4 W/ CC ST GR & T CH4 W/ ARS & LNS ML ST GR & T CH4 W/ ARS & LNS ML ST GR & T CH4 W/ ARS & LNS ML ST BR & GR CH3 W/ ARS & LNS ML ST T CL4 W/ ARS SP	CL4 CL4 CH4 CH4 CH4 CH3 CL4	5	33.9 34.6 36.1 36.7 45.5 35.7 26.3	81.8	112	93	UU	0	1247		82	25	57	0.625	
12B PB-13 PB-14	45 46 48.5	ST T CL4 W/ ARS SP T ML T ML	CL4 ML ML		26.1 29.3 28.7								30	17	13		-#200 = 86.7%

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-06

Sample Number	Depth	Visual Classification	uscs	E(f)	<b>W</b> %	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A	0	ST GR & BR CH2 W/ CC, RT	CH2		31.3												
1B	1	M T & GR CH3 W/ ARS ML, O RT	CH3		35.5								66	27	39		ORG =
1C	2	ST GR & BR CH2 W/ CC, RT	CH2		33.1												5.4%
l NS	3	NS	NS		33.1												
2A	4	ST GR & BR CH2 W/ CC	CH2		31.2												
2B	5	SO GR & BR CH2 W/ CC, WD	CH2	6	30.5	89.8	117	93	UC		424	848	50	20	30	0.500	
NS NS	6	INS	NS	ľ		00.0	1 1 7	33			727	010	50	20	50	0.500	
3A	8	ISO GR & T CL4	CL4	1	32.3												
3B	9	SO GR & T CH3 W/ ARS ML, SL	CH3		36.3												
3C	10	SO GR CH4 W/ ARS ML, SL	CH4	4	48.0	73.0	108	98	UU	0	387		97	33	64	0.625	
NS	11	INS	NS	'		7 0.0	100				00.		"		0.	0.020	
4A	12	SO GR CH4 W/ ARS ML, WD, RT, SL	CH4	1	81.8												
4B	13	SO GR CH4 W/ ARS ML, WD, RT, SL	CH4		78.8												
4C	14	VSO DGR CH0A	CH0A	8	83.0	47.5	92.4	100	UC		245	490	132	32	100	0.150	ORG =
l <sub>NS</sub>	15	NS	NS														9.2%
5A	16	SO GR CH4 W/ ARS ML, O, RT	CH4	1	54.7												
5B	17	SO GR CH4 W/ ARS ML, O, RT	CH4	1	55.2								72	21	51		
5C	18	SO GR CH4 W/ ARS ML, O, RT	CH4		52.8												
NS	19	NS	NS	1													
6A	20	SO GR CH4 W/ ARS ML, RT	CH4		66.3												
6B	21	SO GR CH4 W/ ARS ML, RT	CH4	5	71.0	57.8	98.9	100	UU	0	439		98	27	71	0.150	
6C	22	SO GR CH4 W/ ARS ML, RT	CH4	1	67.4								94	21	73		
6D	23	M GR CH4 W/ ARS ML, RT	CH4		64.6												
7A	24	SO GR CH4 W/ ARS ML, SIF	CH4	1	76.8												
7B	25	SO GR CH4 W/ ARS ML, SIF	CH4		75.7												
7C	26	SO LGR CL6	CL6	5	25.2	97.1	122	93	UC		484	968	41	17	24	0.625	
7D	27	ST LGR CL6 W/ SIF	CL6		25.6												
8A	28	ST GR & T CH3 W/ ARS ML, CC, SL	CH3	1	23.0												
8B	29	ST GR & T CH3 W/ ARS ML, CC, SL	CH3		23.9								63	20	43		
NS	30	NS	NS														
9A	32	ST GR & T CH3 W/ ARS ML, SL CC	CH3		40.4												
9B	33	ST GR & T CH3 W/ ARS ML, SL CC	CH3		40.2												
9C		ST T & LGR CH4 W/ ARS ML, SL, CC	CH3	11	27.4	95.6	122	96	UU	0	1648		66	25	41	1.000	
NS	35	NS	NS														
10A	36	SO GR & T CL4	CL6		32.3												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project:	LOUISIANA,	STATE	OF -	COASTAL	PROTECTION	AND	RESTORATION	Assigned By:
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Project Number: 24384 Current Date: 12/31/2020

Boring: SB-06

Sample Number 10B 10C NS	39	Visual Classification SO GR & T CL6 SO GR & T CL6 NS	USCS CL6 CL6 NS	E(f)	31.3 32.7 	Dry Dens (pcf) 88.4	Wet Dens (pcf)	<b>Sat</b> %	Shear Test Type UC	Angle	Cohesion (psf)	Unconf. Comp. Str. 867	LL 44	PL 16	PI 28	TORVANE (tsf)	Other Tests
11A 11B 11C NS PB-12	41 42 43 44	M T & GR CH3 W/ ARS & LNS ML, CC, SL T ML W/ LNS CH, CC T ML W/ CC NS T ML W/ ARS CH, CC	CH3 ML ML NS ML		38.3 33.9 33.8  32.4								NP	NP			-#200 = 73.2%
PB-13 PB-14	46.5 48.5	M T & GR CL6 T ML W/ ARS CH	CL6 ML		37.1 31.2												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Checked by: RR

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-07

Sample Number	Depth	Visual Classification	USCS	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A	0	ST BR & GR CH4 W/ ARS ML, RT	CH4		41.6								84	36	48		
NS	1	NS	NS														
PB-1A	2	ST BR & GR CH3 W/ ARS ML, CC	CH3		44.3												
2A	4	SO GR & T CL4 W/ RT SO GR CL4 W/ O	CL4 CL4	,,	34.9	00.0	440	00	UU		246		27		47	0.450	ORG =
2B	5	SO GR CL4 W/ O	CL4	14	32.6	88.6	118	98	00	0	346		37	20	17	0.150	3.4%
2C	6	SO GR CL6 W/ ARS SP	CL4		34.1												0.470
NS	7	NS	NS														
3A	8	SO GR CH4 W/ ARS ML, CC, RT	CH4		51.3												
3B	9	SO GR CH4 W/ ARS ML, RT	CH4	8	58.2	65.3	103	99	UU	0	264		101	31	70	0.500	ORG% =
																	6.70%
3C	10	SO GR CH4 W/ ARS ML	CH4	2	43.9	76.2	110	97	UC		486	972	82	23	59	0.625	
NS	11	NS	NS														
4A	12	SO GR CHOB	СНОВ		157.1												
4B	13	SO GR CHOB	CHOB		140.3								450				
4C	14	SO GR CHOA	CHOA		137.7								150	52	98		ORG% =
4D	15	SO GR CHOB	СНОВ		130.1												17.7270
5A	16	SO GR & BR CHOC	CHOC	1	161.6												
5B	17	M DGR CHOB	СНОВ	1	117.4	39.5	85.9	98	UU	0	614		180	63	117	0.375	ORG% =
																	20.83%
PB-5B	18	VSO GR & T CH3 W/ ARS ML, RT	CH3		87.5												
6A	20	SO BR CH3 W/ ARS SP	CH3		59.3												
6B	21	SO GR CL4 W/ WD	CL4	7	32.7	89.5	119	100	UC		479	959	28	17	11	0.450	
6C	22	SO GR CH3 W/ ARS & LNS SP	CH3		42.1												
NS	23	NS	NS														
7A	24	SO GR CH3 W/ ARS ML, SIF	CH3	4.5	60.6	07.4	447	400	0.0		070					0.450	
7B	25	SO GR CL6 W/ SIF	CL6	15 14	34.4	87.4 85.9	117 117	100 100	OB UU	0	272 340		ا ۱	,	23	0.150	
7C 7D	26 27	SO GR CL6 W/ SIF SO GR CH3 W/ ARS ML. O	CL6 CH3	14	35.7 47.2	85.9	117	100	UU	0	340		41	18	23	0.150	
8A	28	ISO GR CH3 W/ ARS ML	CH3		34.8												
8B	29	ST GR & T CH3 W/ ARS ML, RT	CH3		29.3												
8C1	30	IST LGR & T CH3 W/ ARS ML	CH3	13	24.7	102	127	100	UC		1283	2566	56	20	36	0.750	
8C2	30.3	IST LGR & T CH3 W/ ARS ML	CH3	15	23.9	101	125	95	OB	0	1263		56	20	36	0.775	
NS	31	NS	NS	'							00			-			
9A	32	ST GR & T CH3 W/ ARS ML, CC, SL	CH3		33.5												
9B	33	ST GR & T CH3 W/ ARS ML, CC, SL	СНЗ		41.6												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Checked by: RR

Project:	LOUISIANA,	STATE	OF -	COASTAL	PROTECTION	AND	RESTORATION	Assigned 1	By:
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Project Number: 24384 Current Date: 12/31/2020

Boring: SB-07

Sample Number	Depth	Visual Classification	uscs	E(f)	<b>W</b> %	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
9C 9D 10A 10B 10C 10D 11A 11B	35 36 37 38 39 40	ST GR & T CH3 W/ ARS ML, CC, SL M GR & T CL6 SO GR & T CH3 W/ ARS & LNS ML, CC M GR & T CL6 M T CL4 ST T & GR CH3 W/ ARS & LNS ML ST T & GR CH3 W/ ARS & LNS ML, CC, SL BR SM	CH3 CL6 CH3 CL6 CL4 CH3 CH3	6 13	37.7 26.7 47.3 29.3 30.9 35.8 36.1 26.2	94.2	122 120	100 99	UU OB	0	784 548	1095	41 38 56 NP	13 19 19 NP	19	0.375 0.350	PD PD -#200 =
NS PB-12 PB-13 PB-14	45.5	NS BR ML BR ML BR ML	NS ML ML ML		 27.3 29.5 28.1												40.1%

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Checked by: RR

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-08

Sample Number	Depth	Visual Classification	USCS	E(f)	<b>W</b> %	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A	0	VST GR & T CH4 W/ ARS ML, CC, SL, WD	CH4		46.6								98	30	68		ORG% =
NS PB-1B 2A 2B	1 2 4 5	NS ST GR & T CH3 W/ ARS ML, CC, SL ST GR & T CH3 W/ ARS ML, RT, SL M GR & T CH3 W/ ARS ML, RT, SL	NS CH3 CH3 CH3	4	 43.4 47.7 28.8	90.9	117	90	UC		840	1679	61	19	42	0.750	6.67% ORG% =
PB-2B 3A 3B 3C	6 8 9 10	SO GR & T CL4 W/ CC M GR & T CH4 W/ ARS ML, CC, SL M GR CH4 W/ ARS ML, SL M GR & BR CH0A	CL4 CH4 CH4 CHOA	9	32.5 47.9 52.3 72.5	54.9	94.7	94	UU	0	573		144	45	99	0.750	3.95% CONS ORG% =
3D 4A 4B		M GR & BR CHOA M DGR CHOA W/ WD M DGR CHOA W/ WD	CHOA CHOA CHOA		97.2 102.5 118.8												0RG% = 17.4%
4C 4D 5A 5B 5C NS	ı	SO GR CH4 W/ ARS ML, SL M GR CH3 W/ ARS SM, O M GR CH3 W/ ARS ML, WD M GR CH3 W/ ARS ML, WD M GR CH3 W/ ARS ML, WD NS	CH4 CH3 CH3 CH3 CH3 NS	4	58.2 42.9 50.4 46.3 39.3	65.0	103	98	UC		278	558	82 61	19 18	<ul><li>63</li><li>43</li></ul>	0.500	
6A 6B 6C 6D 7A	20 21 22 23 24	SO GR CL6 W/ ARS SP SO GR CH3 W/ ARS SP, SIF	CL6 CL6 CL6 CL6 CL6	15	41.2 39.9 34.3 38.9 49.0	81.0	113	100	UU	0	413		42	19	23	0.150	
7B 7C 7D 8A 8B	25 26 27 28 29	SO GR & T CL6 W/ CC ST LGR & T CH2 W/ ARS ML ST GNG & T CH3 W/ ARS & LNS ML, CC, SL ST GNG & T CH3 W/ ARS & LNS ML, SIF, SL ST GNG & T CH3 W/ ARS & LNS ML, SIF, SL	CL6 CH2 CH3 CH3 CH3	9 11	26.5 23.2 28.8 25.7 26.7	97.6 101	124 125	98 93	OB UC	0	407 1185	814 2370	48 57	16 20	32 37	0.750 0.875	
8C 8D 9A 9B	30 31 32 33	VST LTG & T CH3 W/ ARS ML, CC, SL ST GNG & T CH3 W/ ARS & LNS ML, SIF, SL ST GR & T CL6 W/ CC ST GR & T CL6 W/ CC	CH3 CH3 CL6 CL6	15	25.1 24.8 33.2 33.0	100	125	98	UU	0	2133		62	19	43	1.000	

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

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Project:	LOUISIANA,	STATE	OF	- C	OASTAL!	PROTECTION	AND	RESTORATION	Assigned By:	
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Project Number: 24384 Current Date: 12/31/2020

Boring: SB-08

Sample Number 9C 9D PB-10	35	Visual Classification  M T & GR CH4 W/ ARS & LNS ML, CC ST GR & T CH3 W/ ARS ML, CC, SL T ML W/ ARS CH	USCS CH4 CH3 ML	E(f)	<b>W</b> % 29.0 35.5 31.3	Dry Dens (pcf) 92.1	Wet Dens (pcf) 119	<b>Sat</b> % 94	Shear Test Type OB	Angle	Cohesion (psf) 862	Unconf. Comp. Str. 1724	1L 78 NP	рі 56	TORVANE (tsf) 0.750	Other Tests -#200 = 96.4%
PB-11 PB-12 PB-13 PB-14 PB-15	41 43.5 46	T ML W/ ARS CH T ML W/ ARS CH T ML W/ ARS CH T ML W/ ARS CH T ML W/ ARS CH	ML ML ML ML		30.9 31.1 32.8 26.3 25.6											SV SV

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

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Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-09

Sample Number	Depth	Visual Classification	uscs	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A	0	ST BR CH4 W/ RT	CH4		35.2												
1B	1	ST BR CH4 W/ RT	CH4		35.2								91	21	70		ORG% = 5.88%
1C	2	ST BR CH4 W/ RT	CH4		33.9												5.88%
NS	3	NS	NS														
2A	4	M GR & T CH4 W/ ARS & LNS ML CC	CH4	1	39.5												
2B	5	SO GR & T CH4 W/ ARS & LNS ML, CC	CH4	14	52.7	69.1	105	98	UU	0	311		98	30	68	0.625	ORG% = 2.12%
NS	6	NS	NS														2.12/0
3A	8	M GR & BR CH4 W/ ARS & LNS ML, CC	CH4	1	29.1												
3B	9	M GR & BR CH4 W/ ARS & LNS ML, CC	CH4		28.7								77	25	52		
NS	10	NS	NS	1													
4A	12	SO GR CHOA	CHOA		95.4												
4B	13	SO GR CHOA	CHOA	10	91.7	47.1	90.3	96	UU	0	387		130	32	98	0.375	ORG% = 12.01%
4C	14	SO GR CH3 W/ ARS ML, O, SL	СНЗ		63.0												12.0170
4D	15	SO GR CH3 W/ ARS ML, O, SL	CH3		67.4												
5A	16	M GR CH3 W/ ARS ML	CH3		38.0												
5B	17	SO GR CH3 W/ ARS ML	CH3	13	37.5	81.1	112	93	UU	0	468		64	23	41	0.150	
5C	18	M GR CH3 W/ ARS ML	CH3		38.4								61	21	40		-#200 = 88.3% ORG% = 3.31%
5D	19	M GR CH3 W/ ARS ML	CH3		42.3												
6A	20	M GR CH2 W/ CC, ARS ML	CH2		40.3												
6B	21	SO GR CL4	CL4	7	32.6	86.4	115	92	UC		446	893	41	17	24	0.500	
6C	22	SO GR CL4 W/ CC	CL4		31.5												
NS	23	NS	NS														
7A	24	ST GR CH3 W/ ARS ML, SIF	CH3		43.0												
7B	25	SO GR CH3 W/ ARS ML, SIF	CH3	4	52.7	68.0	104	96	UU	0	462		67	16	-	0.500	PD
7C	26	SO GR CH2 W/ ARS & LNS ML	CH2	11	50.8	71.3	108	100	ОВ	0	374	747	54	22	32	0.200	PD
NS	27	NS	NS	1													
8A	28	ST GR CH2 W/ ARS & LNS ML, RT	CH2		28.1												
8B	29	ST LGR CH2 W/ ARS & LNS ML, RT	CH2	15	25.9	96.6	122	94	UC		1127	2253	52	19	33	0.750	PD
8C	30	ST GR CH2 W/ ARS & LNS ML, RT	CH2		27.9												
NS	31	NS	NS														
9A	32	ST GR CL4 W/ CC	CL4		20.9												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-09

Sample Number	Depth	Visual Classification	USCS	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	ΡI	TORVANE (tsf)	Other Tests
9B	33	ST GR CL4 W/ CC	CL4		20.2												
9C	34	ST GR & T CL4 W/ CC	CL4	15	24.1	100	125	96	UU	0	1570		41	17	24	0.875	
9D	35	ST GR CL4 W/ CC	CL4		24.2												
10A	36	ST GR & T CL4 W/ CC	CL4		30.0												
10B	37	ST GR & T CL4 W/ CC	CL4		29.3												
10C	38	ST GR CL4 W/ CC	CL4	12	26.0	97.5	123	96	UC		1111	2221	40	18	22	0.875	
10D	39	M GR & T CL6 W/ CC	CL6		31.5												
11A	40	M GR & T CL6 W/ CC	CL6		36.6												
11B	41	M GR & T CL6 W/ CC	CL6		36.0												
11C	42	M GR & T CL6 W/ CC	CL6		31.7												
NS	43	NS	NS														
12A	44	T ML	ML		26.7												
12B	45	T ML	ML		25.8												
PB-12B	46	T ML	ML		28.3												-#200 =
																	80.2%
PB-13	48.5	T ML	ML		28.8												
PB-14	50	T ML	ML		27.4												
PB-15	53.5	T ML	ML		29.7												
PB-16	56	T ML	ML		33.1												-#200 =
l																	74.3%
PB-17	58.5	T ML	ML		26.5												
PB-18	61	M GR CL6 W/ SIF	CL6		36.9												
19A	62.5	ST GR CL6 W/ SIF	CL6		28.8												
19B	63.5	ST GR CL6 W/ SIF	CL6	4	29.7	92.1	119	96	UC		1097	2193	43	16	27	0.500	
19C	64.5	ST GR CL6 W/ SIF	CL6		29.1												
NS	65.5	NS	NS														
20A	66	ST GR & T CL6	CL6	l _	32.4					_							
20B	67	ST GR CL6	CL6	7	33.0	89.4	119	100	UU	0	1037		48	18	30	0.500	
20C	68	ST GR CL6	CL6		31.5												
20D	69	ST GR CL6	CL6		31.2												
21A	70	ST GR & T CH2 W/ ARS & LNS ML	CH2	_	32.8								_				
21B	71	ST GR CH2 W/ ARS & LNS ML	CH2	8	32.5	88.8	118	97	UC		1178	2357	50	15	35	0.500	
21C	72	ST GR & T CH2 W/ ARS & LNS ML	CH2		31.8												
NS	73	NS	NS														
22A	74	ST GR CH4 W/ ARS ML, SIF	CH4		44.4												
22B	75	ST GR CH4 W/ ARS ML, SIF	CH4		42.9												
22C	76	M GR CH4 W/ ARS ML, SIF	CH4	5	45.3	74.2	108	96	UU	0	521		91	18	73	0.625	

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-09

Sample Number	Depth	Visual Classification	USCS	E(f)		Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	ΡI	TORVANE (tsf)	Other Tests
22D	77	ST GR CH4 W/ ARS ML, SIF	CH4		44.2												
23A	78	ST GR CH4 W/ SIF	CH4		42.9												
23B	79	ST GR CH4 W/ SIF	CH4		43.2												
23C	80	ST GR CH4 W/ ARS ML	CH4	3	41.8	79.4	113	100	UC		1633	3266	75	19	56	0.625	
23D	81	ST GR CH4 W/ SIF	CH4		44.0												
24A	82	ST GR CH4 W/ ARS ML, WD, SIF	CH4		49.3												
24B	83	ST GR CH4 W/ ARS ML, WD, SIF	CH4		50.6												
24C	84	ST GR CH4 W/ ARS ML, SIF	CH4	3	44.0	77.2	111	100	UU	0	1370		74	20	54	0.625	
24D	85	ST GR CH4 W/ ARS ML, WD, SIF	CH4		51.6												
25A	86	ST GR CH4 W/ ARS ML, WD, SIF	CH4		48.1												
25B	87	ST GR CH4 W/ ARS ML, WD, SIF	CH4		47.7												
25C	88	ST GR CH4 W/ ARS ML, WD, SIF	CH4		47.1												
25D	89	ST GR CH4 W/ ARS ML, WD, SIF, SL	CH4		49.1												
26A	90	ST GR CH3 W/ ARS & LNS ML	CH3		50.3												
26B	91	ST GR CH3 W/ ARS & LNS ML	CH3	2	44.3	76.9	111	100	UC		1631	3263	69	17	52	0.500	
26C	92	ST GR CH3 W/ ARS & LNS ML	CH3		50.2												
NS	93	NS	NS														
27A	94	ST GR CH4 W/ ARS & LNS ML	CH4		44.6												
27B	95	ST GR CH3 W/ ARS & LNS ML, O	CH3	4	38.1	81.9	113	96	UU	0	1082		63	18	45	0.500	
27C	96	GR ML W/ SIF, WD, ARS CH	ML		28.6												
27D	97	GR ML W/ SIF, WD, ARS CH	ML		27.4												-#200 =
			01.0														52.3%
28A	98	M GR CL6 W/ SIF, WD	CL6	_	28.7			٠. ا									
28B	99	M GR & T CL6 W/ SIF, WD	CL6	9	28.7	92.5	119	94	UC		653	1306	45	17	28	0.625	
28C	100	M GR CL6 W/ SIF, WD	CL6		29.9												
NS	101	NS	NS														
29A	102	SO GR CL4 W/ SIF, ARS SP	CL4		29.3												
30A	103	ST GR CL4 W/ ARS SP	CL4		20.2												
PB-31	104	VST GR CL4 W/ ARS SP	CL4		21.3								28	16	12		-#200 =
DD 00	400 5	CD CM M/C M/D	CNA		07.5												68.2%
PB-32	106.5	GR SM W/ O, WD	SM		27.5												
PB-33	109	GR SM	SM		22.8												
PB-34	111.5	GR SP	SP SP		27.1												
PB-35	114	GR SP	_		28.3												
PB-36	ı	GR SP	SP		27.0												
PB-37	119	GR SP W/ ARS CH	SP SP		27.4												
PB-38	121.5	GR SP W/ ARS CH	5P		29.7												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

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Project:	LOUISIANA,	STATE	OF	- COASTAL	PROTECTION	AND	RESTORATION	Assigned By:
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Project Number: 24384 Current Date: 12/31/2020

Boring: SB-09

Sample Number PB-39	Depth 123.5	Visual Classification  GR SP	uscs SP	E(f)	<b>w</b> % 25.0	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

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Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-11

Sample Number	Depth	Visual Classification	USCS	E(f)	W%	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A	0	SO GR & T CH4 W/ ARS ML, RT, SL	CH4		64.5	07.0	400				400	0.7.6			00	0.500	
1B	1	SO GR & T CH4 W/ ARS ML, O	CH4	6	54.0	67.0	103	96	UC		488	976	92	26	66	0.500	ORG% = 6.07%
PB-2	2	M GR CH4 W/ ARS ML	CH4		61.3												0.0770
3A	4	SO GR CH4 W/ ARS ML	CH4		54.0												
3B	5	SO GR & T CH4 W/ ARS ML, WD	CH4	15	55.3	67.6	105	100	UU	0	352		77	30	47	0.500	CONS -#200 = 97.3%
3C	6	SO GR CH4 W/ ARS ML, SL	CH4		54.8												
3D	7	SO GR CH4 W/ ARS ML, SL	CH4		55.0												
4A	8	SO GR CH4 W/ ARS ML, RT, WD	CH4		85.3												
NS DD 5	9	NS	NS										407	00	0.5		ODON/
PB-5	10	VSO GR CHOO W/ WD, RT	CHOA		126.0								127	32	95		ORG% = 15.68%
6A 6B	12 13	VSO GR CHOC VSO DGR CHOC	CHOC	13	191.8 172.3	29.5	80.2	99	UU	0	99		203	41	162	0.120	ORG% =
l ob	13	V30 DGR CHOC	СПОС	13	112.3	29.5	00.2	99		"	99		203	41	102	0.120	16.59%
6C	14	VSO BR PT	PT		253.4												
NS	15	NS	NS														
PB-7	16	VSO GR CHOA	CHOA		106.8												ORG% =
8A	20	ST GNG CH2 W/ ARS ML, RT	CH2		30.8												9.63%
8B		M GNG CH2 W/ ARS ML, RT	CH2	15	27.3	94.4	120	94	UC		554	1108	53	20	33	0.700	ORG% =
			01.12		27.0	0	.20				00.		00	20	33	0.7 00	3.13% -#200 = 99.4%
8C		ST GNG CH2 W/ ARS ML, RT	CH2		25.8												
NS		NS	NS		<del></del>												
9A	24	VST GR & T CH3 W/ ARS ML, RT, CC, SL	CH3	١	20.8				l								
9B	25	VST T & G CH3 W/ ARS ML, CC	CH3	14	21.6	106	129	97	UU	0	2351		65	15	50	1.250	
NS 10A	26 28	NS SO T & GR CH3 W/ ARS ML, RT, O	NS CH3		 35.6												
10A 10B		M GR & T CL6 W/ RT	CL6		31.5												
10C		M GR & T CL6	CL6	10	30.0	91.7	119	96	UC		788	1576	49	24	25	0.500	
10D		M GR & T CL6	CL6	.	31.6	J					'55		-5			0.500	
11A		M GR & T CL6	CL6		31.3												
11B		M GR & T CL6	CL6		31.9												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

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File Name: 24384

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Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-11

Sample Number 11C 11D 12A 12B PB-13	Depth  34  35  36  37  38	Visual Classification  M GR & T CL6 W/ CC ST BR & GR CH2 W/ ARS & LNS ML BR ML W/ ARS SP, O VST BR & GR CL6 T ML	USCS CL6 CH2 ML CL6 ML	E(f)	w% 39.1 36.7 21.5 30.3 30.6	Dry Dens (pcf) 81.0	Wet Dens (pcf) 113	<b>Sat</b> % 98	Shear Test Type UU	Angle 0	Cohesion (psf) 587	Unconf. Comp. Str.	LL 43	PL 20	PI 23	TORVANE (tsf) 0.500	Other Tests -#200 = 75.2%
PB-14	41	T ML	ML		26.6												75.270
PB-15	43.5	BR ML W/ ARS SP	ML		28.1												
PB-16	46	ST BR CL4 W/ ARS SP	CL4		35.1												
PB-17	48.5	BR ML	ML		27.6												SV
PB-18	51	BR ML	ML		26.3												
PB-19	53.5	ST GR CH3 W/ ARS SM	CH3		50.9												
20A	55	ST T & GR CH3 W/ ARS ML, SL	CH3		33.6												
20B	56	ST T & GR CH3 W/ SL, ARS ML	CH3	2	37.2	80.1	110	90	UC		968	1936	70	28	42	0.750	
20C	57	ST T & GR CH3 W/ ARS ML, SL	CH3		33.4												
NS	58	NS	NS														
21A	59	ST GR & T CH4 W/ ARS ML, CC, SL	CH4		45.4												
21B	60	ST GR & T CH4 W/ ARS ML ,CC, SL	CH4	45	28.3	00.0	440				4004		_,	40		0.750	
21C NS	61	ST LGR & T CH4 W/ CONC, SL	CH4 NS	15	35.4	82.9	112	92	UU	0	1061		74	18	56	0.750	
22A	62 63	NS VST T & GR CH4 W/ ARS & LNS ML, SIF, CC	NS CH4		 30.1												
22A 22B	64	VST T & GR CH4 W/ ARS & LNS ML, SIF, CC	CH4		37.3												
22C	65	ST BR & LGR CH4 W/ SIF, SL	CH4	4	36.1	83.3	113	95	UC		1482	2964	77	30	47	1.000	
NS	66	INS	NS	"		00.0	110	"			1402		''		-11	1.000	
23A	67	VST T & GR CH4 W/ ARS ML, O, SIF	CH4		36.4												
23B	68	VST T & GR CH4 W/ ARS ML, O, SIF	CH4		38.9												
23C	69	VST T & GR CH4 W/ ARS ML, O, SIF	CH4		39.2												
NS	70	NS	NS														
24A	71	VST GNG & T CH4 W/ ARS ML, CC, SL	CH4		31.2												
24B	72	ST LGR & T CH4 W/ CONC, SL	CH4	11	40.6	78.1	110	94	UU	0	1252		101	26	75	1.125	
24C	73	VST GNG & T CH4 W/ ARS ML, CC, SL	CH4		30.0												
24D	74	VST GNG & T CH4 W/ ARS ML, CC, SL	CH4		41.5												
25A	75	VST GR CH4 W/ ARS ML, CC, SL	CH4	l .	41.2			l									
25B	76	ST LGR CH4 W/ SL	CH4	1	39.6	77.6	108	91	UC		1179	2359	93	28	65	1.000	
25C	77	VST BR & GR CH3 W/ ARS & LNS ML, CC,	CH3		28.6												
NS	78	SL NS	NS														

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

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Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-11

Sample Number	Depth	Visual Classification	USCS	E(f)		Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
26A 26B	79 80	ST BR & GR CH2 W/ ARS & LNS ML, SL ST BR & GR CH2 W/ ARS & LNS ML, SL	CH2 CH2	9	36.3 30.5	89.7	117	93	lυυ	0	1516		52	22	30	0.875	
PB-27	81	BR & GR SM	SM	"	22.7	09.7	117	93	00	"	1310		32		30	0.873	
PB-28	-	BR & GR CL6 W/ ARS SP	CL6		35.4												sv
PB-29	86	IGR & BR SM	SM		28.5												"
PB-30	88.5	ST GR CH3 W/ ARS SM	CH3		39.4												
31A	90	BR & GR SM	SM		26.9												
31B	91	SO GR CH3 W/ ARS SM, O	CH3		46.7												-#200 =
																	40.8%
NS	92	NS	NS														
32A	94	ST GR CH4 W/ ARS ML, SIF, SL	CH4		46.0												
32B	95	ST GR CH4 W/ ARS ML, SIF, SL	CH4		45.4												
32C	96	ST GR CH4 W/ SL	CH4	2	41.3	76.5	108	92	UC		1597	3194	93	26	67	1.250	
NS	97	NS	NS														
33A	98	VST GNG CH4 W/ ARS SM, SL, WD	CH4		39.7												
33B	99	VST GNG CH4 W/ ARS SM, SL, WD	CH4		39.6												
33C	100	VST GNG CH4 W/ ARS SM, SL, WD	CH4		40.0												
NS	101	NS	NS														
34A	102	VST GNG CH4 W/ ARS ML, SL	CH4		34.3												
34B	103	VST GNG CH4 W/ SL	CH4	2	32.3	88.4	117	95	UU	0	2857		78	23	55	1.375	
34C	104	VST GNG CH4 W/ ARS ML, SL	CH4		43.5												
NS	105	NS	NS														
35A	106	VST GR CH2 W/ ARS ML, SL	CH2		45.2												
35B	107	VST GR CH2 W/ ARS ML, SL	CH2		44.6	05.0					4000	01.55		4.0		0.750	
35C		ST GNG CH2 W/ ARS ML, SL	CH2	3	33.0	85.9	114	93	UC		1083	2167	55	18	37	0.750	
NS	109	NS	NS														
36A	110	VST GNG CH3 W/ ARS ML, CC, SL	CH3		35.7												
36B NS	111 112	VST GNG CH3 W/ ARS ML, CC, SL NS	NS NS		39.0												
PB-37	114	VST GR CL4	CL4		29.5								32	21	11		-#200 =
PD-3/	114	VST GR CL4	CL4		29.5								32	41			98.9%
PB-38	117.5	VST GR CL4	CL4		29.5												30.370
PB-39	120	VST GR CH2	CH2		31.5												
PB-40	_	ST GR CL6	CL6		33.1								48	15	33		-#200 =
					55								``	'			99.4%
								<u> </u>	l								

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Checked by: RR

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-12

Sample Number	Depth	Visual Classification	uscs	E(f)	<b>W</b> %	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A	0	SO GR & T CH4 W/ ARS ML, RT	CH4		57.3												
1B	1	SO GR & T CH4 W/ ARS ML, RT	CH4	12	56.5	65.9	103	98	UU	0	416		100	27	73	0.375	ORG% =
																	7.06%
PB-2	2	SO GR & T CH3 W/ ARS ML	CH4		54.3												
3A	4	SO GR & T CH3 W/ ASR ML, WD, RT	CH4	_	57.8												
3B	5	SO GR & T CH4 W/ ARS ML, RT	CH4	8	60.5	64.2	103	100	UC		464	929	97	26	71	0.375	
NS	6	NS	NS														
PB-4	8	SO GR & DGR CHOA W/ WD	CHOA		146.5								118	31	87		ORG% =
5A	12	SO DGR CHOC	СНОС		142.5								268	44	224		8.75% ORG% =
SA	12	SO DGK CHOC	СПОС		142.5								200	44	224		27.08%
5B	13	SO DGR & BR CHOB	СНОВ	4	156.6	31.7	81.3	98	UC		415	831				0.375	PD PD
5C	14	M GR & DGR CL6 W/ O	CL6	10	26.0	98.6	124	98	UU		739	""	42	16	26	0.350	ORG% =
	'-	IN OR a BOR OLO W/ O	020	'	20.0	00.0	127				700		12		20	0.000	2.67%
NS	15	NS	NS														
6A	16	ST GNG CL6 W/ WD	CL6		25.1												
6B	17	ST GNG & T CL6 W/ WD	CL6	14	24.3	98.6	123	92	UU	0	1082		43	19	24	0.625	
6C	18	ST GNG CL6 W/ WD	CL6		25.2												
6D	19	ST GNG CL6 W/ WD	CL6		25.0												
7A	20	VST GR & T CH2 W/ ARS & LNS ML, CC	CH2		22.0												
7B	21	VST GR & T CH2 W/ ARS & LNS ML, CC	CH2		22.0												
7C	22	VST GR & T CH2 W/ ARS & LNS ML	CH2	8	21.9	102	125	90	UC		2207	4413	55	21	34	0.750	
NS	23	NS	NS														
8A	24	ST T & GR CL6 W/ CC	CL6		35.0												
8B	25	ST T & GR CL6 W/ CC	CL6		26.5												
8C	26	ST T & GR CL6 W/ CC	CL6	15	28.2	93.0	119	94	UU	0	1029		43	19	24	0.375	
8D	27	ST T & GR CL6 W/ CC	CL6		30.7												
9A	28	ST T & GR CL6	CL6		33.3												
9B	29	ST T & GR CL6	CL6		37.3												
9C	30	ST T & GR CL6	CL6	4	26.4	95.9	121	93	UC		1559	3117	40	19	21	0.500	
NS	31	NS	NS														
10A	32	M GR & T CL6 W/ CC	CL6		40.0												
10B	33	M GR & T CL6 W/ CC	CL6		34.0												
10C	34	M GR & T CL6 W/ CC	CL6	14	32.4	86.2	114	91	UU	0	706		48	18	30	0.375	
NS	35	NS	NS														
PB-11	36	T ML	ML		29.6												
PB-12	39.5	T ML	ML		29.1												

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Project:	LOUISIANA,	STATE	OF	- COASTAI	<b>PROTECTION</b>	AND	RESTORATION	Assigned By:
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Project Number: 24384 Current Date: 12/31/2020

Boring: SB-12

Sample Number PB-13	Depth 42	Visual Classification	ປຣCS ML	E(f)	₩% 37.7	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	ΡΙ	TORVANE (tsf)	Other Tests
PB-13 PB-14 15A 15B PB-16	44.5 46 47	T ML T ML ST GR & T CH3 W/ ARS ML, SL ST GR & T CH3 W/ ARS ML, SL T SM	ML CH3 CH3 SM	3	31.2 28.9 31.8 29.8	89.5	118	96	UC		768	1535	64	18	46	0.625	

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Checked by: RR

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION Assigned By: \_\_\_\_\_

Project Number: 24384 Current Date: 12/31/2020

Boring: SB-13

Sample Number	Depth	Visual Classification	uscs	E(f)	<b>W</b> %	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Shear Test Type	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
1A 1B PB-2	1	M GR CH4 W/ ARS ML, RT M GR & T CH4 W/ ARS ML, CC, SL M GR & T CH4 W/ ARS ML	CH4 CH4 CH4	6	55.6 47.0 50.5	72.7	107	96	UC		562	1124	101	31	70	0.375	ORG% = 6.58% -#200 = 96.4%
3A 3B 3C NS 4A	5 6 7	M GR & T CH4 W/ ARS ML, WD, SL M GR & T CH4 W/ ARS ML, WD, SL M GR & T CH4 W/ ARS ML, WD, SL NS M DGR CH3 W/ WD, O	CH4 CH4 CH4 NS CH3	5	62.2 62.5 62.2  55.4	59.7	97.0	92	υυ	0	771		104	30	74		ORG% = 6.77% -#200 =
4B 4C 4D 5A 5B		M DGR CH3 W/ WD, O ST GR CL6 W/ O ST GR CL6 W/ O ST GR CL6 W/ RT ST GR CL6 W/ RT	CH3 CL6 CL6 CL6 CL6	15 15	44.4 25.5 37.7 26.8 25.9	76.5 97.6	110 122	99 94	UC	 0	511 1094	1023	69 40	24 20	45 20	0.375 0.375	97.7%
5C 5D 6A 6B 6C NS 7A	14 15 16 17 18 19	ST GR CL6 W/ RT ST GR CL6 W/ RT ST LGR & T CH3 W/ ARS ML ST LGR & T CH3 W/ ARS ML ST LGR & T CH3 W/ ARS ML NS SO T & GR CL4	CL6 CL6 CH3 CH3 CH3 NS CL4	15	24.3 25.7 34.8 36.6 38.2  37.4	98.2	122	92	UU	0	1089		49	20	29	0.375	
7B 7C NS 8A	21 22 23 24	SO T & GR CL4 M T & GR CH4 W/ ARS ML, SL NS SO GR & T CH3 W/ ARS ML	CL4 CH4 NS CH3	2	37.1 41.3  43.3	76.6	108	92	UC		675	1350	79	26	53	0.375	
8B 8C NS 9A 9B 9C NS	26 27 28 29 30	M T & GR CH3 W/ ARS ML ST T & GR CH3 W/ ARS ML NS ST T & GR CH3 W/ ARS & LNS ML, SL ST T & GR CH3 W/ ARS & LNS ML, SL ST T & GR CH3 W/ ARS & LNS ML, SL NS	CH3 CH3 NS CH3 CH3 CH3 NS	15	48.9 34.9  42.1 42.2 43.5 	85.9	116	97	υυ	0	1248		60	23	37	0.500	

Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

EUSTIS ENGINEERING L.L.C.

Checked by: RR

Project:	LOUISIANA,	STATE	OF -	COASTAL	PROTECTION	AND	RESTORATION	Assigned By:
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Project Number: 24384 Current Date: 12/31/2020

Boring: SB-13

Sample Number 10A	Depth 32	Visual Classification BR ML W/ LYS CH	uscs ML	E(f)	29.8	Dry Dens (pcf)	Wet Dens (pcf)	Sat %	Туре	Angle	Cohesion (psf)	Unconf. Comp. Str.	LL	PL	PI	TORVANE (tsf)	Other Tests
10B		M GR CL6 W/ ARS SP	CL6	15	35.9	84.6	115	98	UU	0	573		48	21	27	0.150	
NS		NS	NS														
11A		BR ML	ML		30.1												
11B		BR ML	ML		29.3												
PB-12		BR & GR SM	SM		29.2												
PB-13		BR & GR SM	SM		29.2												
PB-14		BR & GR SM	SM		29.3												
PB-15		BR & GR SM	SM		29.1												
PB-16	48.5	ST GNG CH3 W/ ARS ML	CH3		46.7												

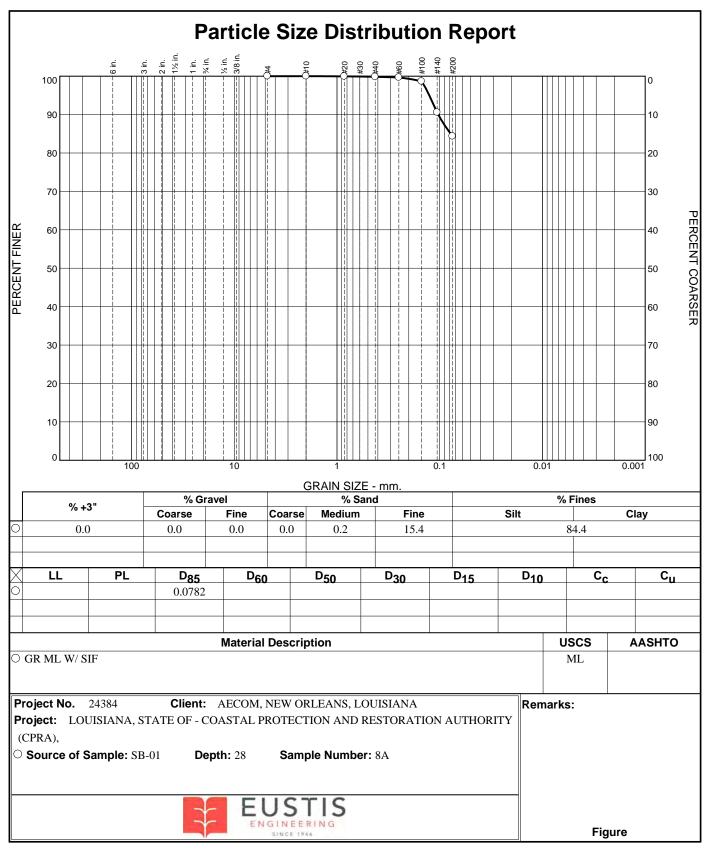
Remarks: MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN

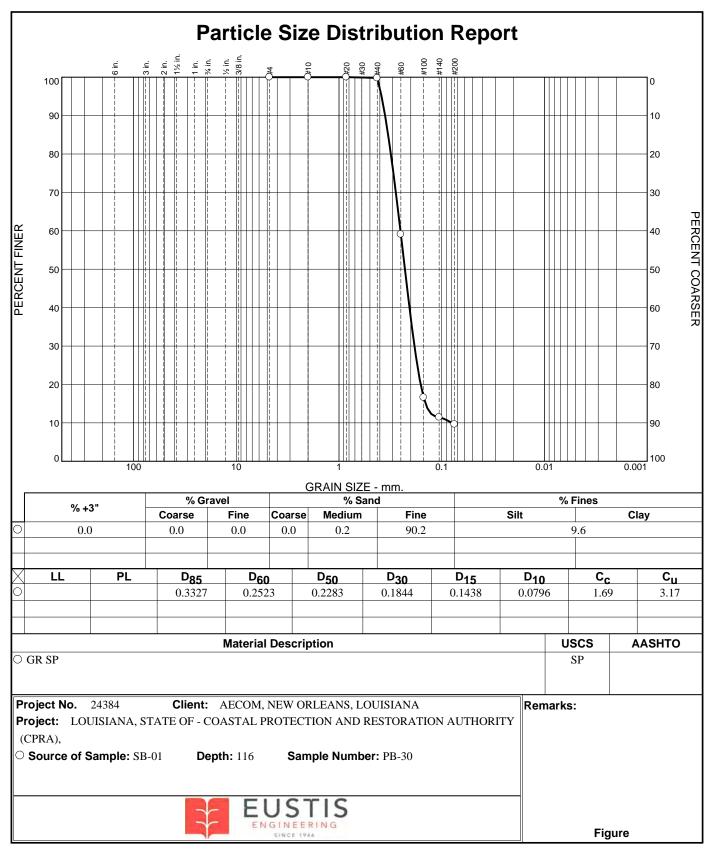
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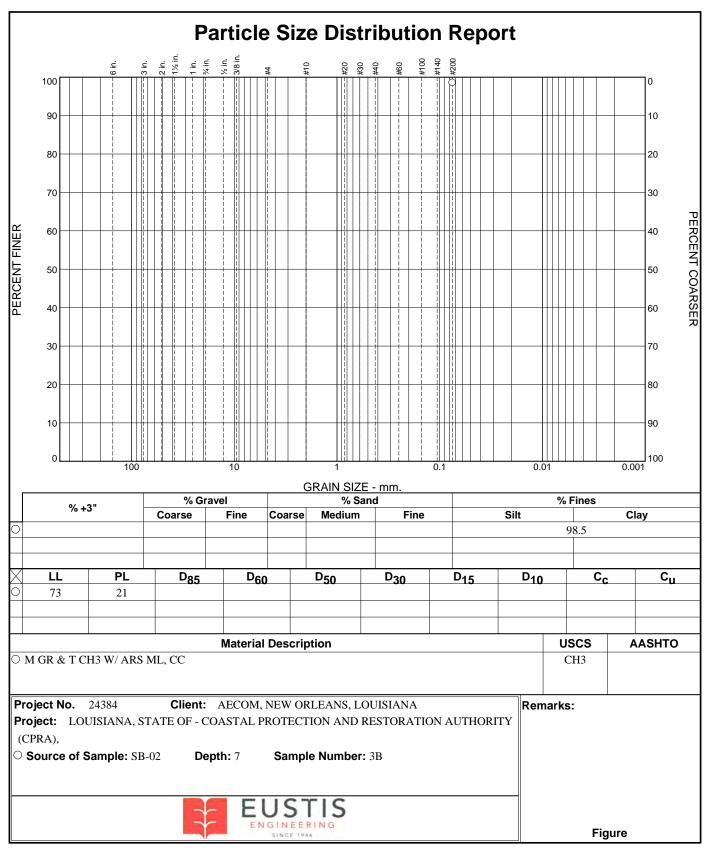
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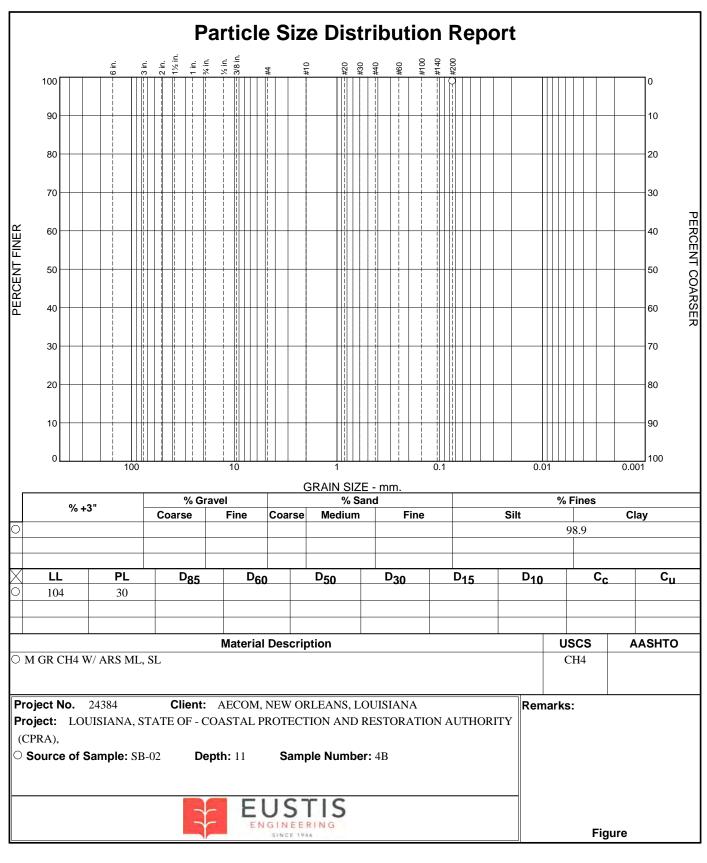
## APPENDIX VI

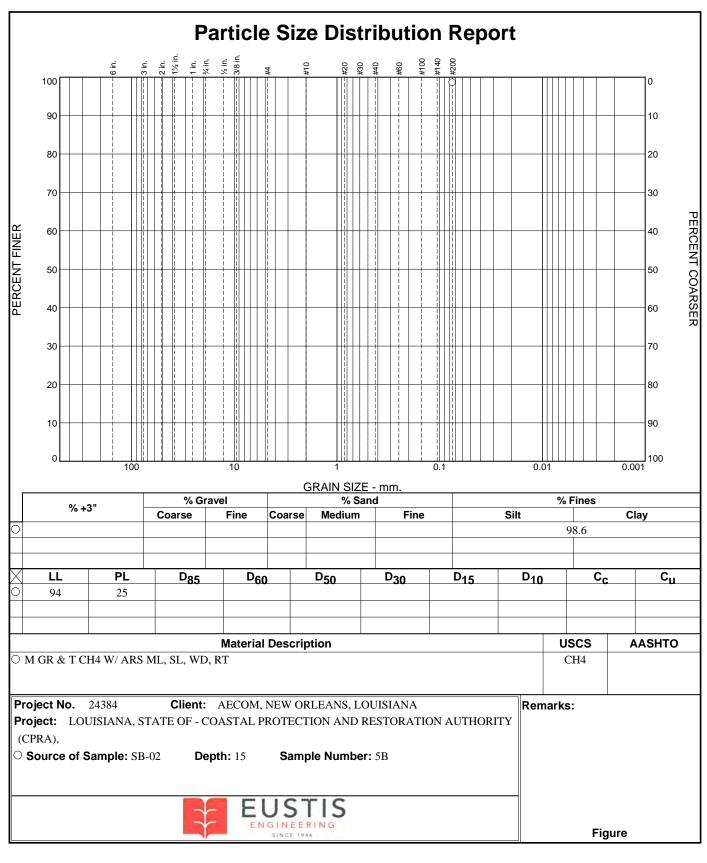
SUMMARY OF GRAIN SIZE TEST DATA (SDR-A, SDR-B, SDR-C)

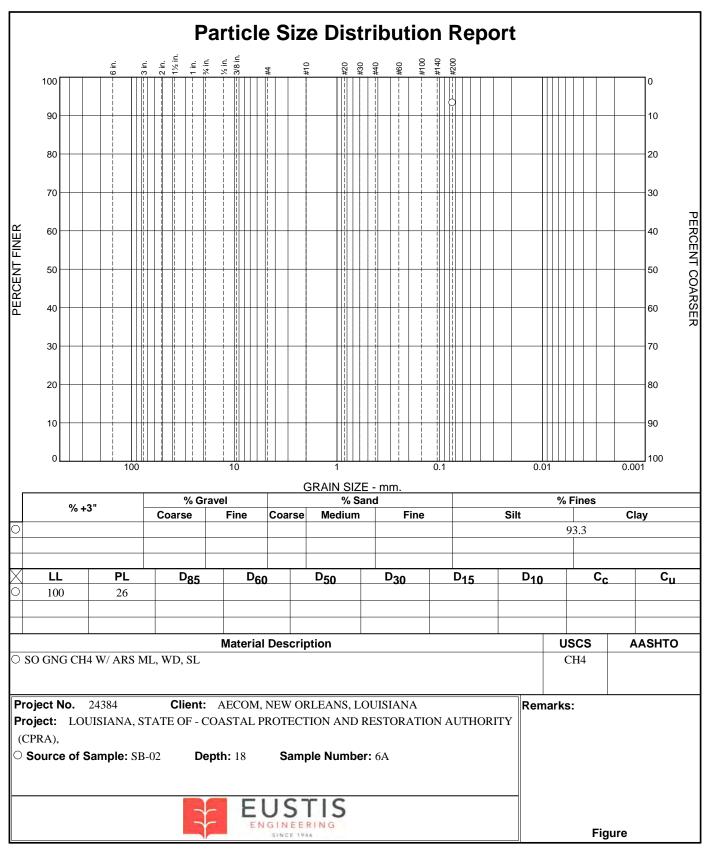


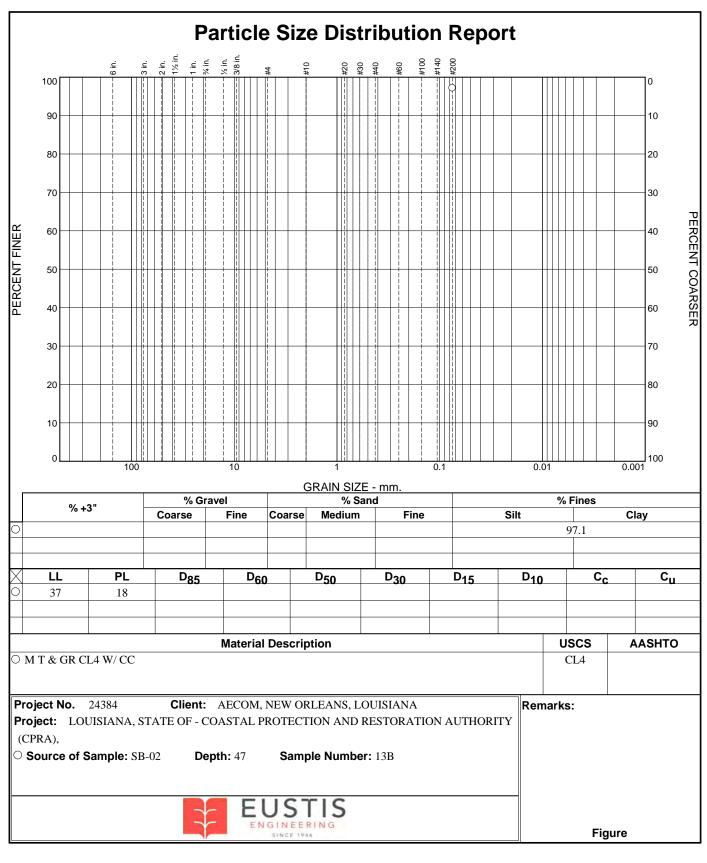


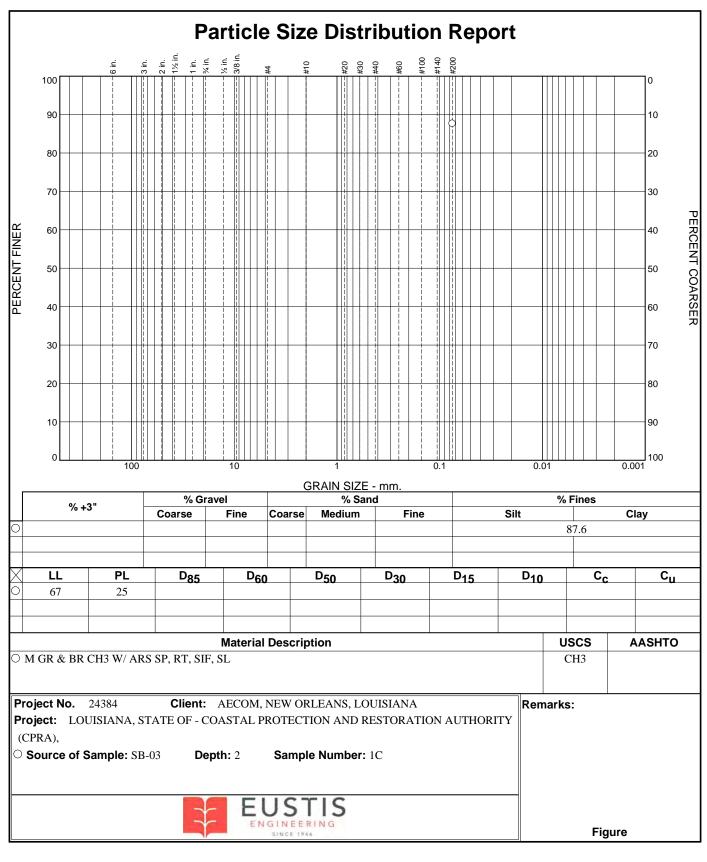


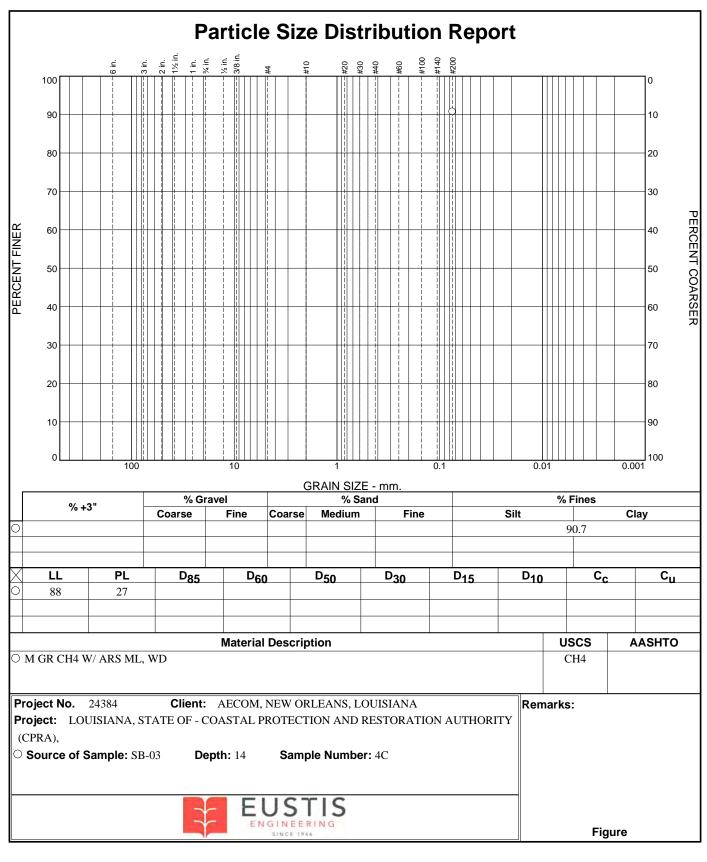


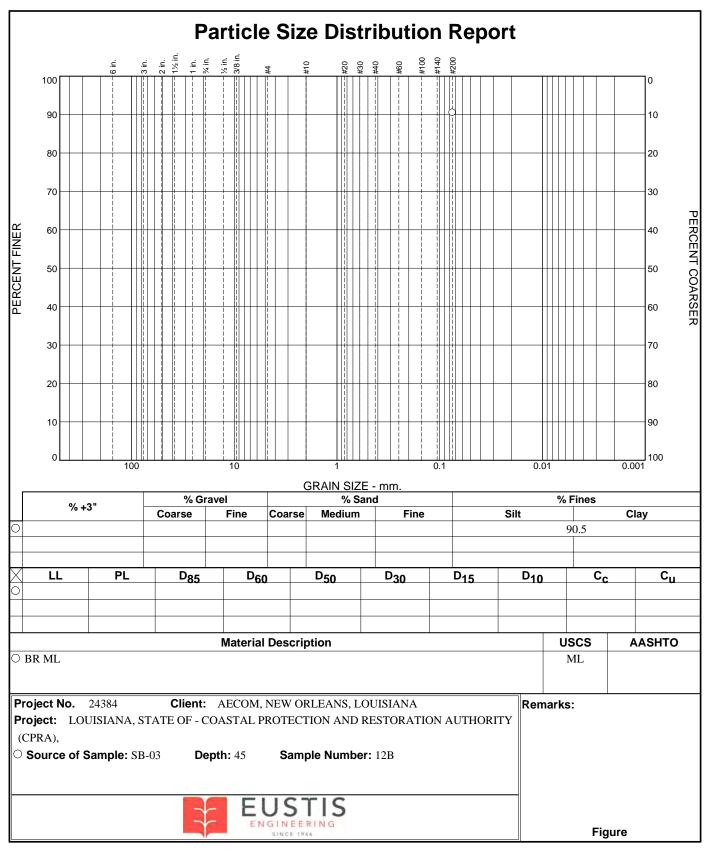


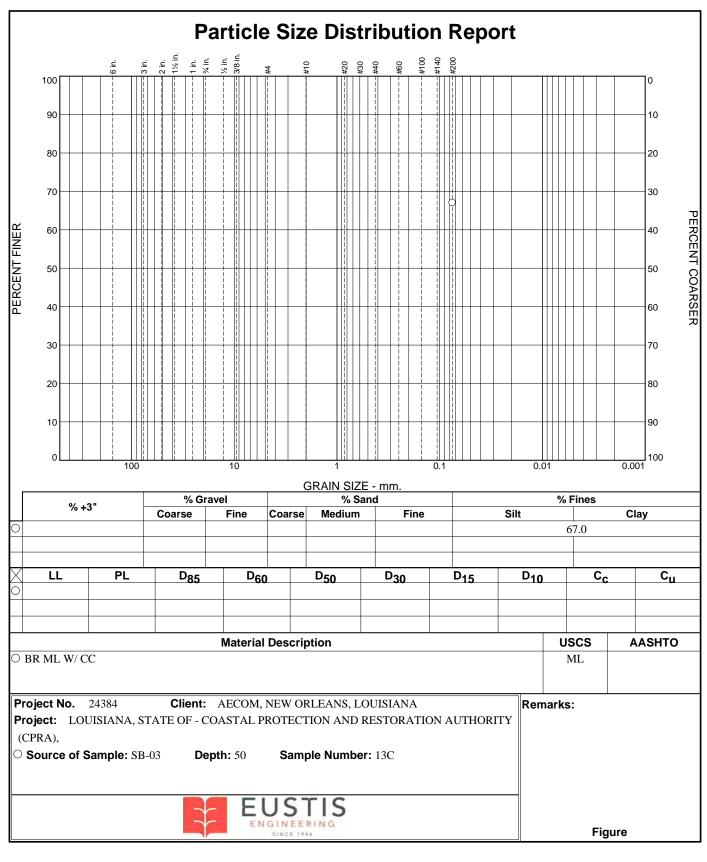


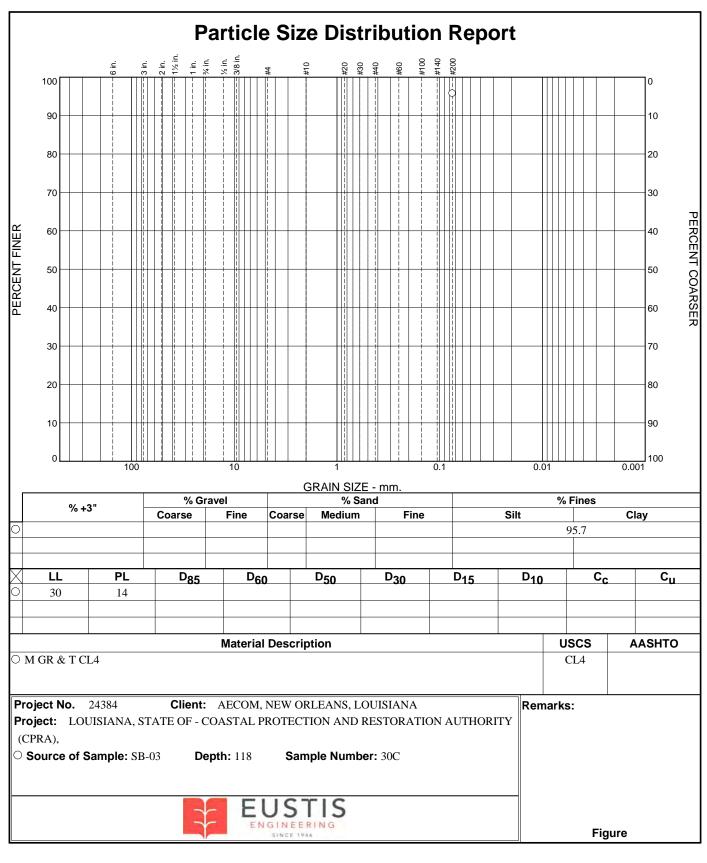


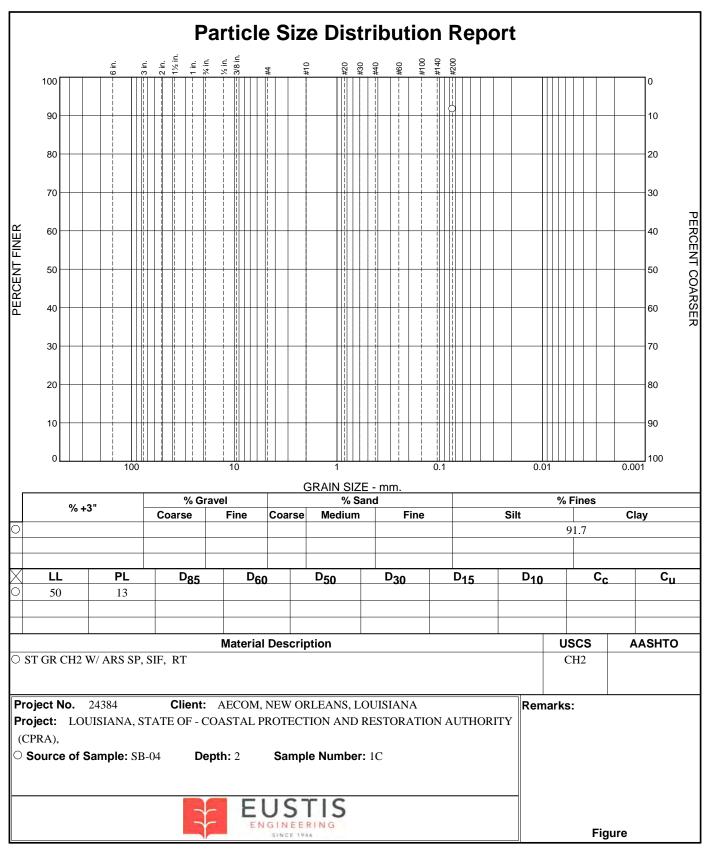


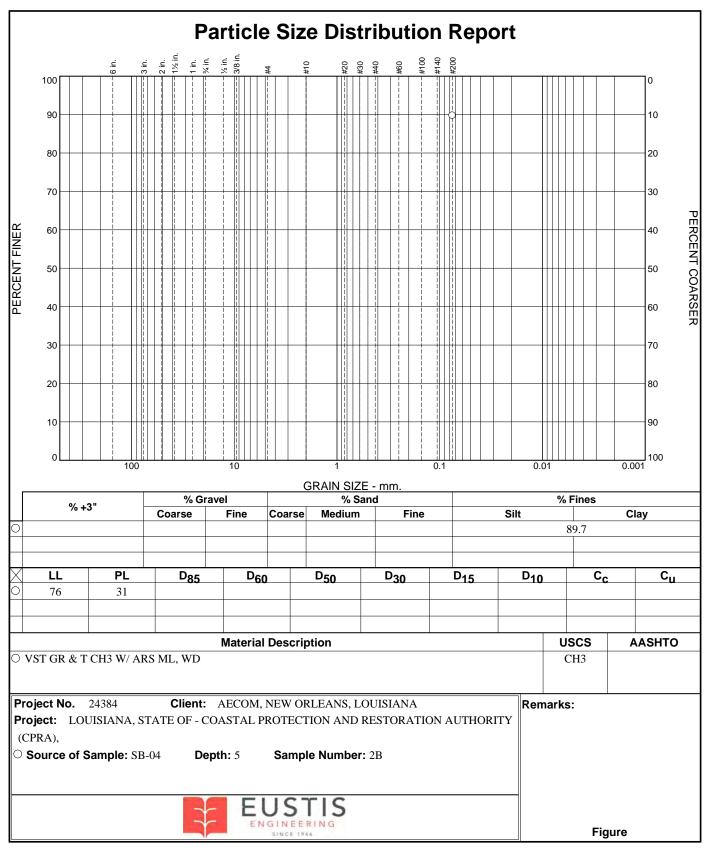


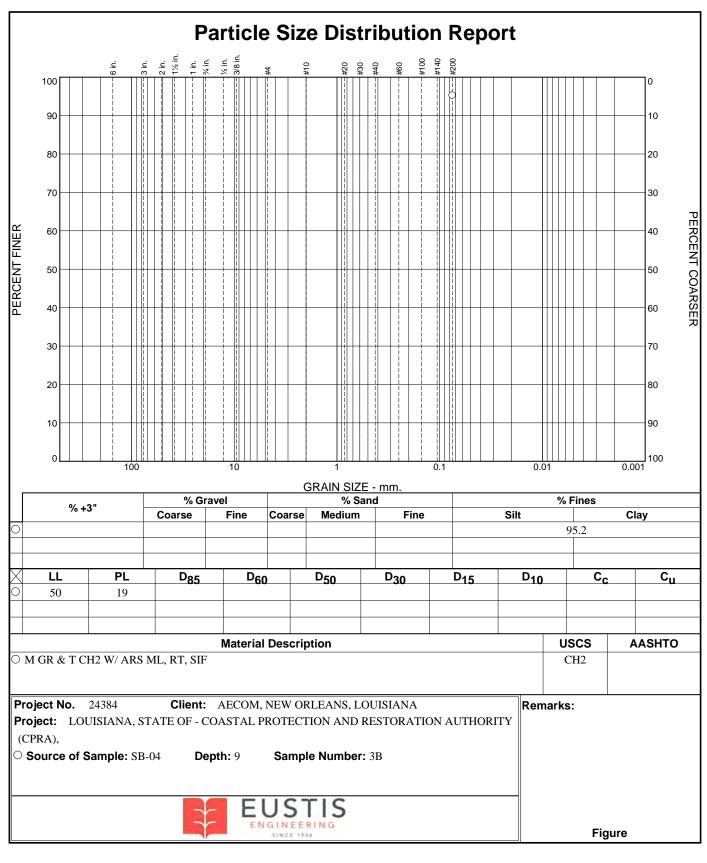


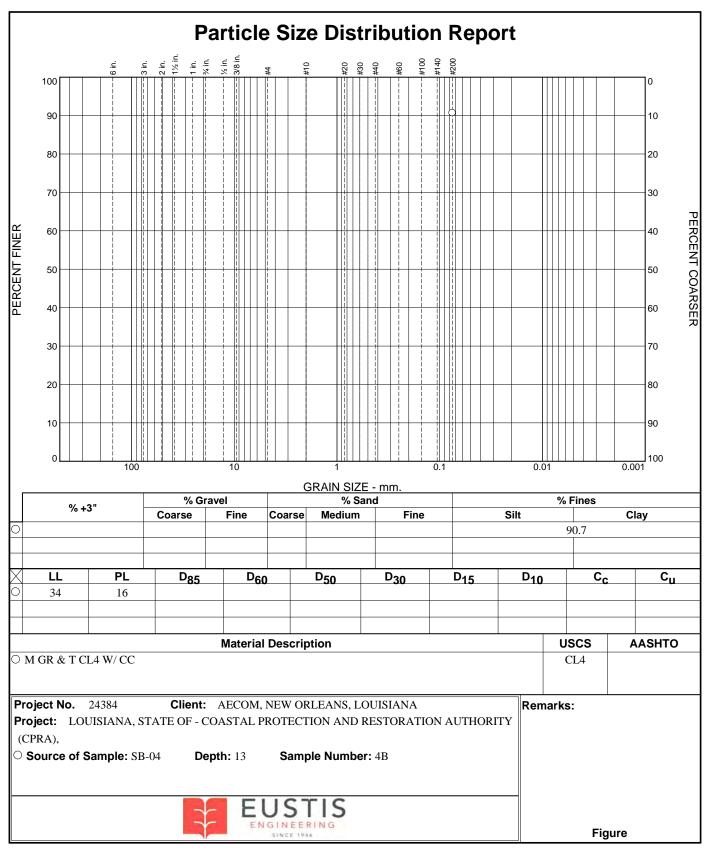


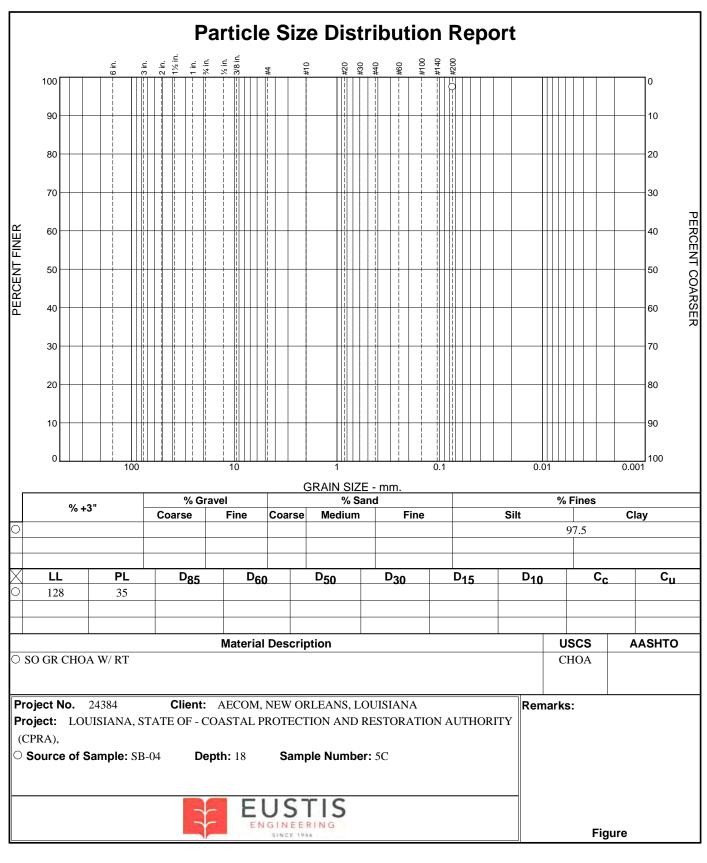


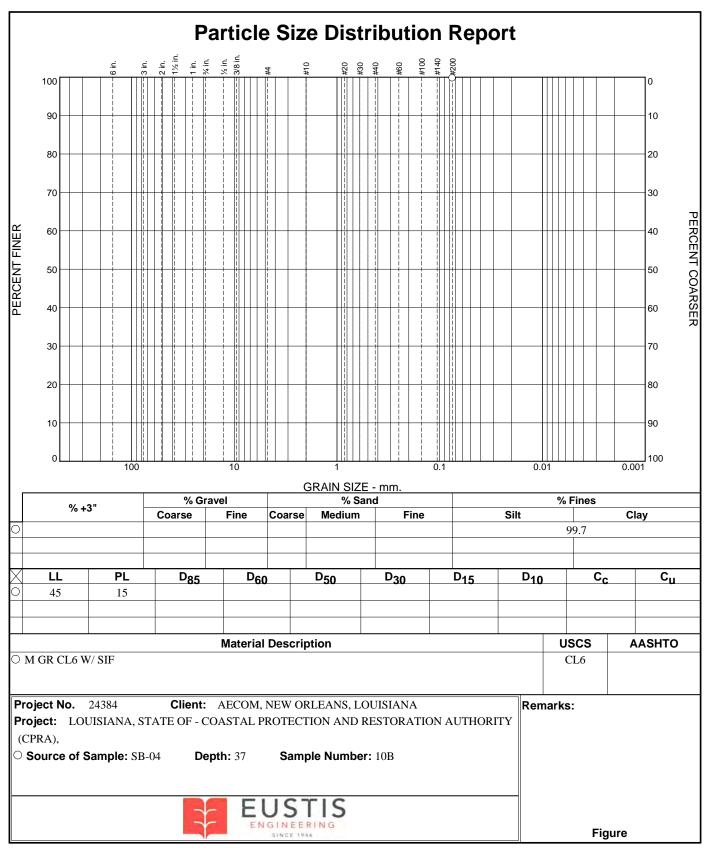


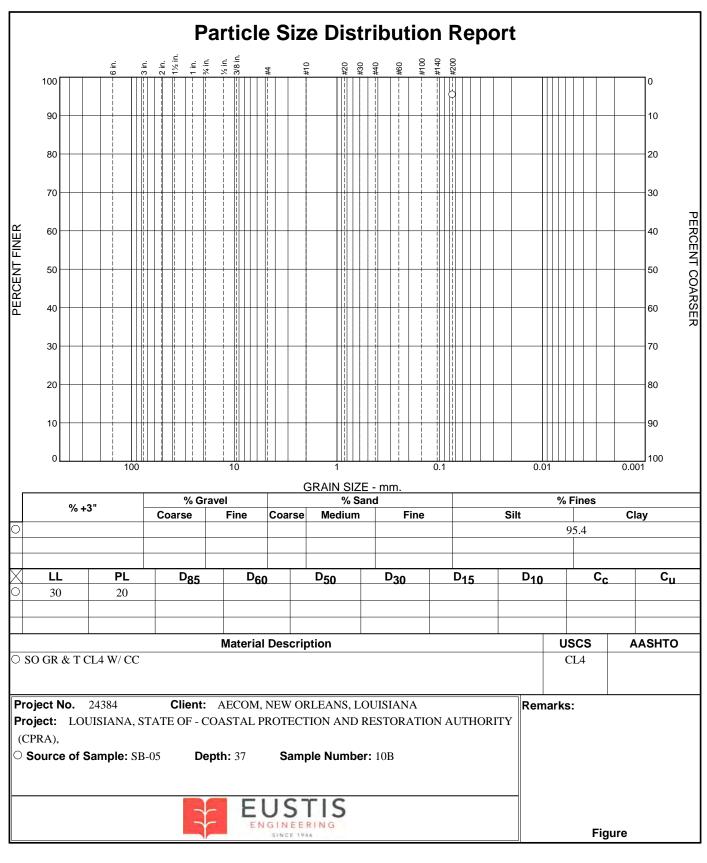


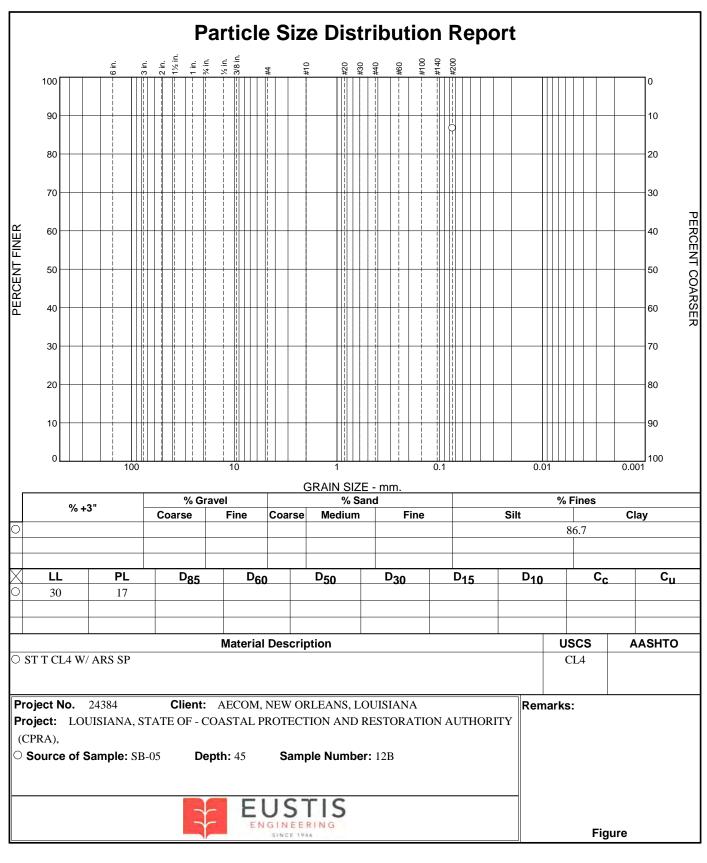


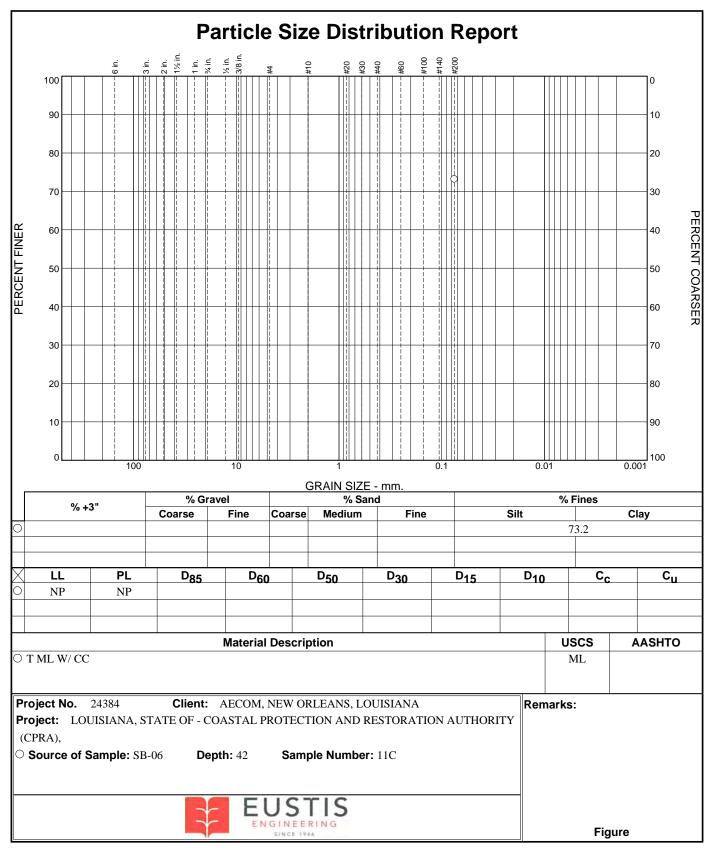


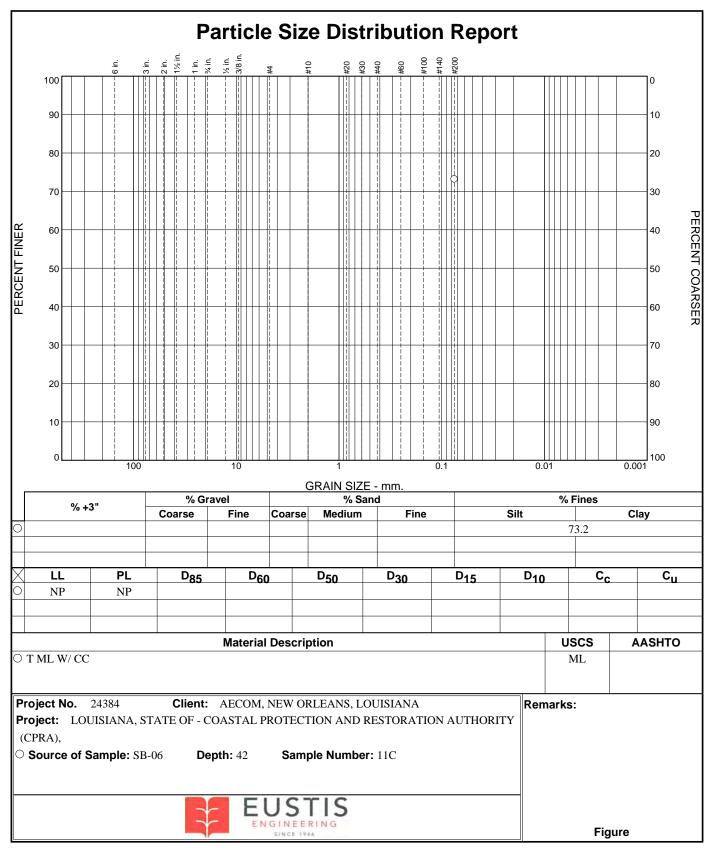


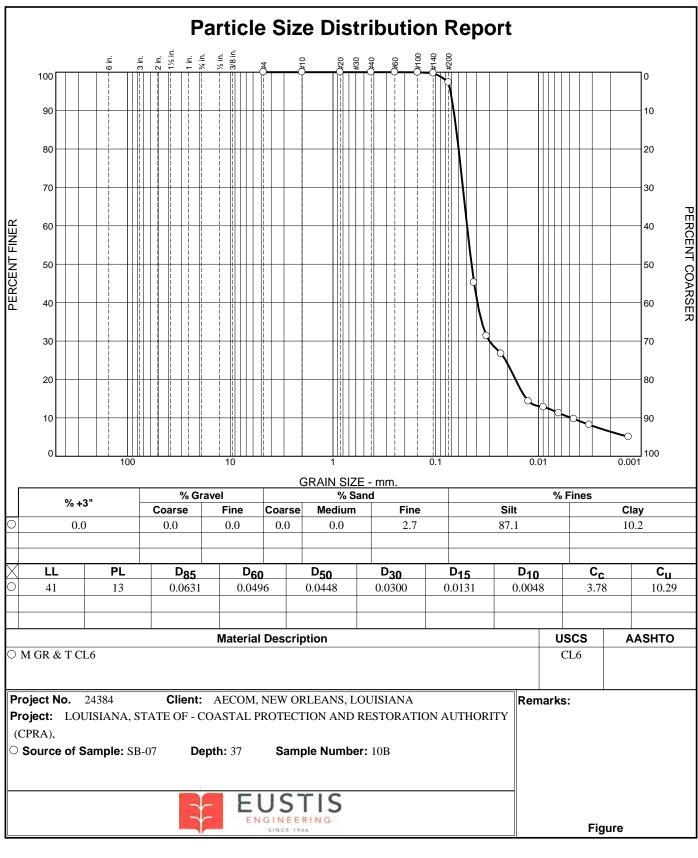


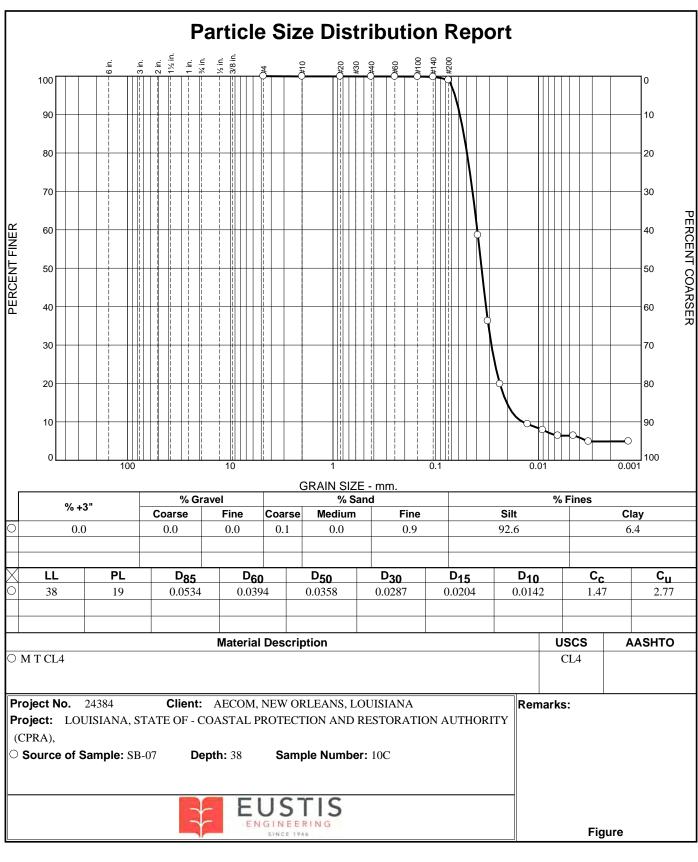


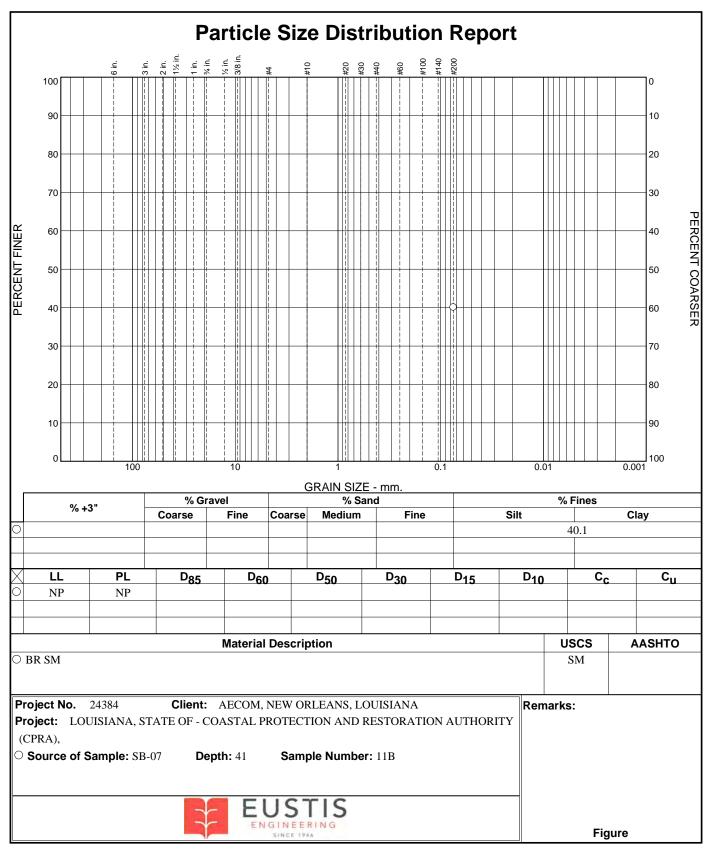


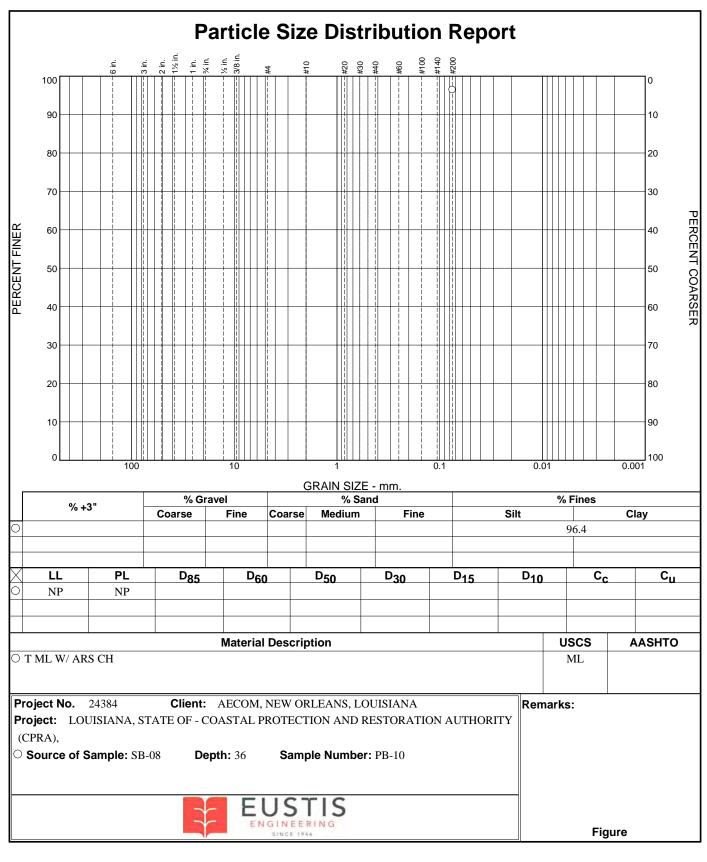


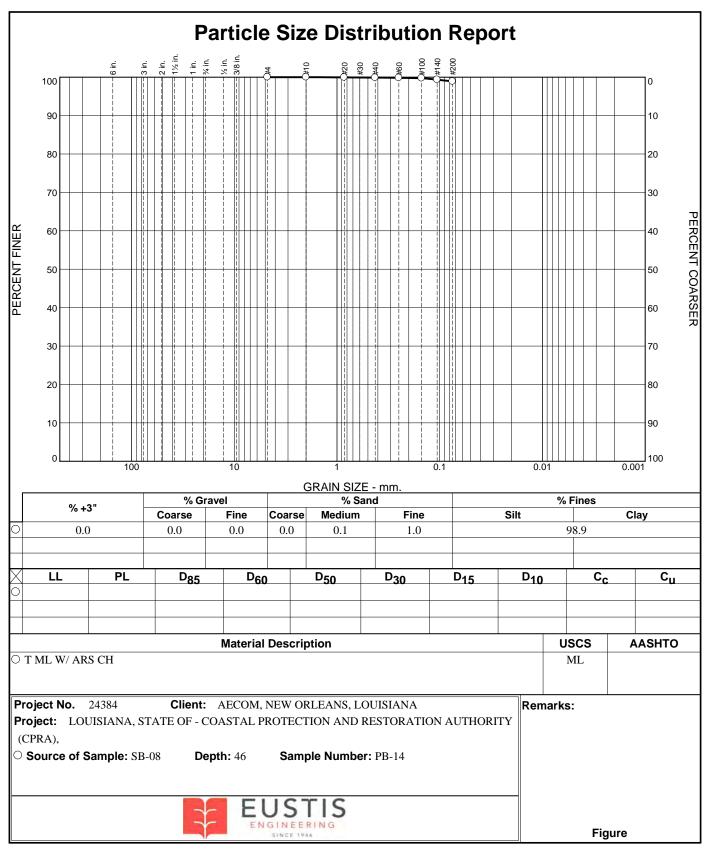


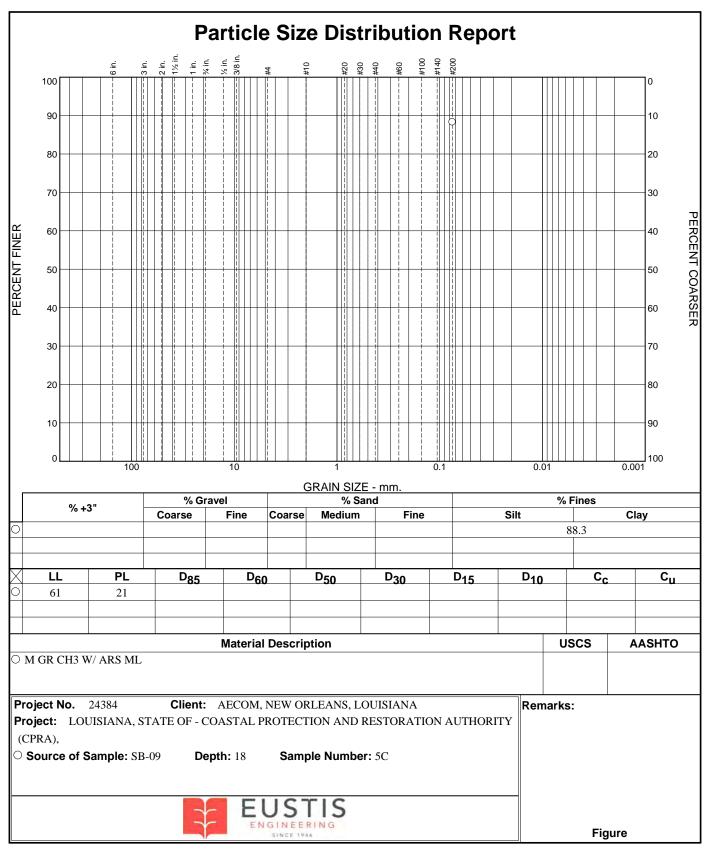


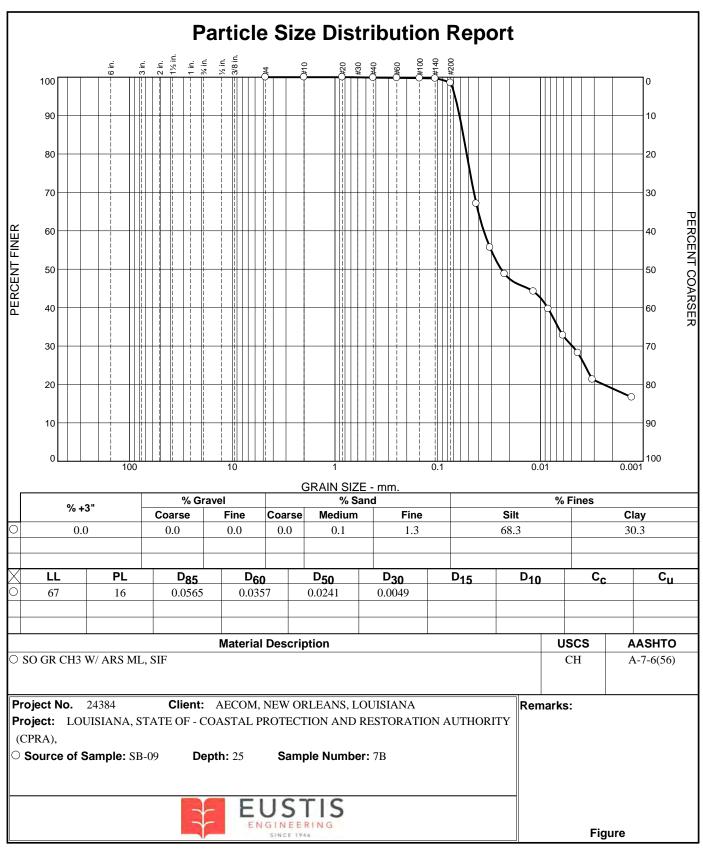


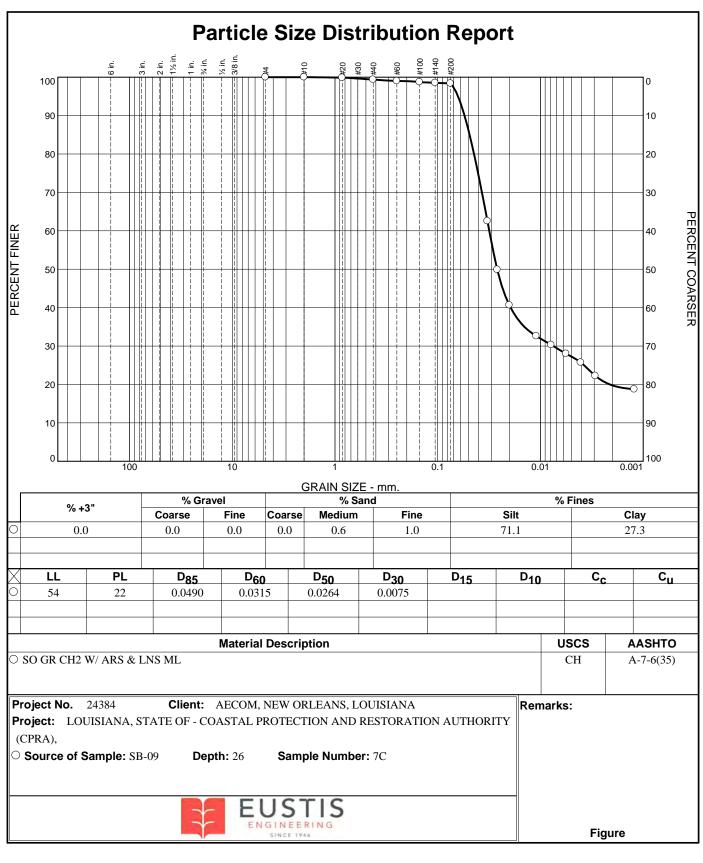


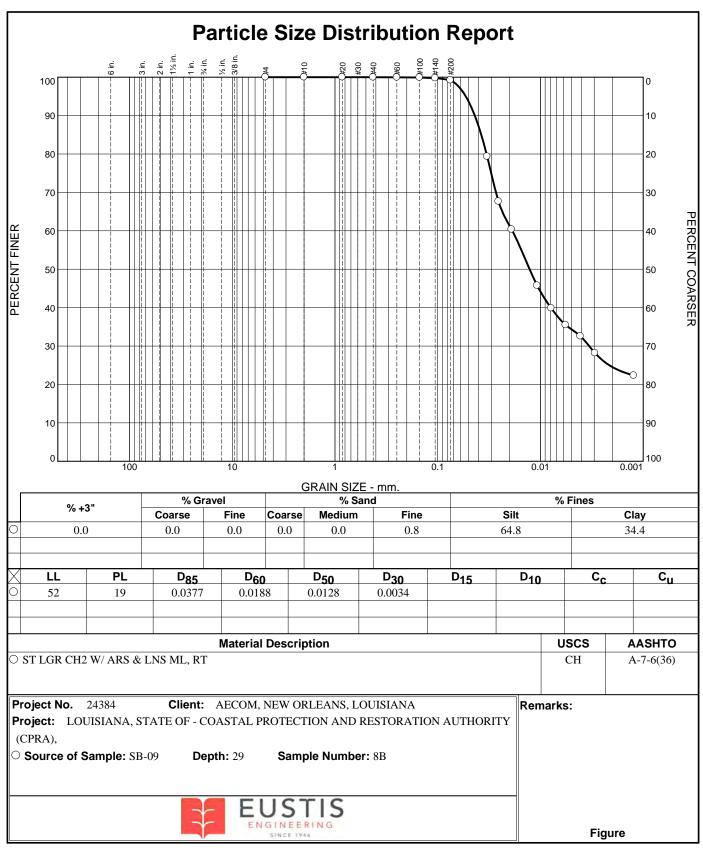


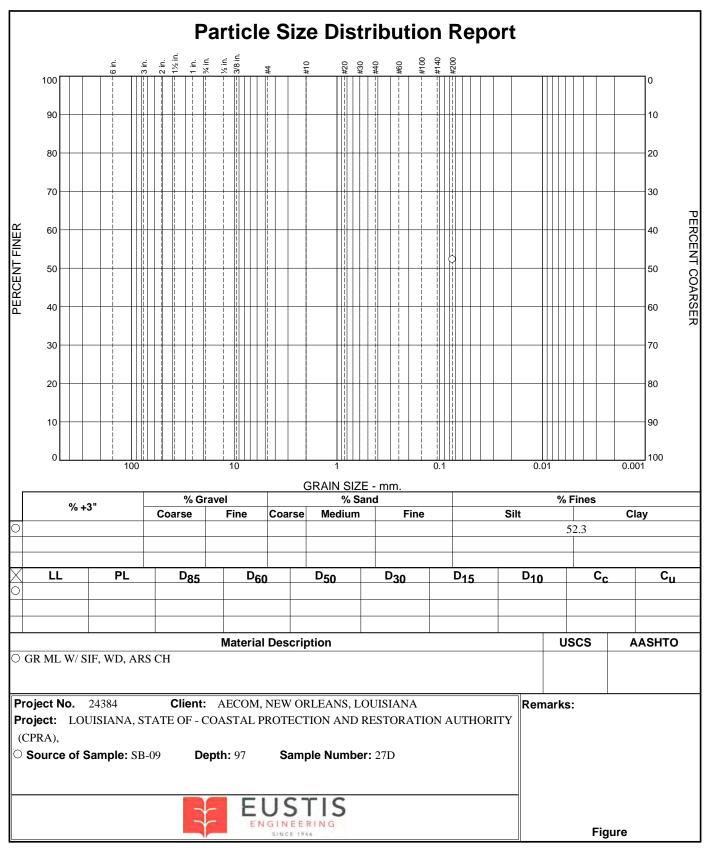


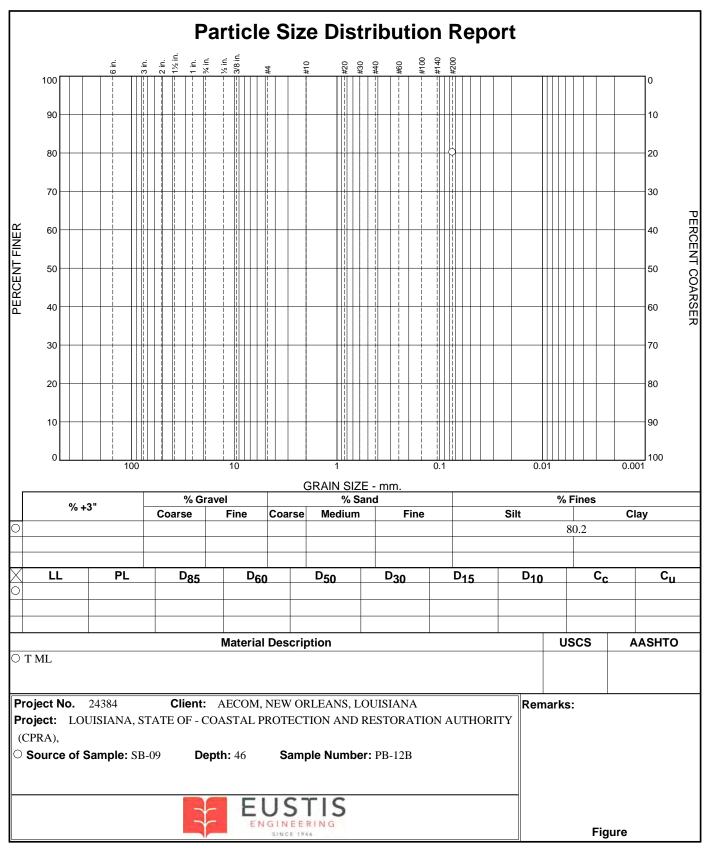


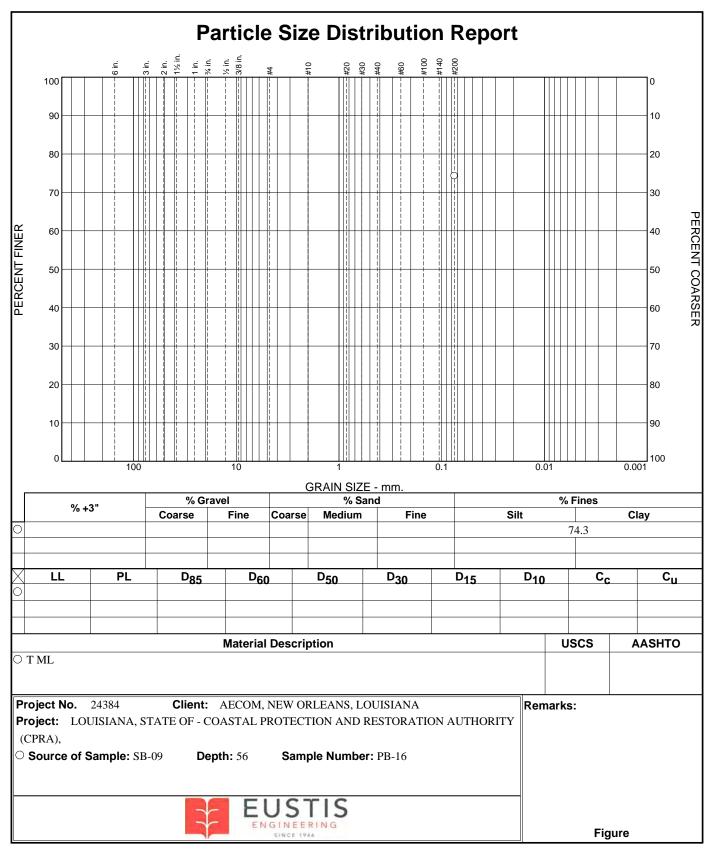


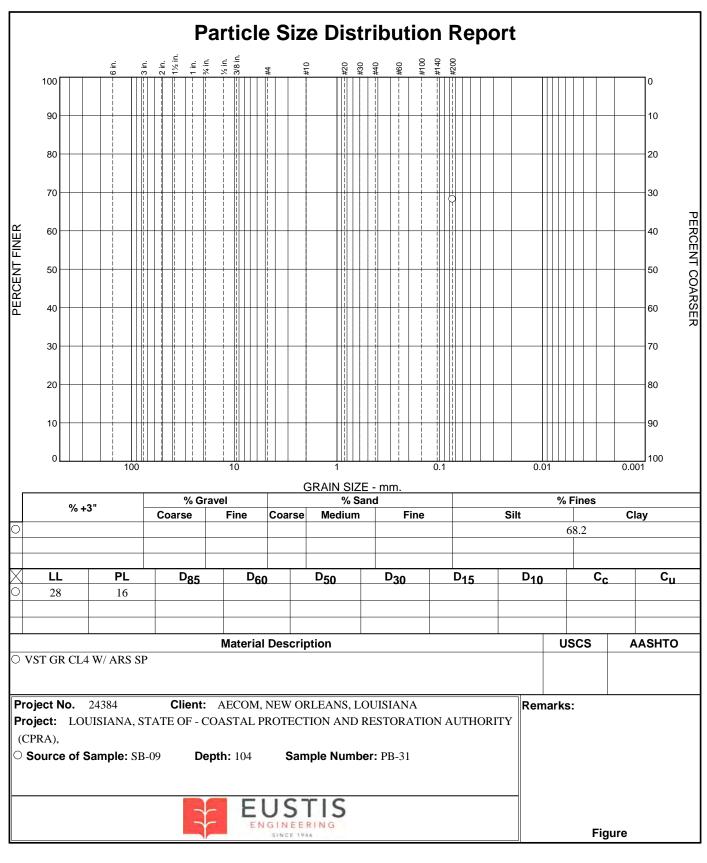


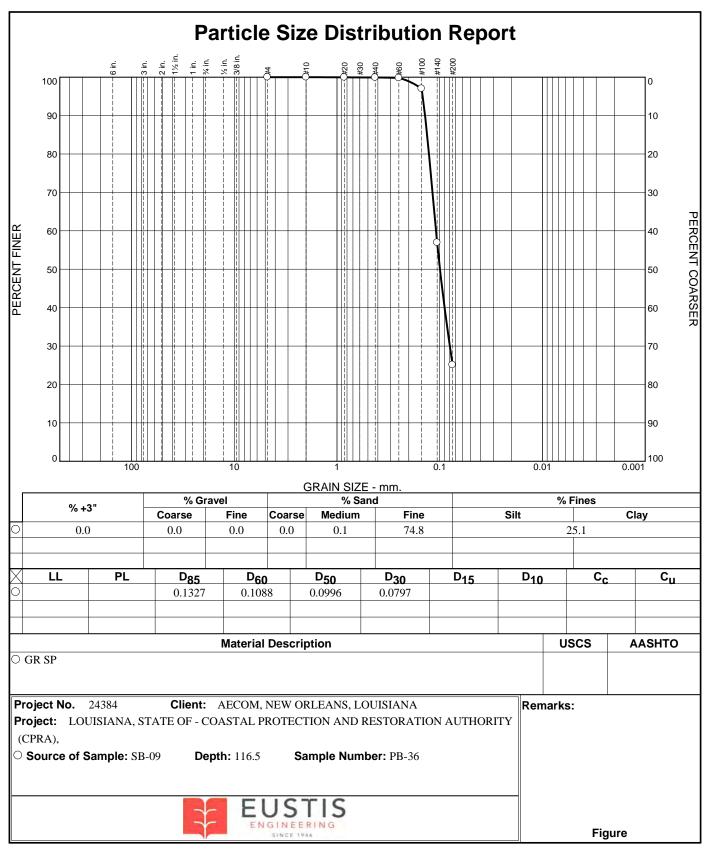


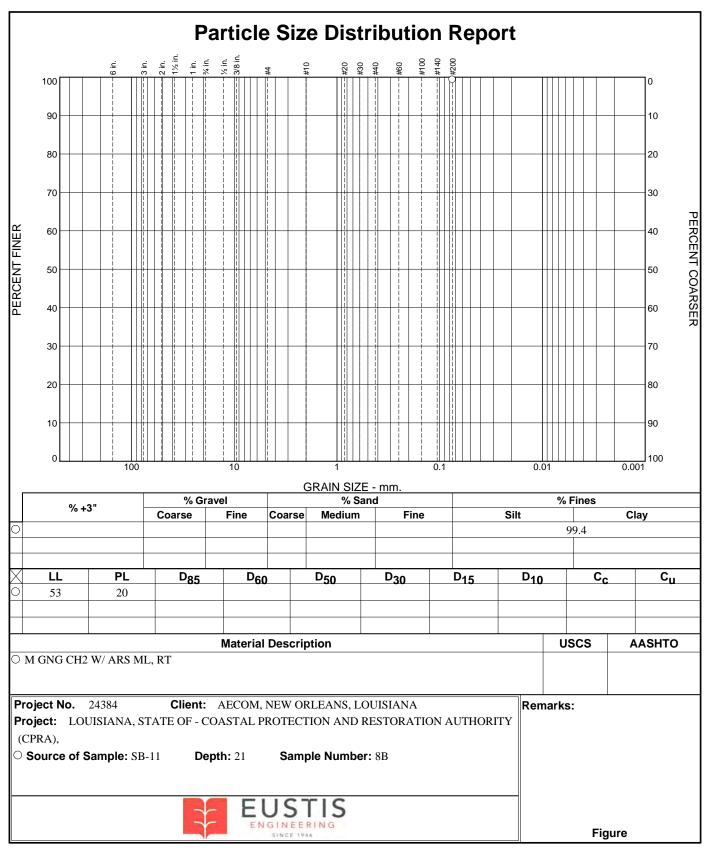


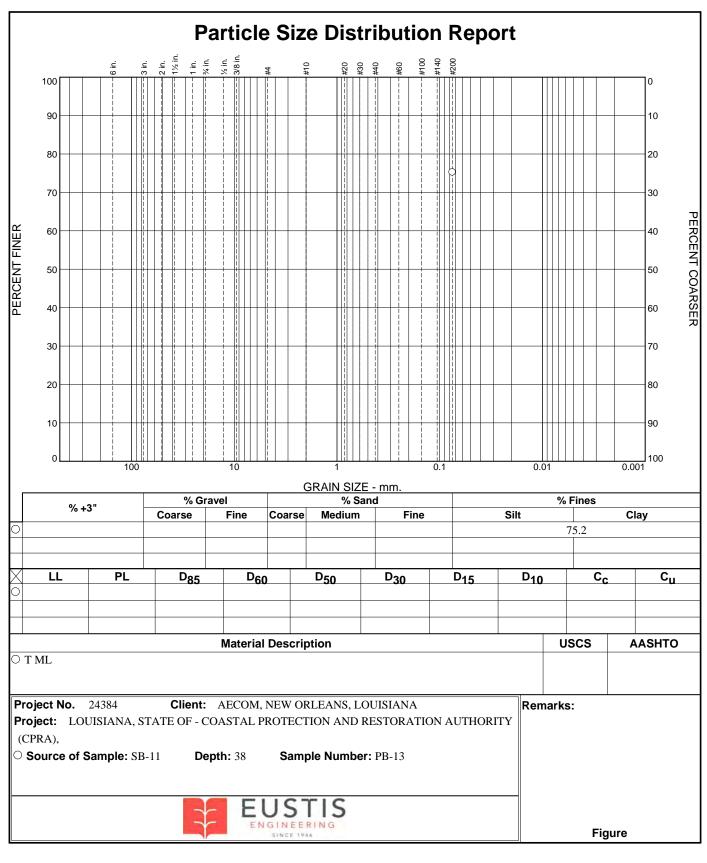


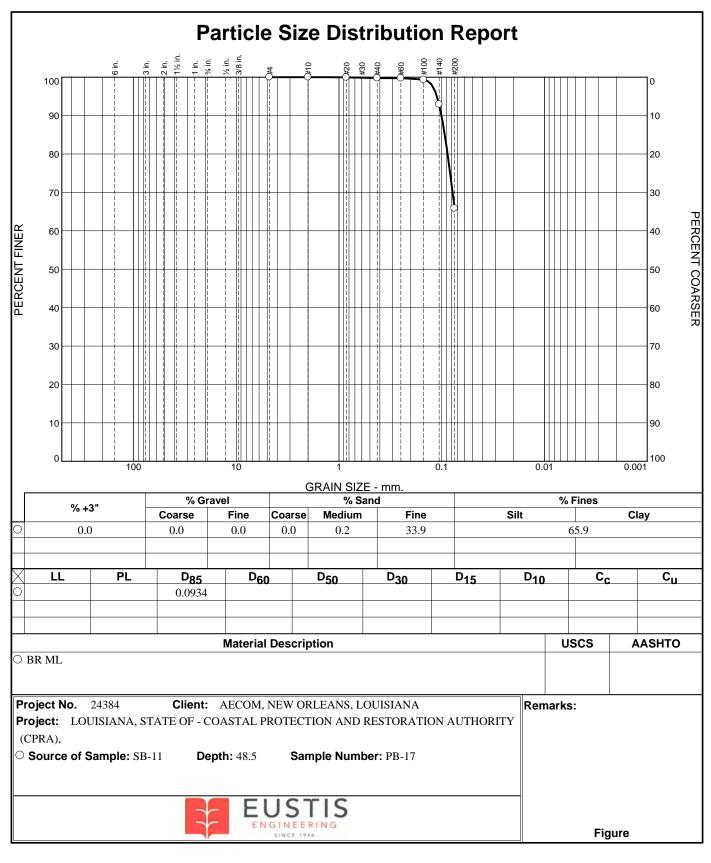


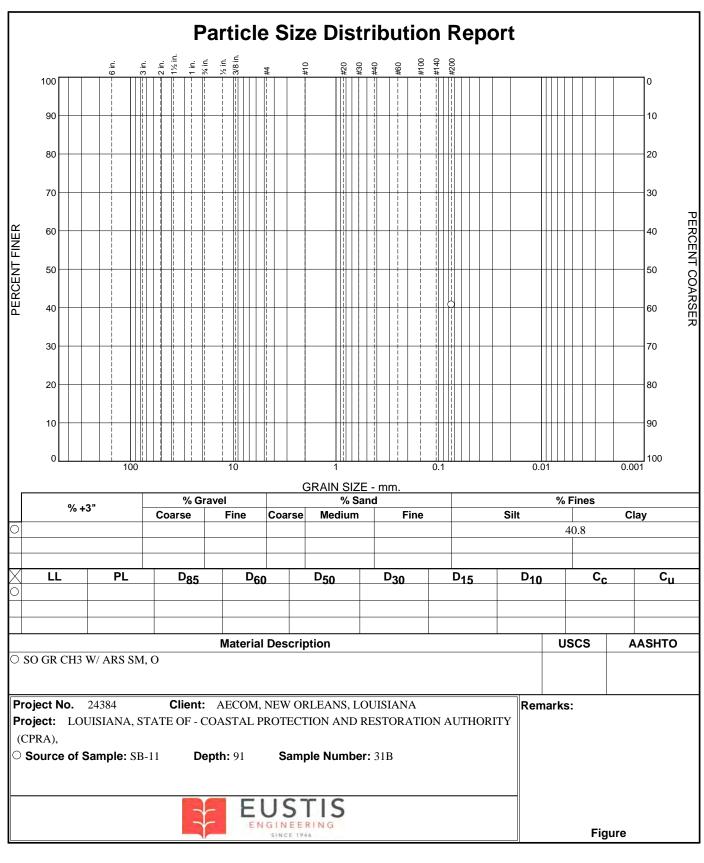


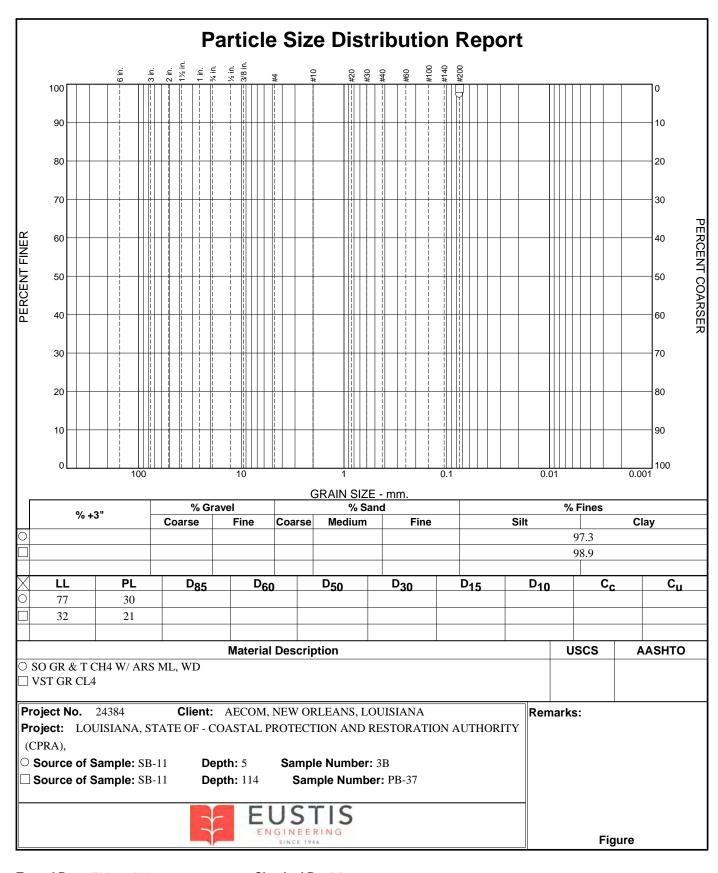




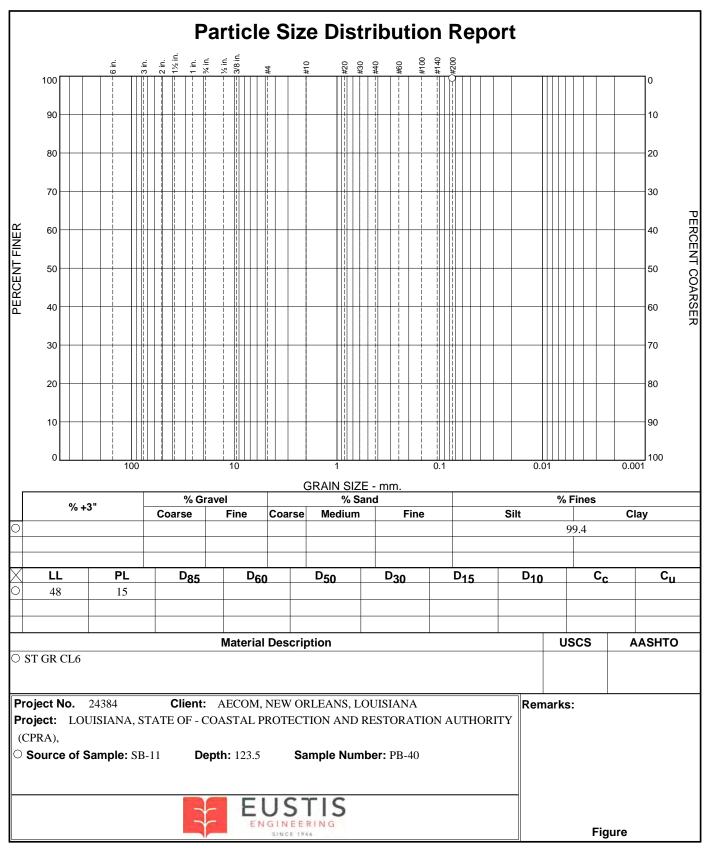


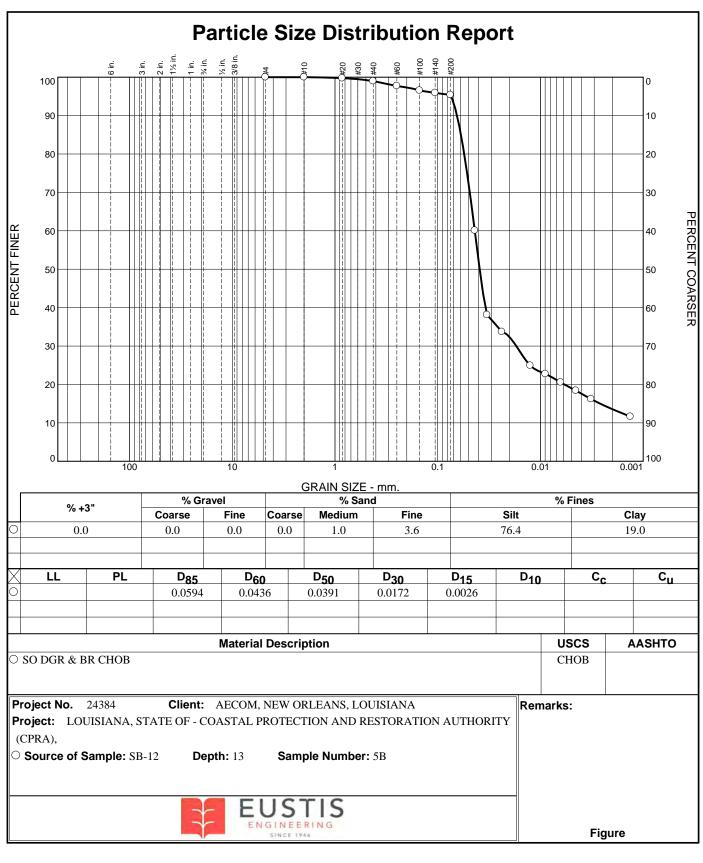


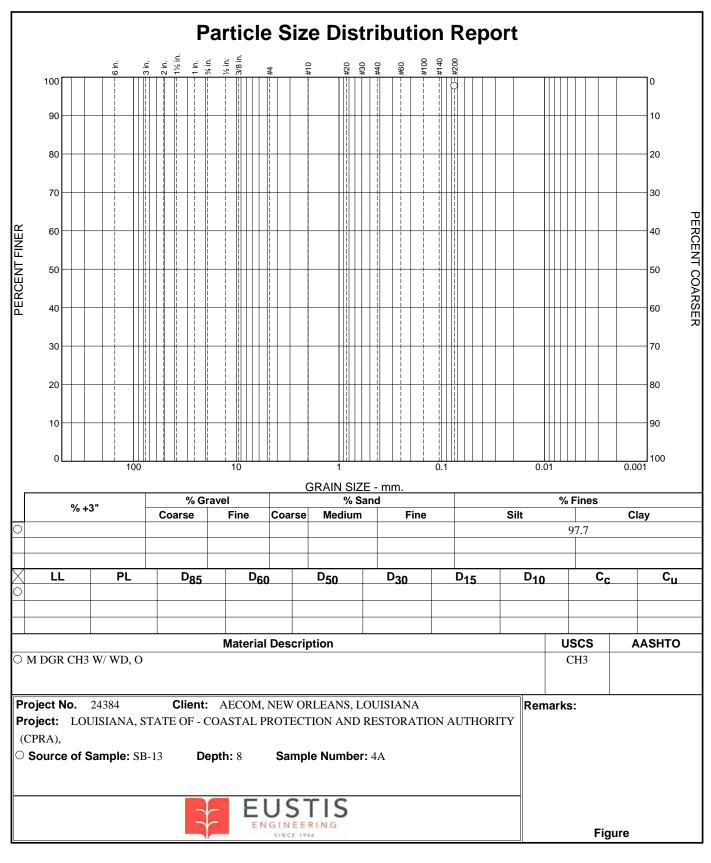


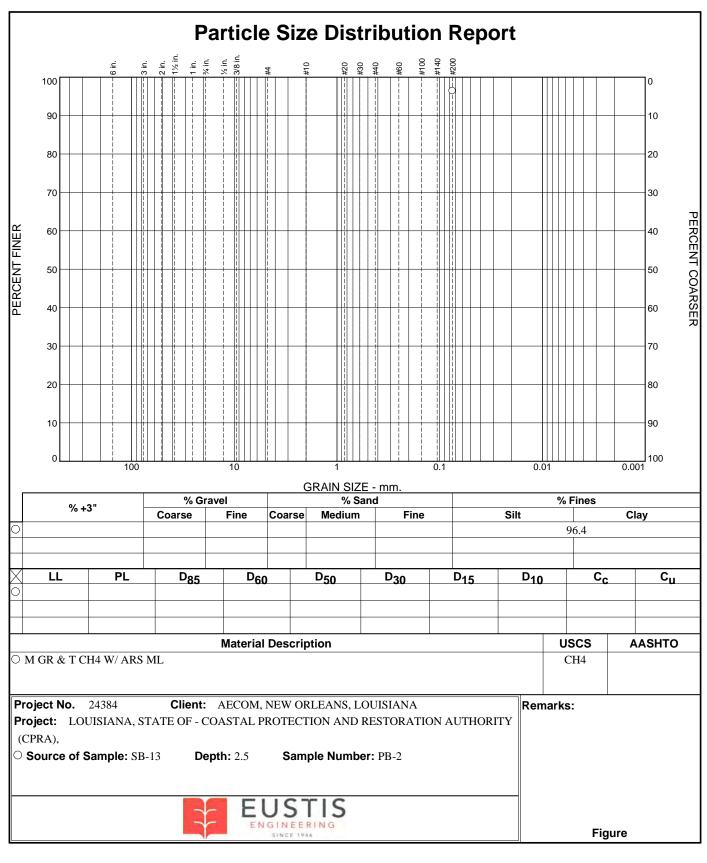


Tested By: ○ EM □ BH Checked By: RR









## APPENDIX VII

SUMMARY OF CONSOLIDATION DATA RESULTS (SDR-A, SDR-B, SDR-C)

EE 24384 - Maurepas Consolidation Test Data Summary

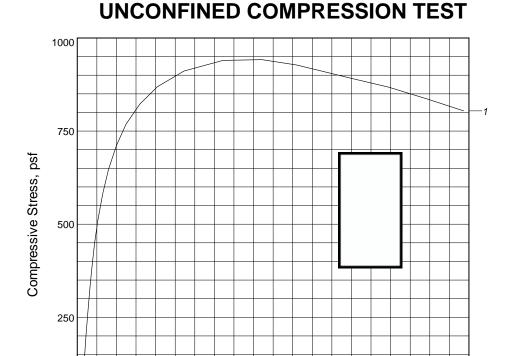
Soil Reach No.	Boring No.	Top of Boring El. (NAVD 88)	Sample El. (NAVD 88)	USACE Soil Class.	Liquid Limit	Plasticity Index	Moisture Content (%)	Initial Wet Density (pcf)	Compression Index Cc	Recompression Index Cr	Initial Void Ratio eo	Compression Ratio Cce	Recompression Ratio Cre	In Situ Effective Vertical Stress Po (tsf)	Preconsolidation Pressure Pc (tsf)	Over Consolidation Ratio OCR	Shansep Su (psf) $S_u = 0.22 \frac{P_c^{0.80}}{P_o}$
Reach C	B-05A	5.6	-6.4	СН	106	68	51.6	107.3	0.498	0.049	1.423	0.206	0.020	0.29	2.00	6.92	598
Reach B	B-6	3.2	-7.8	СН	179	127	126.6	83.6	1.542	0.193	3.652	0.331	0.041	0.25	0.70	2.82	250
Reach A	B-07A	3.7	-7.3	СН	175	134	121.9	83.4	1.658	0.221	3.560	0.364	0.048	0.14	0.60	4.36	197
Reach A	B-9	2.5	-9.5	СН	185	118	194.4	77.7	1.931	0.209	4.677	0.340	0.037	0.15	1.10	7.33	325
Reach A	B-10	-1.3	-11.3	СН	109	73	82.9	94.6	0.886	0.125	2.320	0.267	0.038	0.13	0.80	6.40	243
Reach A	B-11	-2.7	-14.7	СН	58	39	45.2	110.1	0.484	0.319	1.264	0.214	0.141	0.15	0.70	4.67	226
Reach A	B-12	-3.0	-11.0	ОН	259	184	146.0	80.0	0.997	0.216	3.607	0.216	0.047	0.10	0.58	5.80	180
Reach A	B-14	-0.5	-14.5	CL	44	23	28.4	120.4	0.260	0.038	0.831	0.142	0.021	0.18	1.22	6.97	364
Reach A	B-15	-0.4	-12.4	СН	55	38	40.7	116.5	0.328	0.039	1.074	0.158	0.019	0.15	0.85	5.67	264
Reach A	B-17	0.3	-9.7	ОН	285	188	203.5	74.1	2.000	0.400	5.151	0.325	0.065	0.13	0.50	4.00	167
Reach A	B-19	0.0	-12.0	СН	58	16	31.2	119.9	0.246	0.031	0.879	0.131	0.016	0.15	0.90	6.00	277
Reach B	KCS-01	2.6	-20.4	СН	70	44	51.3	105.2	0.346	0.118	1.514	0.138	0.047	0.51	0.38	0.74	177
Reach C	SB-01	7.4	-17.7	СНОВ	163	114	117.8	85.2	1.790	0.384	3.056	0.441	0.095	0.57	1.40	2.45	515
Reach C	SB-02	5.5	-1.5	CH3	73	52	21.4	129.3	0.277	0.091	0.576	0.176	0.058	0.17	1.50	8.75	428
Reach C	SB-02	5.5	-22.5	ML	NP	NP	31.2	119.0	0.138	0.022	0.831	0.075	0.012	0.64	1.50	2.36	556
Reach C	SB-03	5.7	-25.3	CH2	50	32	61.5	100.6	0.856	0.230	1.676	0.320	0.086	0.70	0.90	1.29	377
Reach C	SB-04	11.8	-14.2	СНОС	195	133	141.0	81.2	2.400	0.443	3.791	0.501	0.092	0.59	1.20	2.03	458
Reach B	SB-05	4.4	-14.6	СНОА	137	100	80.3	93.0	1.045	0.300	2.111	0.336	0.096	0.42	1.50	3.54	513
Reach B	SB-06	4.0	-10.0	СНОА	132	100	43.3	107.9	0.741	0.187	1.148	0.345	0.087	0.31	1.50	4.78	483
Reach B	SB-07	3.1	-10.9	СНОА	150	98	110.0	87.2	1.495	0.255	2.883	0.385	0.066	0.31	2.80	8.92	795

## EE 24384 - Maurepas Consolidation Test Data Summary

Soil Reach No.	Boring No.	Top of Boring El. (NAVD 88)	Sample El. (NAVD 88)	USACE Soil Class.	Liquid Limit	Plasticity Index	Moisture Content (%)	Initial Wet Density (pcf)	Compression Index Cc	Recompression Index Cr	Initial Void Ratio eo	Compression Ratio Cce	Recompression Ratio Cre	In Situ Effective Vertical Stress Po (tsf)	Preconsolidation Pressure Pc (tsf)	Over Consolidation Ratio OCR	Shansep Su (psf) $S_u = 0.22 \frac{P_c}{P_o}^{0.80}$	
Reach B	SB-08	3.1	-6.9	СНОА	144	99	93.7	88.9	1.214	0.257	2.495	0.347	0.074	0.23	1.50	6.64	452	į

## APPENDIX VIII

COMPILATION OF SHEAR DATA REPORTS (SDR-A, SDR-B, SDR-C)



Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	941.9	
Undrained shear strength, psf	470.9	
Failure strain, %	9.4	
Strain rate, %/min.	1.00	
Water content, %	49.9	
Wet density, pcf	108.0	
Dry density, pcf	72.0	
Saturation, %	100.0	
Void ratio	1.3580	
Specimen diameter, in.	1.395	
Specimen height, in.	2.776	
Height/diameter ratio	1.99	

**Description:** SO GR & T CH3 W/ ARS ML, RT

**LL =** 75 PL = 25**PI** = 50 Assumed GS= 2.72 **Type:** UNDISTURBED

Project No.: 24384 **Date Sampled:** 9/10/20

Remarks:

TORVANE = 0.375 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

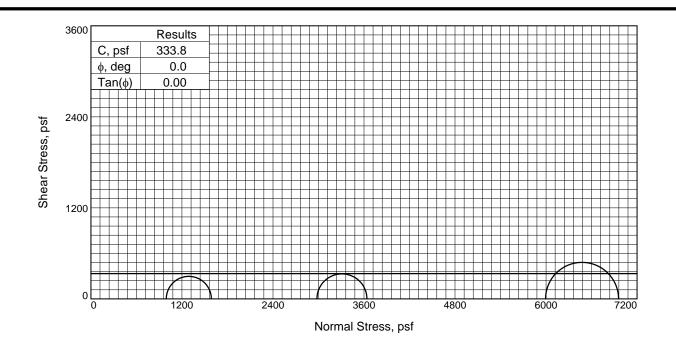
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

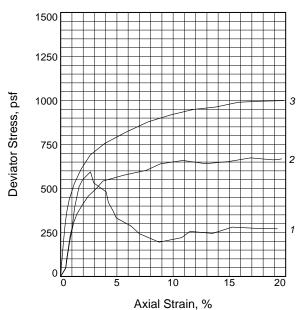
RESTORATION AUTHORITY (CPRA), **Source of Sample: SB-01** 

Sample Number: 2B

Figure ASTM D2166

Checked By: RR Tested By: DE





Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR & T CH2 W/ ARS ML, CC, SL

**LL=** 58 **PL=** 23 **PI=** 35

**Assumed Specific Gravity=** 2.7 **Remarks:** TORVANE =

= 0.375 TSF	Sample Number: 3B	
	<b>Proj. No.:</b> 24384	Date Sampled: 9/

Figure ASTM D2850

	Spe	ecimen No.	1	2	3	
		Water Content, %	35.1 86.7	35.8 83.9	34.7 87.1	
	ial	Dry Density, pcf Saturation, %	100.2	95.8	100.4	
	Initial	Void Ratio	0.9450	1.0093	0.9342	
3		Diameter, in.	1.405	1.385	1.387	
		Height, in.	2.741	2.754	2.761	
		Water Content, %	35.1	35.8	34.7	
	st	Dry Density, pcf	86.7	83.9	87.1	
	At Test	Saturation, %	100.2	95.8	100.4	
	- - -	Void Ratio	0.9450	1.0093	0.9342	
	`	Diameter, in.	1.405	1.385	1.387	
		Height, in.	2.741	2.754	2.761	
,	Stra	ain rate, %/min.	0.50	0.50	0.50	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.900	20.690	41.620	
	Fai	I. Stress, psf	594.1	659.4	962.8	
	Ult.	Stress, psf	244.8	641.1	962.8	
	σ <sub>1</sub>	Failure, psf	1587.7	3638.7	6956.1	
	$\sigma_3$	Failure, psf	993.6	2979.4	5993.3	

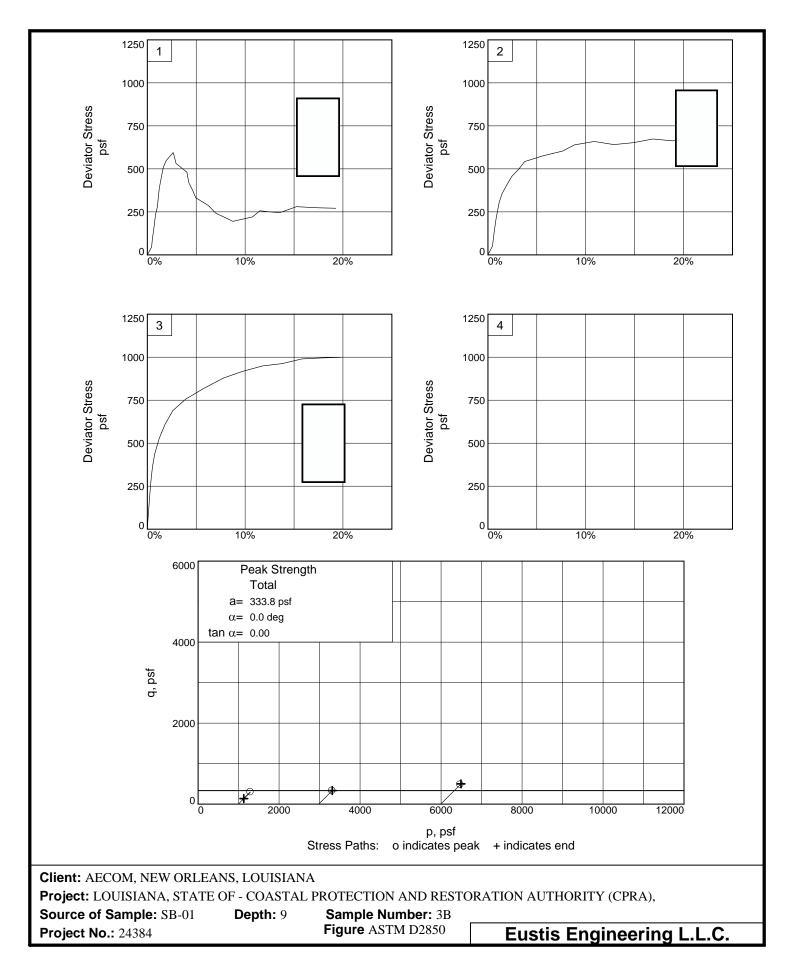
Client: AECOM, NEW ORLEANS, LOUISIANA

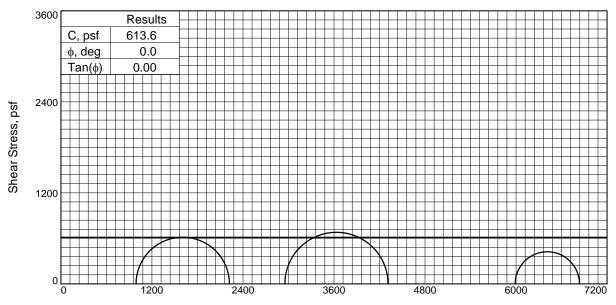
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-01 Depth: 9

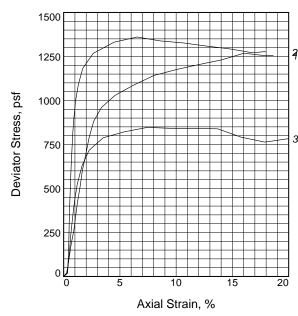
9/10/20







Normal Stress, psf



Type	Ωf	To	et.
IVDE	OI.	16	Sι.

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: M GR & T CH4 W/ ARS ML, CC, SL

**LL=** 98 **PL=** 29 **PI=** 69

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.375 TSF

	Spe	ecimen No.	1	2	3	
2		Water Content, % Dry Density, pcf	43.8 75.4	44.1 73.7	45.1 72.9	
	Initial	Saturation, %	95.1	92.0	92.3	
	Ξ	Void Ratio	1.2521	1.3047	1.3294	
		Diameter, in.	1.394	1.392	1.395	
		Height, in.	2.812	2.752	2.733	
3		Water Content, %	46.0	48.0	48.9	
,	, t	Dry Density, pcf	75.4	73.7	72.9	
	At Test	Saturation, %	100.0	100.0	100.0	
	<u>'</u>	Void Ratio	1.2521	1.3047	1.3294	
	`	Diameter, in.	1.394	1.392	1.395	
		Height, in.	2.812	2.752	2.733	
	Stra	ain rate, %/min.	0.30	0.30	0.30	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	ll Pressure, psi	6.880	20.500	41.590	
	Fai	I. Stress, psf	1230.4	1360.8	847.2	
	5	Strain, %	14.0	6.5	7.4	
	Ult.	Stress, psf	1230.4	1293.2	838.7	
	5	Strain, %	14.0	14.8	13.7	
_	σ <sub>1</sub>	Failure, psf	2221.2	4312.8	6836.2	
	$\sigma_3$	Failure, psf	990.7	2952.0	5989.0	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

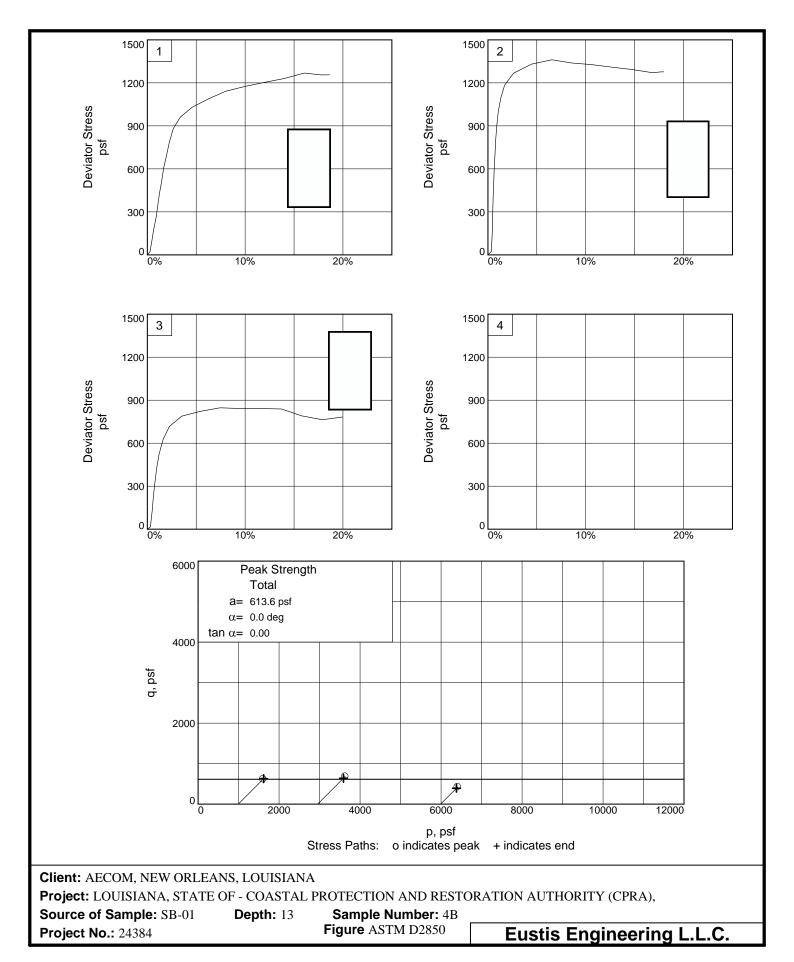
Source of Sample: SB-01 Depth: 13

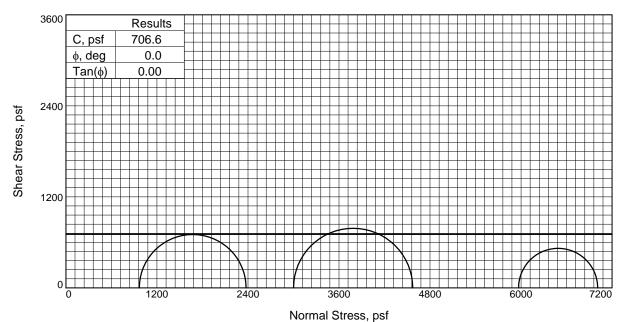
Sample Number: 4B

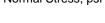
**Proj. No.:** 24384 **Date Sampled:** 9/10/20



Figure ASTM D2850

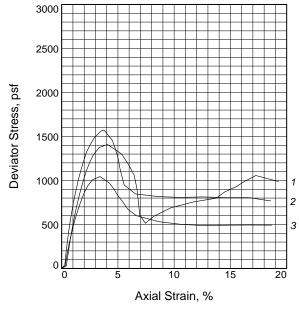






Water Content, %

Specimen No.



Туре	of	Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: M GR & BR CHOB

**LL=** 172 **PL=** 54 **PI=** 118

**Assumed Specific Gravity=** 2.65 **Remarks:** TORVANE = 0.250 TSF

		Dry Density, pcf	36.6	38.8	36.9	
	Initial	Saturation, %	96.8	99.8	96.5	
	<u>=</u>	Void Ratio	3.5236	3.2633	3.4870	
		Diameter, in.	1.414	1.392	1.420	
		Height, in.	2.755	2.765	2.753	
		Water Content, %	133.0	123.1	131.6	
	#	Dry Density, pcf	36.6	38.8	36.9	
	ĕ	Saturation, %	100.0	100.0	100.0	
	At Test	Void Ratio	3.5236	3.2633	3.4870	
	1	Diameter, in.	1.414	1.392	1.420	
		Height, in.	2.755	2.765	2.753	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
1	Bac	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.720	20.840	41.440	
	Fail	. Stress, psf	1407.5	1570.9	1042.7	
	S	Strain, %	4.0	3.7	3.4	
	Ult.	Stress, psf	514.8	805.7	487.3	
	S	Strain, %	7.5	14.6	12.7	
	$\sigma_1$	Failure, psf	2375.2	4571.8	7010.1	
	$\sigma_3$	Failure, psf	967.7	3001.0	5967.4	

1

128.7

2

122.9

3

126.9

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

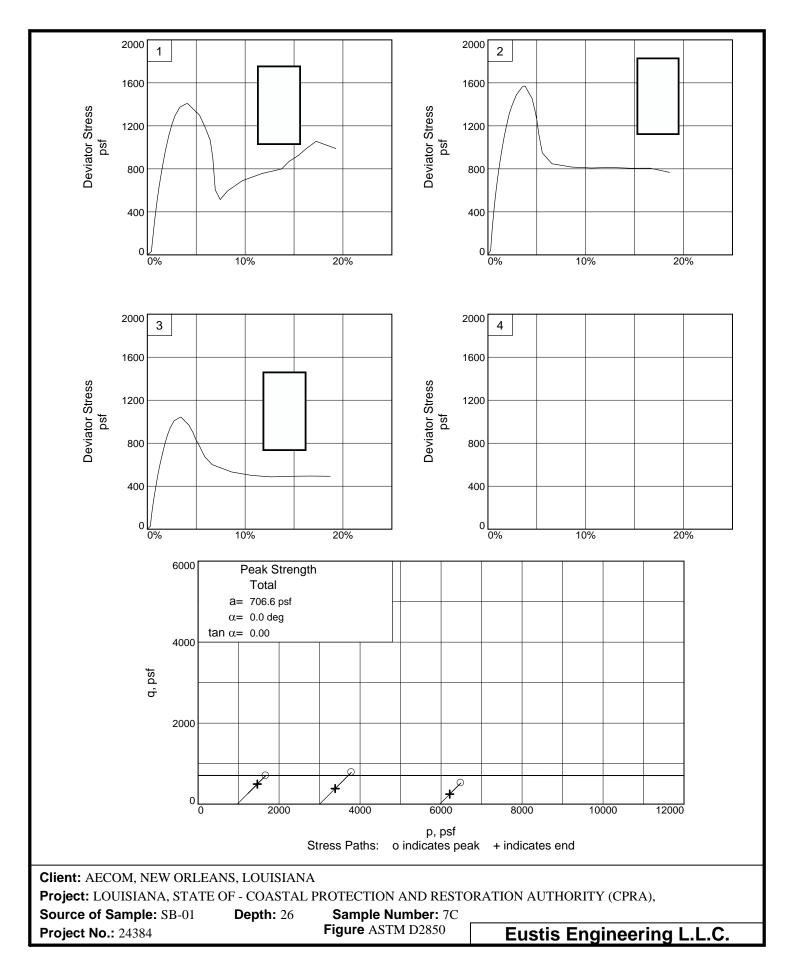
Source of Sample: SB-01 Depth: 26

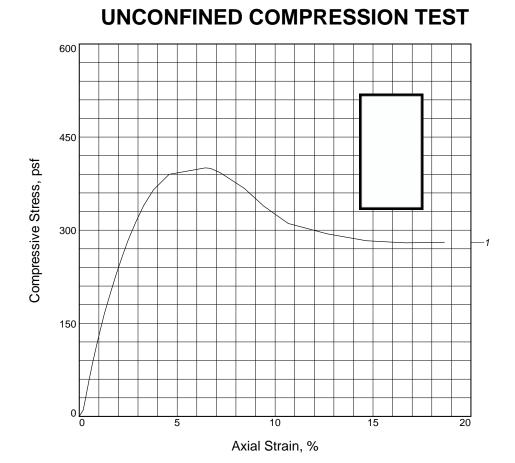
**Sample Number: 7**C

**Proj. No.:** 24384 **Date Sampled:** 9/10/20



Figure ASTM D2850





Specimen No.	1	
Unconfined strength, psf	400.5	
Undrained shear strength, psf	200.2	
Failure strain, %	6.4	
Strain rate, %/min.	1.00	
Water content, %	69.1	
Wet density, pcf	99.6	
Dry density, pcf	58.9	
Saturation, %	99.8	
Void ratio	1.8825	
Specimen diameter, in.	1.413	
Specimen height, in.	2.719	
Height/diameter ratio	1.92	

Description: SO GR CH4 W/ ARS & LNS SM, O, SIF, SL

**LL** = 78 **PL** = 30 **PI** = 48 **Assumed GS**= 2.72 **Type:** UNDISTURBED

Project No.: 24384

Date Sampled: 9/10/20

Remarks:

TORVANE = 0.110 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

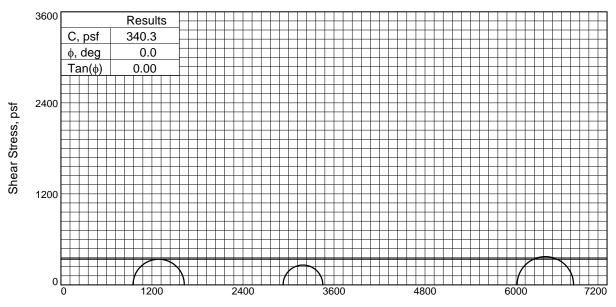
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-01 **Depth:** 30

Sample Number: 8C

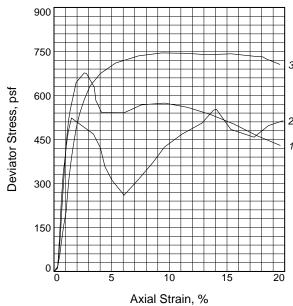
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Figure ASTM D2166



Normal Stress, psf

Specimen No.



	Spe	ecimen No.	ı	2	3	
		Water Content, %	69.6	69.1	67.3	
		Dry Density, pcf	57.7	57.0	57.8	
3	<u>iā</u>	Saturation, %	97.5	94.8	94.4	
	Initia	Void Ratio	1.9423	1.9813	1.9381	
		Diameter, in.	1.405	1.419	1.383	
		Height, in.	2.753	2.765	2.761	
2		Water Content, %	71.4	72.8	71.3	
1	+;	Dry Density, pcf	57.7	57.0	57.8	
	မြိ	Saturation, %	100.0	100.0	100.0	
	At Test	Void Ratio	1.9423	1.9813	1.9381	
		Diameter, in.	1.405	1.419	1.383	
		Height, in.	2.753	2.765	2.761	
	Stra	ain rate, %/min.	0.50	0.50	0.50	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.600	20.340	41.760	
	Fai	I. Stress, psf	676.8	523.1	745.1	
	Ult. Stress, psf		534.8	260.1	738.8	
	σ <sub>1</sub> Failure, psf		1627.2	3452.1	6758.5	
	$\sigma_3$ Failure, psf		950.4	2929.0	6013.4	

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR CH3 W/ ARS ML, SIF

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.150 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

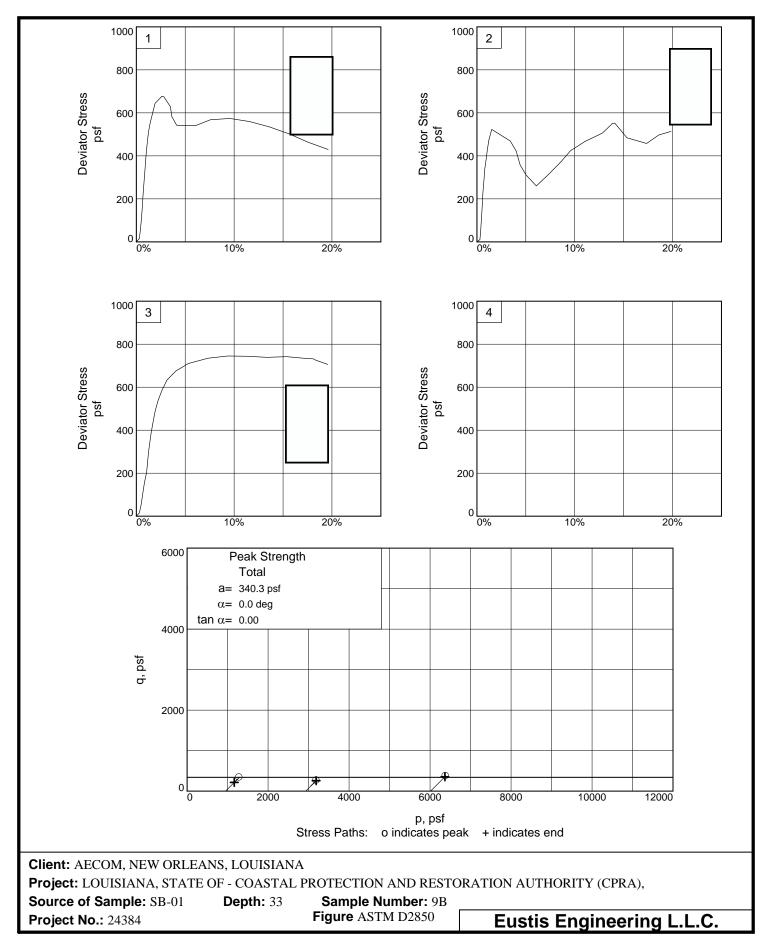
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-01 **Depth:** 33

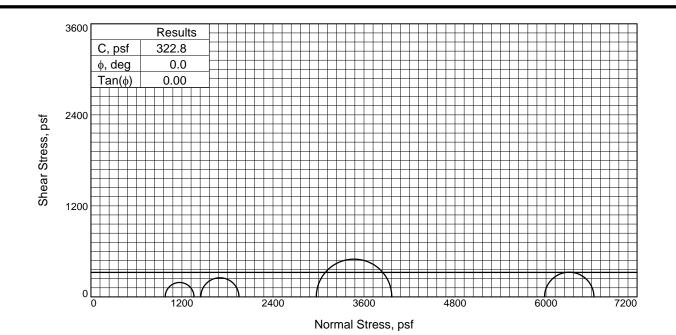
Sample Number: 9B

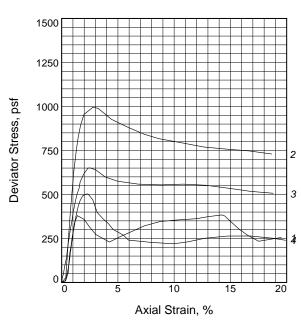
**Proj. No.:** 24384 **Date Sampled:** 9/21/20



Figure ASTM D2850







Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR CH4 W/ ARS ML, SIF, SL

**LL=** 91 **PL=** 22 **PI=** 69

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.200 TSF

Spe	ecimen No.	1	2	3	4
	Water Content, %	72.3	72.0	71.9	72.2
	Dry Density, pcf	57.2	57.4	57.3	56.2
nitial	Saturation, %	99.9	99.9	99.7	97.2
<u>=</u>	Void Ratio	1.9676	1.9585	1.9616	2.0199
	Diameter, in.	1.399	1.395	1.426	1.405
	Height, in.	2.778	2.765	2.763	2.763
	Water Content, %	72.3	72.0	72.1	74.3
;;	Dry Density, pcf	57.2	57.4	57.3	56.2
At Test	Saturation, %	100.0	100.0	100.0	100.0
=	Void Ratio	1.9676	1.9585	1.9616	2.0199
~	Diameter, in.	1.399	1.395	1.426	1.405
	Height, in.	2.778	2.765	2.763	2.763
Stra	ain rate, %/min.	1.00	1.00	1.00	1.00
Bad	ck Pressure, psi	0.000	0.000	0.000	0.000
Cel	l Pressure, psi	6.800	20.630	41.530	10.060
Fai	l. Stress, psf	379.8	995.9	650.6	503.4
5	Strain, %	1.4	2.8	2.5	2.3
Ult.	Stress, psf	229.8	757.7	535.6	219.6
5	Strain, %	4.2	14.7	14.8	10.0
σ <sub>1</sub>	Failure, psf	1359.0	3966.6	6630.9	1952.0
$\sigma_3$	Failure, psf	979.2	2970.7	5980.3	1448.6
I —					

Client: AECOM, NEW ORLEANS, LOUISIANA

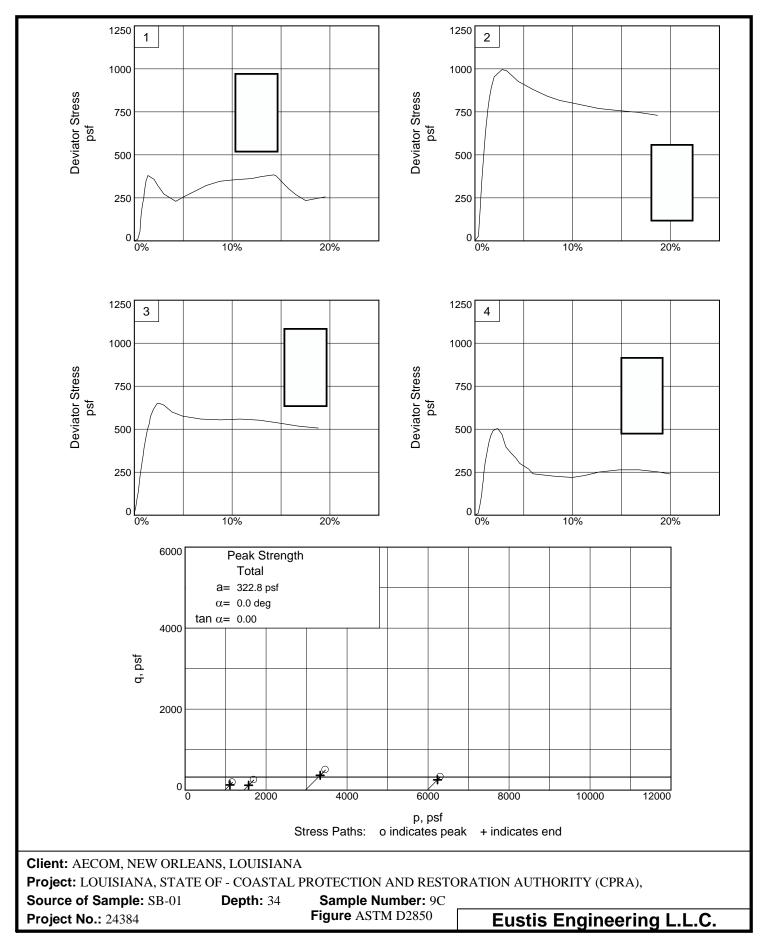
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

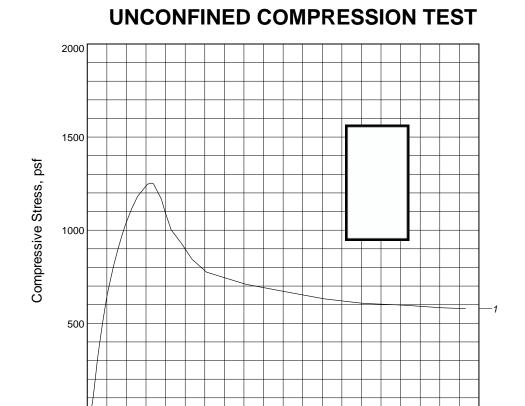
Sample Number: 9C

**Proj. No.:** 24384 **Date Sampled:** 9/10/20



Figure ASTM D2850





Axial Strain, %

Specimen No.	1		
Unconfined strength, psf	1252.7		
Undrained shear strength, psf	626.4		
Failure strain, %	3.3		
Strain rate, %/min.	1.00		
Water content, %	68.9		
Wet density, pcf	99.8		
Dry density, pcf	59.1		
Saturation, %	100.0		
Void ratio	1.8749		
Specimen diameter, in.	1.392		
Specimen height, in.	2.753		
Height/diameter ratio	1.98		

**Description:** M GR CH4 W/ ARS ML, SIF

LL = 95 PL = 34 PI = 61 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/10/20

Remarks:

TORVANE = 0.250 TSF

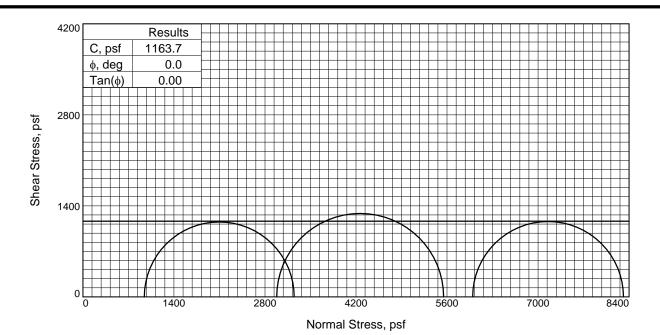
Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

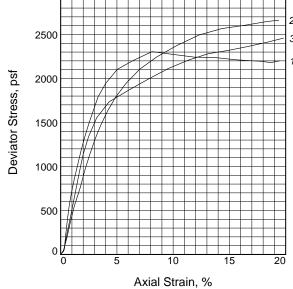
Sample Number: 10C

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Figure ASTM D2166



3000 Specimen No.



Type of Test:

Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: ST GNG CH2 W/ ARS ML

**LL=** 58 **PL=** 20 **PI=** 38

**Assumed Specific Gravity=** 2.70 **Remarks:** TORVANE = 0.500 TSF

Specimen No. 1 2	3
Water Content, % 25.4 25.2	26.3
Dry Density, pcf 98.4 99.8	95.7
Saturation, % 96.2 98.6	93.5
[편] Saturation, % 96.2 98.6 [표] Void Ratio 0.7126 0.6890 0	.7613
Diameter, in. 1.393 1.385	1.384
Height, in. 2.843 2.826	2.875
Water Content, % 26.4 25.5	28.2
Dry Density, pcf 98.4 99.8	95.7
Saturation, % 100.0 100.0 Void Ratio 0.7126 0.6890 0	100.0
$\downarrow$ Void Ratio 0.7126 0.6890 0	0.7613
Diameter, in. 1.393 1.385	1.384
Height, in. 2.843 2.826	2.875
Strain rate, %/min. 1.00 1.00	1.00
Back Pressure, psi 0.000 0.000	0.000
Cell Pressure, psi 6.560 20.700 4	1.630
Fail. Stress, psf 2304.6 2566.7 2	318.6
Strain, % 8.1 14.3	14.9
Ult. Stress, psf 2238.7 2566.7 2	318.6
Strain, % 13.6 14.3	14.9
$\sigma_1$ Failure, psf 3249.2 5547.5 8	313.3
$\sigma_3$ Failure, psf 944.6 2980.8 5	994.7

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

Sample Number: 11B



Figure ASTM D2850

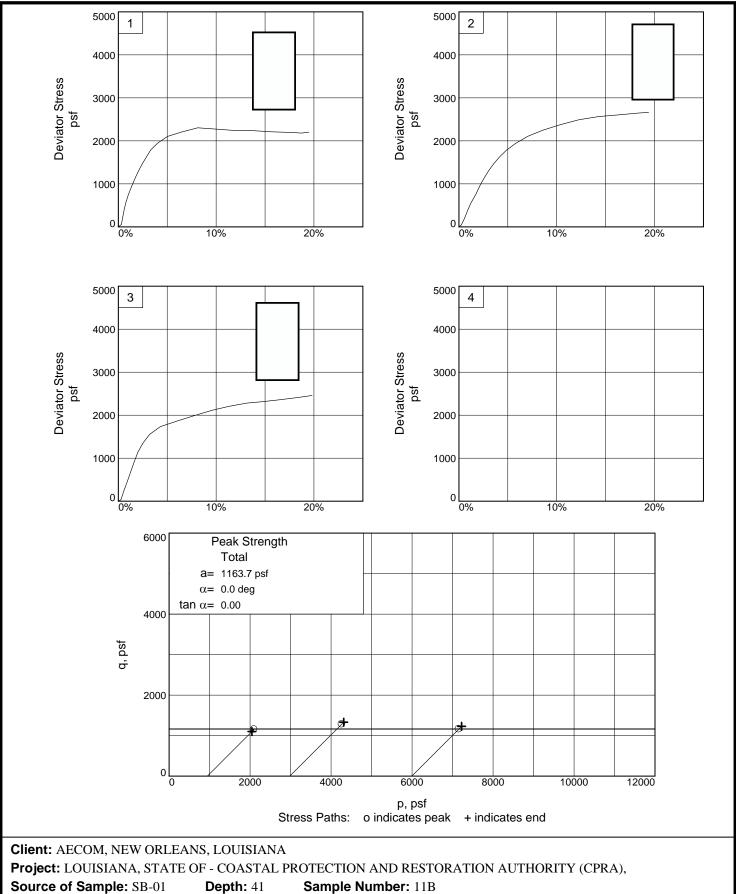
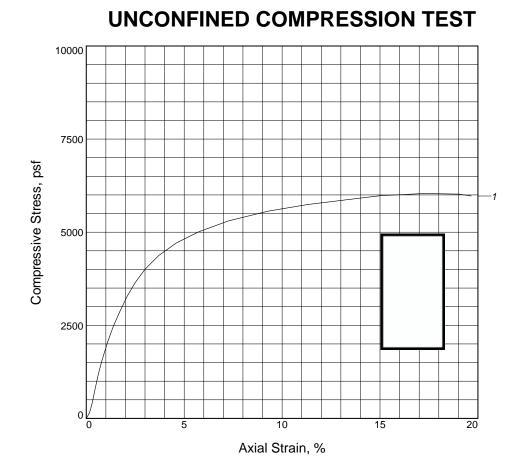


Figure ASTM D2850

**Eustis Engineering L.L.C.** Project No.: 24384



Specimen No.	1	
Unconfined strength, psf	5981.8	
Undrained shear strength, psf	2990.9	
Failure strain, %	15.0	
Strain rate, %/min.	1.00	
Water content, %	21.4	
Wet density, pcf	128.3	
Dry density, pcf	105.7	
Saturation, %	97.0	
Void ratio	0.5949	
Specimen diameter, in.	1.389	
Specimen height, in.	2.757	
Height/diameter ratio	1.98	

Description: VST GNG CH2 W/ ARS ML

LL = 53 PL = 16 PI = 37 Assumed GS= 2.70 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/10/20

Remarks:

TORVANE = 0.875 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

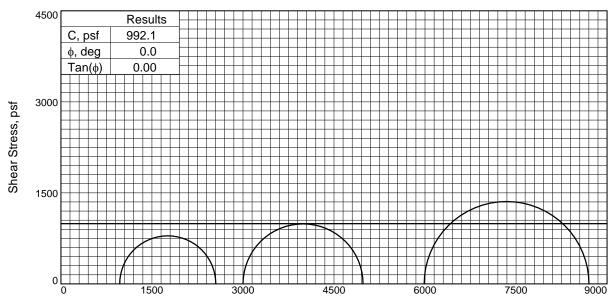
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-01 **Depth:** 46

Sample Number: 12C

EUSTIS ENGINEERING SINCE 1946

Figure ASTM D2166

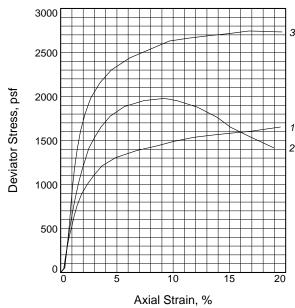


Normal Stress, psf

Water Content, %

Dry Density, pcf

Specimen No.



4		<u>.e</u>	Saturation, %	90.4	93.0	94.0
1		Initia	Void Ratio	0.9323	0.8485	0.8359
7			Diameter, in.	1.394	1.394	1.393
1			Height, in.	2.863	2.866	2.868
1	1		Water Content, %	34.3	31.2	30.7
1	2	±.	Dry Density, pcf	87.9	91.9	92.5
+	-	آو ا	Saturation, %	100.0	100.0	100.0
1		At Test	Void Ratio	0.9323	0.8485	0.8359
+		1	Diameter, in.	1.394	1.394	1.393
1			Height, in.	2.863	2.866	2.868
1		Stra	ain rate, %/min.	1.00	1.00	1.00
1		Bac	ck Pressure, psi	0.000	0.000	0.000
1		Cel	l Pressure, psi	6.730	20.840	41.570
1		Fail	I. Stress, psf	1582.8	1977.5	2714.8
)		S	Strain, %	15.1	9.2	14.9
		Ult.	Stress, psf	1582.8	1651.4	2714.8
		S	Strain, %	15.1	15.1	14.9

1

31.0

87.9

2

29.0

91.9

3

28.9

92.5 94.0

8700.9

5986.1

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: M GNG & T CH3 W/ ARS ML, SL

**PL=** 20 **PI=** 45 **LL=** 65

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 1.000 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

2552.0

969.1

4978.5

3001.0

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-01 Depth: 50

Sample Number: 13C

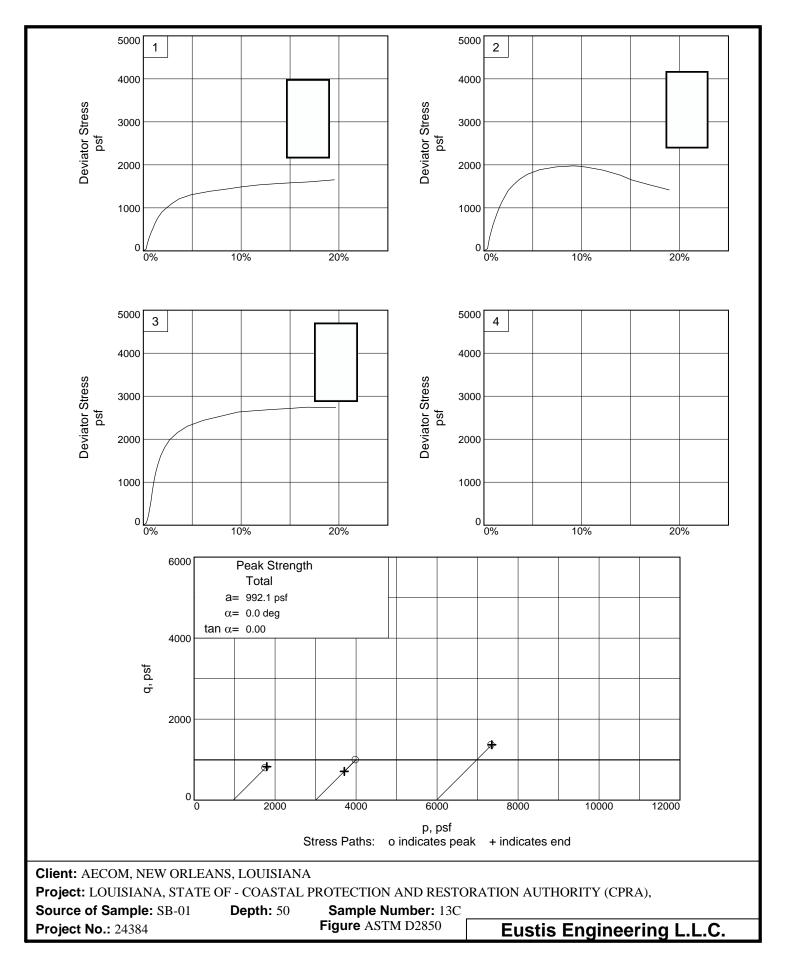
σ<sub>1</sub> Failure, psf

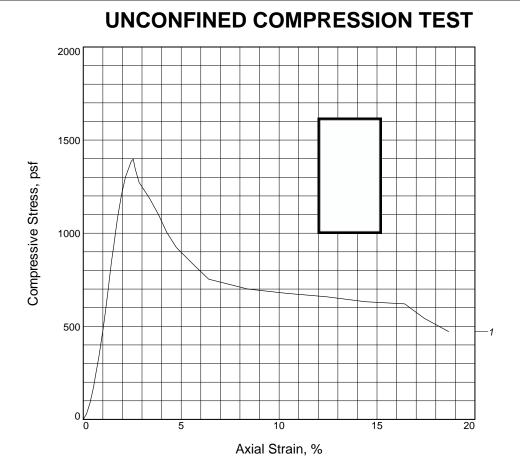
 $\sigma_3$  Failure, psf

**Date Sampled: 9/10/20** Proj. No.: 24384



Figure ASTM D2850





Specimen No.	1	
Unconfined strength, psf	1397.5	
Undrained shear strength, psf	698.8	
Failure strain, %	2.6	
Strain rate, %/min.	1.00	
Water content, %	30.0	
Wet density, pcf	121.0	
Dry density, pcf	93.0	
Saturation, %	99.9	
Void ratio	0.8119	
Specimen diameter, in.	1.383	
Specimen height, in.	2.746	
Height/diameter ratio	1.99	

**Description:** M T & GR CL6

**LL** = 47 **PL** = 19 **PI** = 28 **Assumed GS**= 2.70 **Type:** UNDISTURBED

Project No.: 24384

Date Sampled: 9/10/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

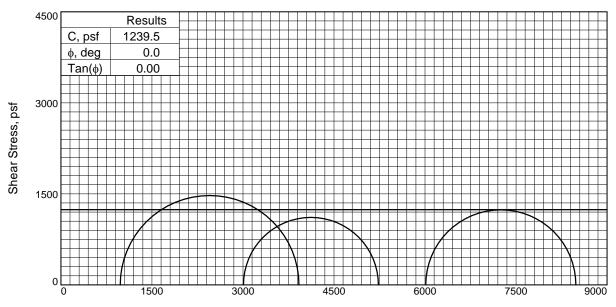
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-01 **Depth:** 54

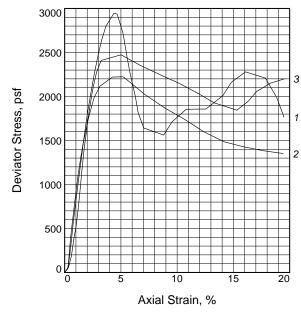
Sample Number: 14C

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Figure ASTM D2166



Normal Stress, psf



_		_	
Type	<b>∩</b> t	ΙД	ct.
IVDE	vı	16	ЭL.

Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: ST BR & GR CH3 W/ ARS & LNS ML,

SL

**LL=** 72 **PL=** 26 **PI=** 46

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 1.250 TSF

	Spe	ecimen No.	1	2	3	
		Water Content, %	37.6	36.9	37.4	
		Dry Density, pcf	82.5	84.4	84.1	
	Initial	Saturation, %	96.7	99.1	100.0	
3	' <u>=</u>	Void Ratio	1.0584	1.0118	1.0183	
		Diameter, in.	1.404	1.401	1.398	
1		Height, in.	2.868	2.864	2.851	
'		Water Content, %	38.9	37.2	37.4	
	;;	Dry Density, pcf	82.5	84.4	84.1	
2	At Test	Saturation, %	100.0	100.0	100.0	
		Void Ratio	1.0584	1.0118	1.0183	
	`	Diameter, in.	1.404	1.401	1.398	
		Height, in.	2.868	2.864	2.851	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.790	20.890	41.750	
	Fai	l. Stress, psf	2944.5	2223.5	2473.4	
	8	Strain, %	4.5	5.2	5.0	
	Ult.	Stress, psf	1561.2	1486.1	1843.8	
	5	Strain, %	8.8	14.2	15.3	
	σ <sub>1</sub>	Failure, psf	3922.2	5231.6	8485.4	
	$\sigma_3$	Failure, psf	977.8	3008.2	6012.0	

Client: AECOM, NEW ORLEANS, LOUISIANA

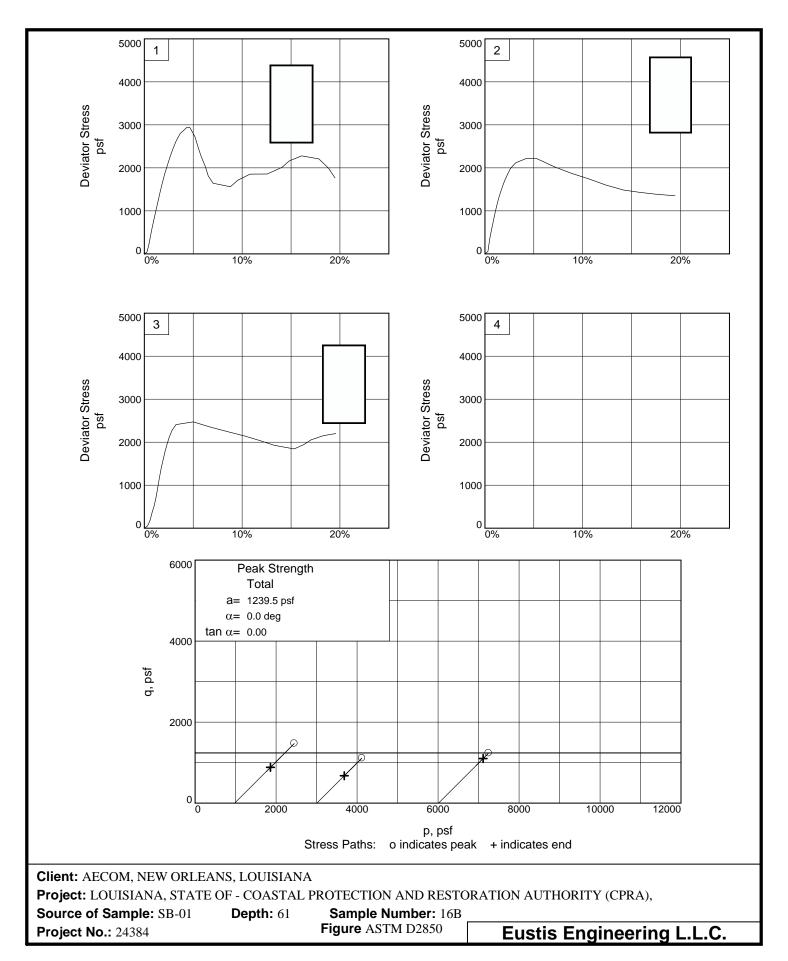
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

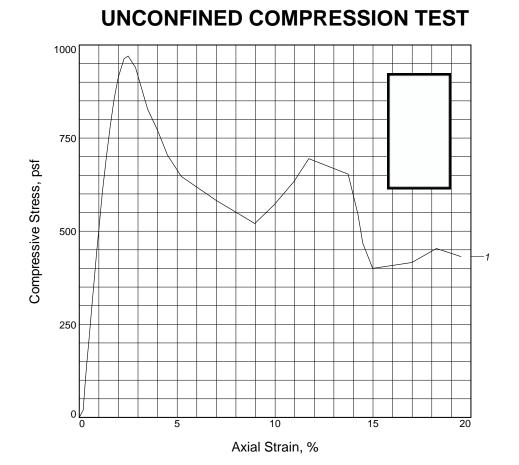
Sample Number: 16B

**Proj. No.:** 24384 **Date Sampled:** 9/10/20



Figure ASTM D2850





Specimen No.	1		
Unconfined strength, psf	970.3		
Undrained shear strength, psf	485.1		
Failure strain, %	2.5		
Strain rate, %/min.	1.00		
Water content, %	44.2		
Wet density, pcf	109.1		
Dry density, pcf	75.6		
Saturation, %	97.1		
Void ratio	1.2288		
Specimen diameter, in.	1.395		
Specimen height, in.	2.837		
Height/diameter ratio	2.03		

**Description:** SO GR & BR CL6

LL = 46 PL = 24 PI = 22 Assumed GS = 2.70 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/10/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

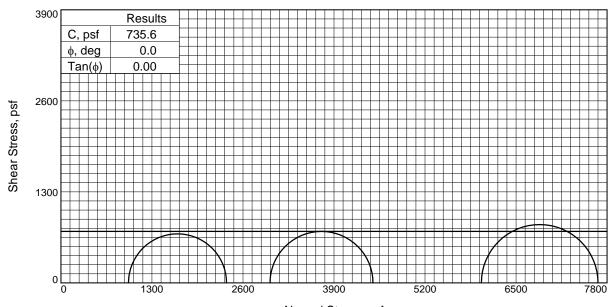
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-01 **Depth:** 66

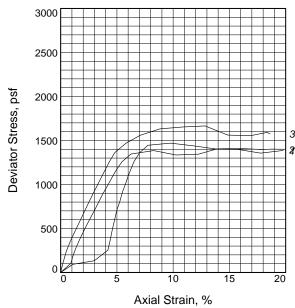
Sample Number: 17C

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Figure ASTM D2166



Normal Stress, psf



Туре	of	Test:

Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: M GR CH2 W/ ARS & LNS ML

**Assumed Specific Gravity=** 2.7 **Remarks:** TORVANE = 0.250 TSF

Figure A	ASTM	D2850
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	Spe	ecimen No.	1	2	3	
	Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	38.5 82.8 100.4 1.0345 1.381 2.749	36.0 83.9 96.2 1.0098 1.430 2.753	37.9 83.4 100.3 1.0200 1.387 2.746	
3 <b>2</b>	At Test	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	38.3 82.8 100.0 1.0345 1.381 2.749	37.4 83.9 100.0 1.0098 1.430 2.753	37.8 83.4 100.0 1.0200 1.387 2.746	
		ain rate, %/min. ck Pressure, psi	0.50 0.000	0.50 0.000	0.50 0.000	
		l Pressure, psi I. Stress, psf	6.700 1400.4	20.740 1469.9	41.700 1665.6	
		Stress, psf Failure, psf	1400.4 2365.2	1403.6 4456.5	1665.6 7670.4	
	$\sigma_3$	Failure, psf	964.8	2986.6	6004.8	

Client: AECOM, NEW ORLEANS, LOUISIANA

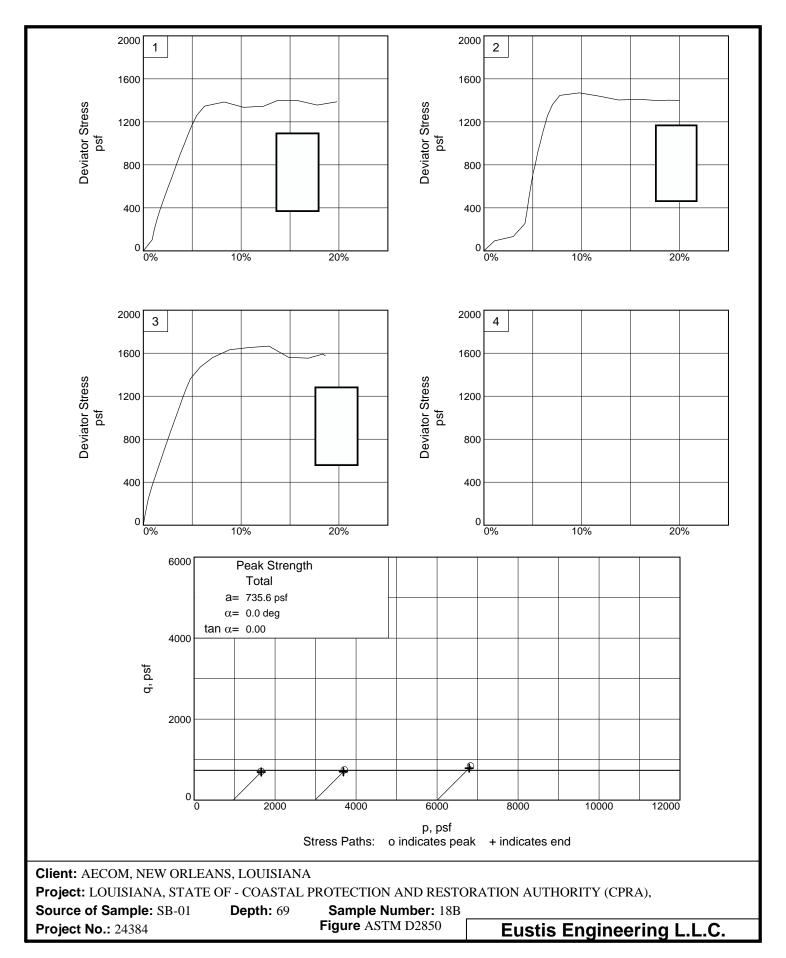
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

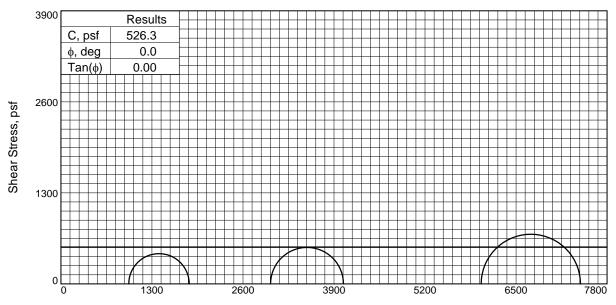
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-01 **Depth:** 69

Sample Number: 18B

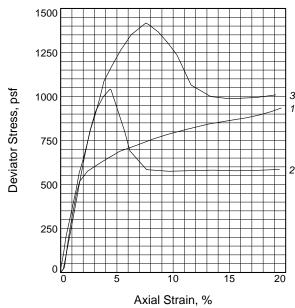
**Proj. No.:** 24384 **Date Sampled:** 9/17/20







Normal Stress, psf



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250																	Ī
230																	L
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	U																L
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Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: M GR & T CH3 W/ ARS & LNS ML

**LL=** 74 **PL=** 26 **PI=** 48

**Assumed Specific Gravity= 2.72 Remarks:** TORVANE = 0.375 TSF

	Spe	ecimen No.	1	2	3	
	_	Water Content, % Dry Density, pcf	50.6 71.5	48.8 73.0	49.3 72.5	
	Initial	Saturation, %	100.0	100.0	100.0	
	<u></u>	Void Ratio	1.3760	1.3261	1.3418	
3		Diameter, in.	1.419	1.411	1.419	
1		Height, in.	2.849	2.805	2.844	
		Water Content, %	50.6	48.8	49.3	
	#	Dry Density, pcf	71.5	73.0	72.5	
	At Test	Saturation, %	100.0	100.0	100.0	
2	Ę	Void Ratio	1.3760	1.3261	1.3418	
	4	Diameter, in.	1.419	1.411	1.419	
		Height, in.	2.849	2.805	2.844	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.730	20.790	41.680	
	Fai	l. Stress, psf	863.6	1041.3	1417.7	
	5	Strain, %	15.0	4.4	7.6	
	Ult.	Stress, psf	863.6	575.1	987.1	
	5	Strain, %	15.0	9.6	15.0	
-	$\sigma_{1}$	Failure, psf	1832.7	4035.0	7419.6	
	$\sigma_{\text{3}}$	Failure, psf	969.1	2993.8	6001.9	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

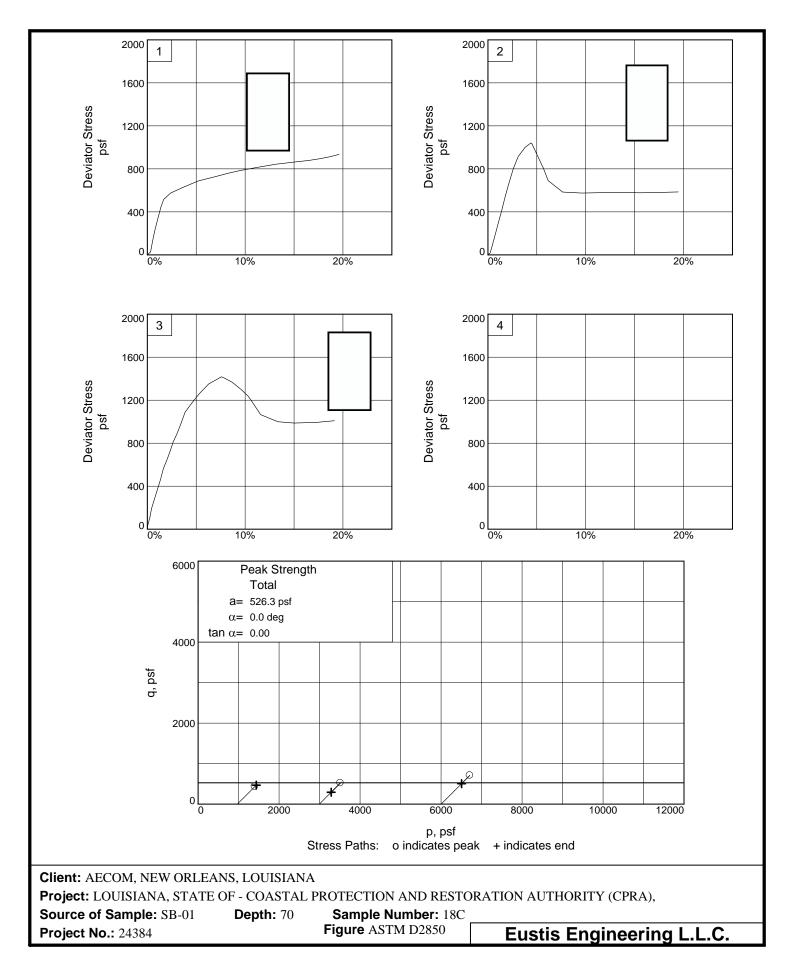
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-01 **Depth:** 70

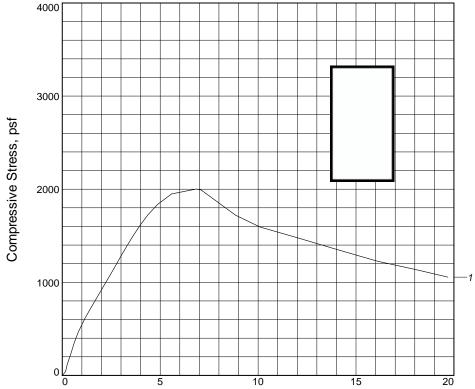
Sample Number: 18C

Proj. No.: 24384 **Date Sampled:** 9/10/20



Figure ASTM D2850





Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	2001.5	
Undrained shear strength, psf	1000.7	
Failure strain, %	6.8	
Strain rate, %/min.	1.00	
Water content, %	41.5	
Wet density, pcf	112.9	
Dry density, pcf	79.8	
Saturation, %	100.1	
Void ratio	1.1278	
Specimen diameter, in.	1.403	
Specimen height, in.	2.764	
Height/diameter ratio	1.97	

**Description:** ST GR & T CH3 W/ ARS & LNS ML

LL = 57 PL = 23 PI = 34 Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

**Date Sampled:** 9/10/20

Remarks:

TORVANE = 0.375 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

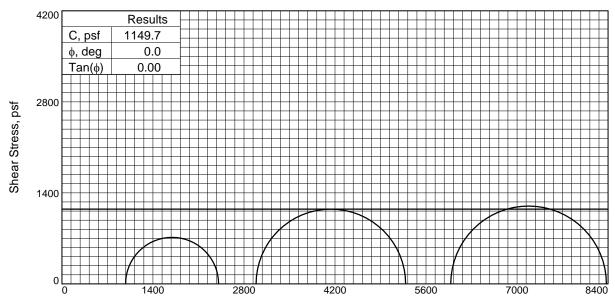
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-01 **Depth:** 77

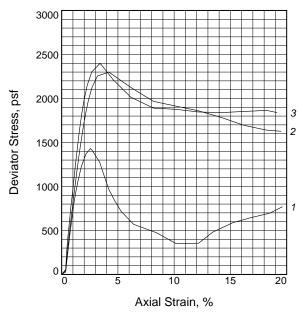
Sample Number: 20B

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Figure ASTM D2166



Normal Stress, psf



Туре	of	Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: ST GR CH3 W/ ARS ML, SIF, SL

**LL=** 59 **PL=** 18 **PI=** 41

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.500 TSF

	Spe	ecimen No.	1	2	3	
		Water Content, % Dry Density, pcf	36.7 84.4	36.8 84.2	35.3 86.6	
	Initial	Saturation, %	98.6	98.4	99.7	
		Void Ratio	1.0128	1.0170	0.9617	
		Diameter, in.	1.397	1.394	1.385	
3		Height, in.	2.753	2.754	2.754	
2		Water Content, %	37.2	37.4	35.4	
	;;	Dry Density, pcf	84.4	84.2	86.6	
	At Test	Saturation, %	100.0	100.0	100.0	
	'₹	Void Ratio	1.0128	1.0170	0.9617	
	`	Diameter, in.	1.397	1.394	1.385	
1		Height, in.	2.753	2.754	2.754	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	ll Pressure, psi	6.800	20.740	41.530	
	Fai	I. Stress, psf	1430.4	2299.6	2394.0	
	8	Strain, %	2.6	4.2	3.4	
	Ult.	Stress, psf	351.3	1784.4	1839.3	
	5	Strain, %	10.2	14.2	14.2	
	σ <sub>1</sub>	Failure, psf	2409.6	5286.2	8374.3	
	$\sigma_3$	Failure, psf	979.2	2986.6	5980.3	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

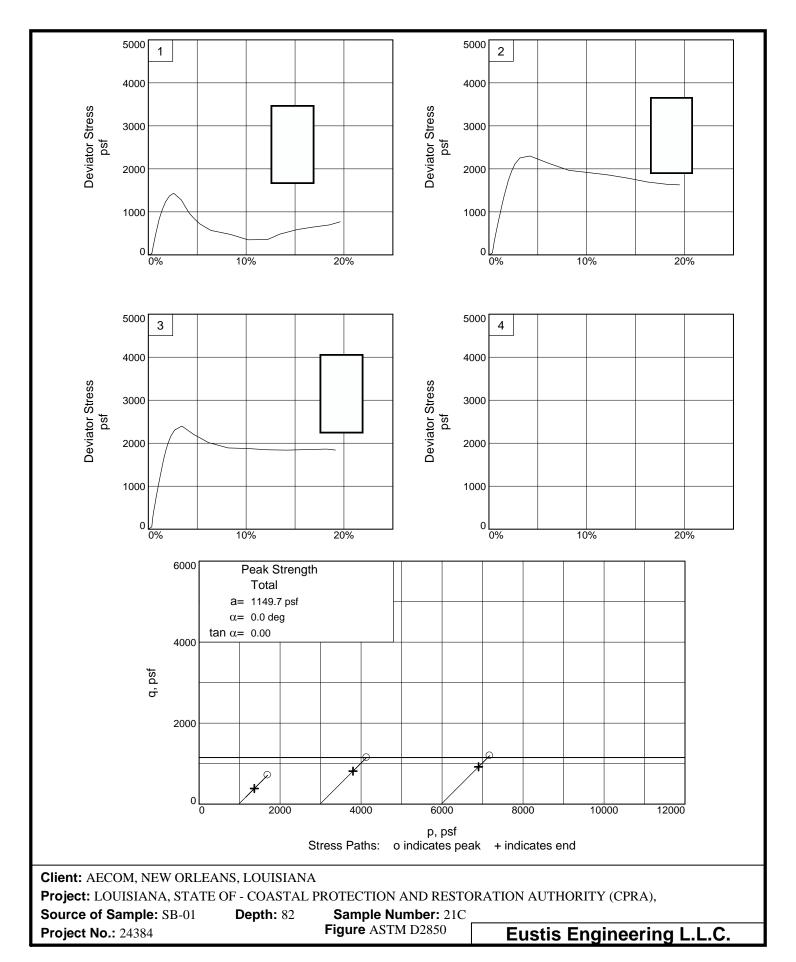
Source of Sample: SB-01 Depth: 82

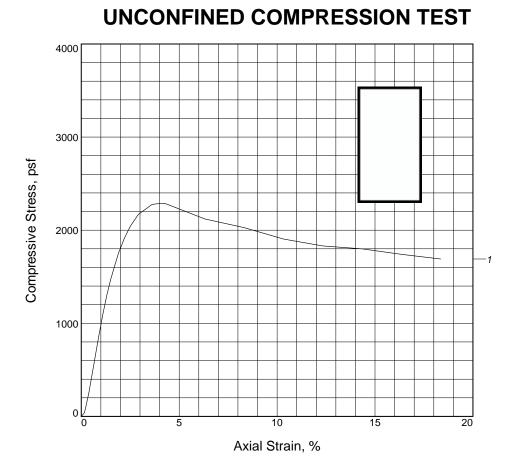
Sample Number: 21C

**Proj. No.:** 24384 **Date Sampled:** 9/10/20



Figure ASTM D2850





Specimen No.	1	
Unconfined strength, psf	2289.0	
Undrained shear strength, psf	1144.5	
Failure strain, %	4.1	
Strain rate, %/min.	1.00	
Water content, %	34.3	
Wet density, pcf	117.9	
Dry density, pcf	87.8	
Saturation, %	99.9	
Void ratio	0.9345	
Specimen diameter, in.	1.395	
Specimen height, in.	2.760	
Height/diameter ratio	1.98	

Description: ST GR CH4 W/ ARS ML, SL

LL = 94 PL = 22 PI = 72 Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/10/20

Remarks:

TORVANE = 0.400 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

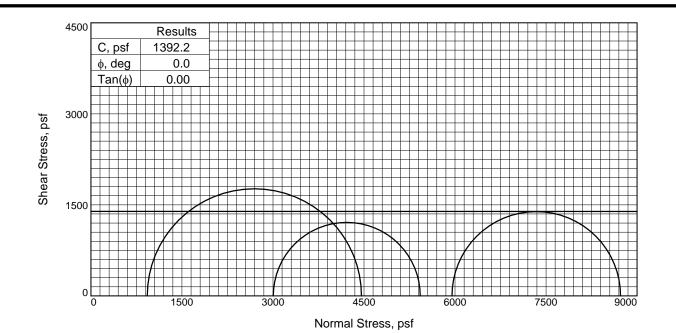
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

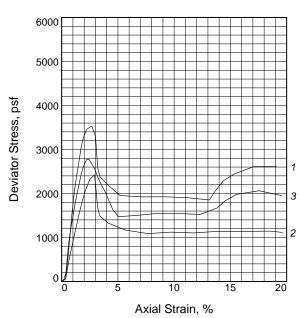
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-01 Depth: 86

Sample Number: 22C

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Figure ASTM D2166





Type of Test:

Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: ST GR CH4 W/ ARS ML, SL

**LL=** 89 **PL=** 29 **PI=** 60

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.400 TSF

Spe	ecimen No.	1	2	3	
	Water Content, %	44.8	43.4	46.7	
	Dry Density, pcf	76.2	77.2	74.8	
<u>a</u> .	Saturation, %	99.3	98.4	100.0	
Initial	Void Ratio	1.2279	1.1996	1.2708	
	Diameter, in.	1.398	1.393	1.393	
	Height, in.	2.787	2.774	2.786	
	Water Content, %	45.1	44.1	46.7	
;;	Dry Density, pcf	76.2	77.2	74.8	
At Test	Saturation, %	100.0	100.0	100.0	
🗐	Void Ratio	1.2279	1.1996	1.2708	
~	Diameter, in.	1.398	1.393	1.393	
	Height, in.	2.787	2.774	2.786	
Stra	ain rate, %/min.	1.00	1.00	1.00	
Bad	ck Pressure, psi	0.000	0.000	0.000	
Cel	l Pressure, psi	6.450	20.870	41.320	
Fai	l. Stress, psf	3529.7	2421.4	2777.8	
5	Strain, %	2.6	2.9	2.3	
Ult.	Stress, psf	1852.0	1089.0	1471.0	
5	Strain, %	13.1	7.7	5.0	
σ <sub>1</sub>	Failure, psf	4458.5	5426.7	8727.9	
σ3	Failure, psf	928.8	3005.3	5950.1	

Client: AECOM, NEW ORLEANS, LOUISIANA

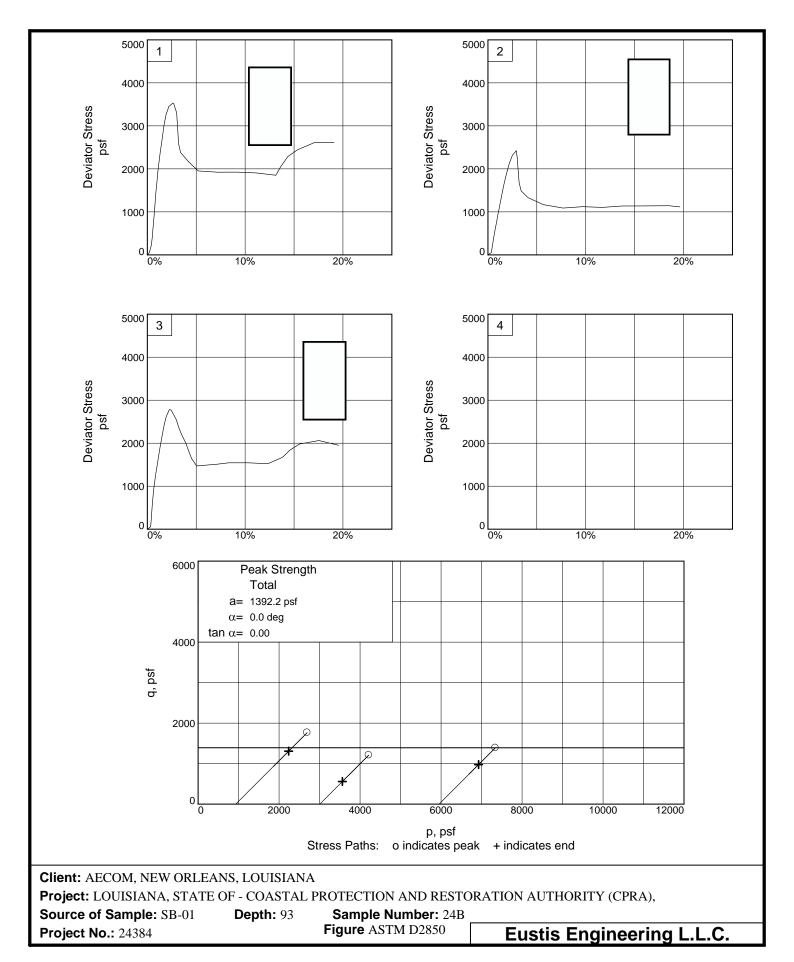
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

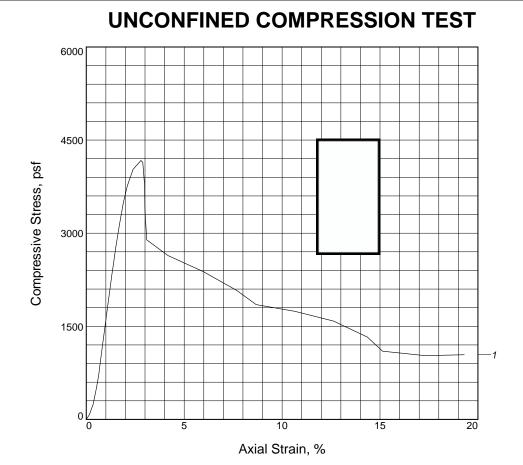
Sample Number: 24B

**Proj. No.:** 24384 **Date Sampled:** 9/10/20



Figure ASTM D2850





Specimen No.	1		
Unconfined strength, psf	4171.5		
Undrained shear strength, psf	2085.7		
Failure strain, %	2.8		
Strain rate, %/min.	1.00		
Water content, %	48.2		
Wet density, pcf	108.9		
Dry density, pcf	73.5		
Saturation, %	100.0		
Void ratio	1.3109		
Specimen diameter, in.	1.396		
Specimen height, in.	2.796		
Height/diameter ratio	2.00		

**Description:** VST GR CH4 W/ ARS & LNS ML

**LL =** 89 PL = 28**PI** = 61 Assumed GS= 2.72 **Type:** UNDISTURBED

Project No.: 24384

**Date Sampled:** 9/10/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

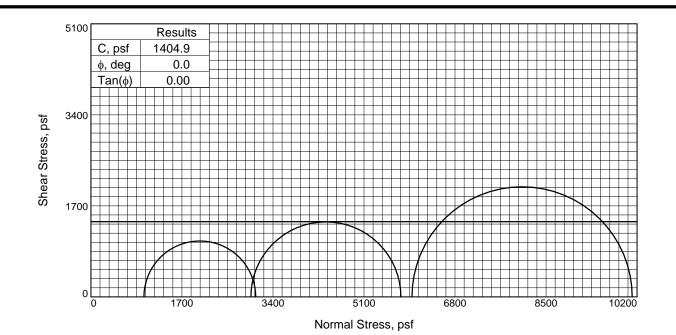
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

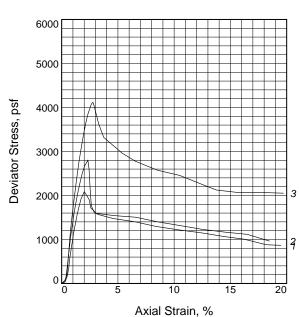
RESTORATION AUTHORITY (CPRA), **Source of Sample: SB-01** 

Sample Number: 25B

Figure ASTM D2166

Checked By: RR Tested By: DE





Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: ST GR CH4 W/ ARS ML

**LL=** 91 **PL=** 28 **PI=** 63

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.400 TSF

Figure	ASTM	D2850
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	Spe	ecimen No.	1	2	3	
		Water Content, %	46.0	45.9	45.3	
		Dry Density, pcf	75.4	75.6	76.1	
	Initial	Saturation, %	100.0	100.0	100.0	
	in	Void Ratio	1.2517	1.2476	1.2322	
		Diameter, in.	1.398	1.396	1.394	
		Height, in.	2.768	2.798	2.788	
		Water Content, %	46.0	45.9	45.3	
	, <del>,</del>	Dry Density, pcf	75.4	75.6	76.1	
	At Test	Saturation, %	100.0	100.0	100.0	
	'≠	Void Ratio	1.2517	1.2476	1.2322	
3		Diameter, in.	1.398	1.396	1.394	
		Height, in.	2.768	2.798	2.788	
	Strain rate, %/min.		1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cell Pressure, psi		6.880	20.750	41.640	
	Fail. Stress, psf		2087.7	2801.0	4112.4	
	5	Strain, %		2.3	2.7	
	Ult.	Stress, psf	1067.5	1167.9	2124.1	
	5	Strain, %		14.5	13.8	
	σ <sub>1</sub>	Failure, psf	3078.4	5789.0	10108.6	
	$\sigma_3$	Failure, psf	990.7	2988.0	5996.2	

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

Sample Number: 26C

**Proj. No.:** 24384 **Date Sampled:** 9/10/20



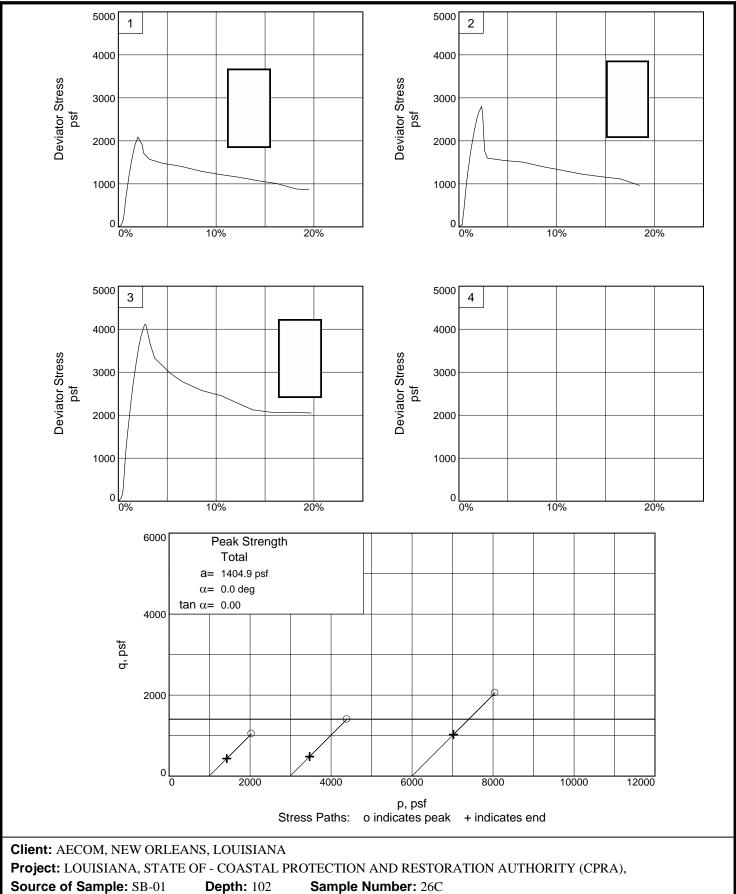
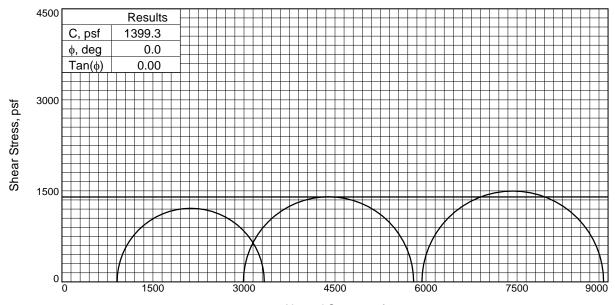


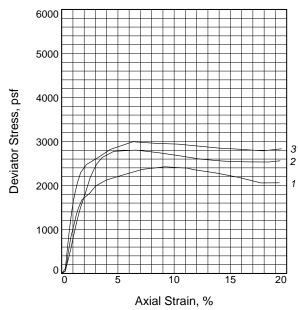
Figure ASTM D2850

**Eustis Engineering L.L.C.** Project No.: 24384



Normal Stress, psf

Specimen No.



- 1	٠,	001110111101	•	_	•	
		Water Content, % Dry Density, pcf	24.4 101.0	24.3 99.8	24.7 99.1	
-	<u>a</u>	Saturation, %	97.2	94.5	94.2	
-	Initial	Void Ratio	0.6818	0.7011	0.7132	
-	_	Diameter, in.	1.373	1.396	1.394	
		Height, in.	2.763	2.753	2.757	
		Water Content, %	25.1	25.8	26.2	
3	<del>,,</del>	Dry Density, pcf	101.0	99.8	99.1	
2	At Test	Saturation, %	100.0	100.0	100.0	
-	7	Void Ratio	0.6818	0.7011	0.7132	
1	_	Diameter, in.	1.373	1.396	1.394	
		Height, in.	2.763	2.753	2.757	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
-	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	ll Pressure, psi	6.320	20.770	41.200	
	Fai	I. Stress, psf	2424.4	2805.0	2993.9	
	5	Strain, %	9.2	6.6	6.3	
	Ult.	Stress, psf	2285.1	2553.5	2832.7	
	5	Strain, %	13.7	14.4	15.3	
┪	$\sigma_{\text{1}}$	Failure, psf	3334.5	5795.9	8926.7	
	$\sigma_{\text{3}}$	Failure, psf	910.1	2990.9	5932.8	

1

2

3

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: ST GR CL6 W/ WD, RT

**LL=** 41 **PL=** 16 **PI=** 25

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

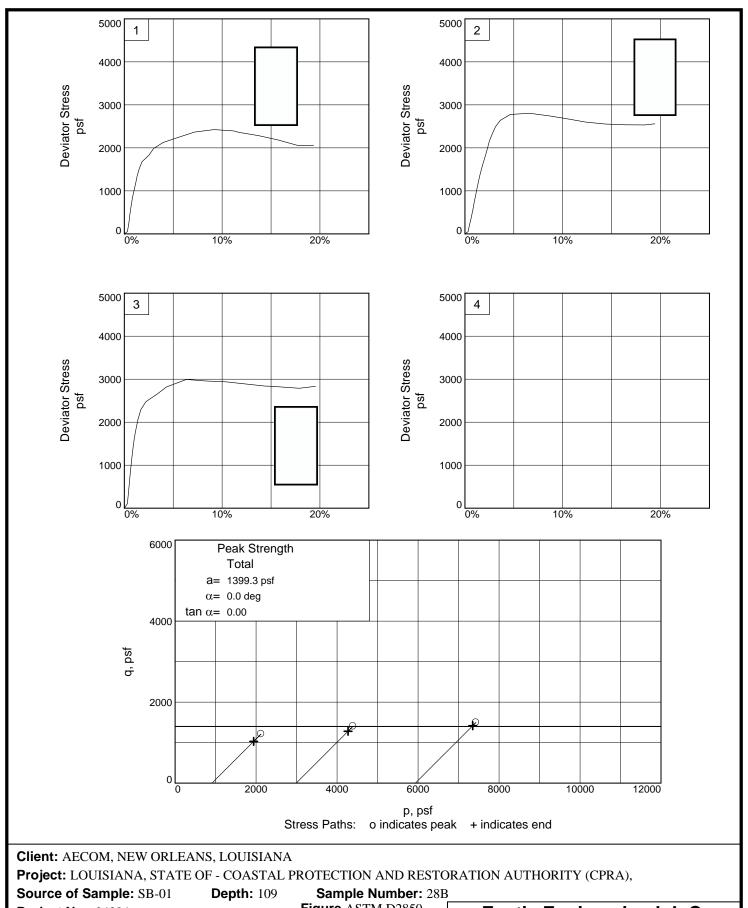
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

Sample Number: 28B

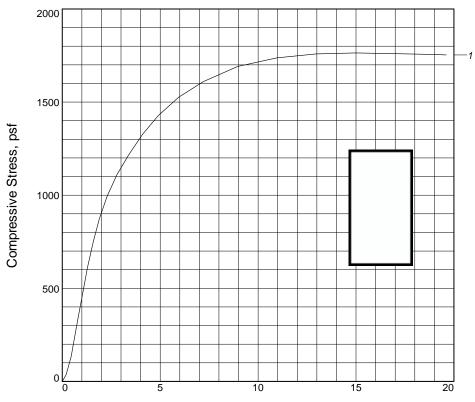
**Proj. No.:** 24384 **Date Sampled:** 9/10/20



Figure ASTM D2850



Project No.: 24384 Figure ASTM D2850 Eustis Engineering L.L.C.



Axial Strain, %

Specimen No.	1		
Unconfined strength, psf	1764.1		
Undrained shear strength, psf	882.1		
Failure strain, %	15.0		
Strain rate, %/min.	1.00		
Water content, %	35.1		
Wet density, pcf	115.3		
Dry density, pcf	85.4		
Saturation, %	96.5		
Void ratio	0.9895		
Specimen diameter, in.	1.377		
Specimen height, in.	2.761		
Height/diameter ratio	2.01		

**Description:** M GR & T CH3 W/ ARS ML, CC, SL

LL = 67 PL = 25 PI = 42 Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/7/20

Remarks:

TORVANE = 0.750 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

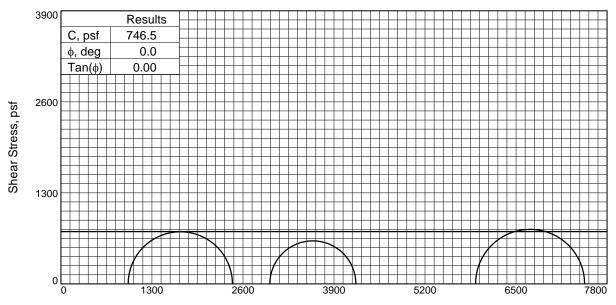
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-02 **Depth:** 3

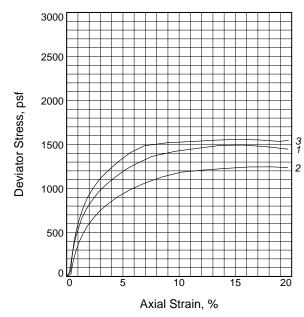
Sample Number: 2B

EUSTIS ENGINEERING

Figure ASTM D2166



Normal Stress, psf



Туре	of	Test:
·ypc	0.	

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: M GR & T CH3 W/ ARS ML, CC

**LL=** 73 **PL=** 21 **PI=** 52

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.625 TSF

Spe	ecimen No.	1	2	3	
	Water Content, %	40.0	38.9	39.6	
	Dry Density, pcf	81.2	82.4	81.2	
lä.	Saturation, %	99.8	99.7	98.8	
Initia	Void Ratio	1.0903	1.0616	1.0904	
	Diameter, in.	1.420	1.400	1.396	
	Height, in.	2.759	2.753	2.760	
	Water Content, %	40.1	39.0	40.1	
;;	Dry Density, pcf	81.2	82.4	81.2	
At Test	Saturation, %	100.0	100.0	100.0	
=	Void Ratio	1.0903	1.0616	1.0904	
~	Diameter, in.	1.420	1.400	1.396	
	Height, in.	2.759	2.753	2.760	
Stra	ain rate, %/min.	1.00	1.00	1.00	
Bad	ck Pressure, psi	0.000	0.000	0.000	
Cel	l Pressure, psi	6.670	20.720	41.120	
Fai	l. Stress, psf	1489.2	1228.9	1558.1	
5	Strain, %	13.6	14.2	14.9	
Ult.	Stress, psf	1489.2	1228.9	1558.1	
5	Strain, %	13.6	14.2	14.9	
σ₁	Failure, psf	2449.7	4212.6	7479.4	
$\sigma_3$	Failure, psf	960.5	2983.7	5921.3	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

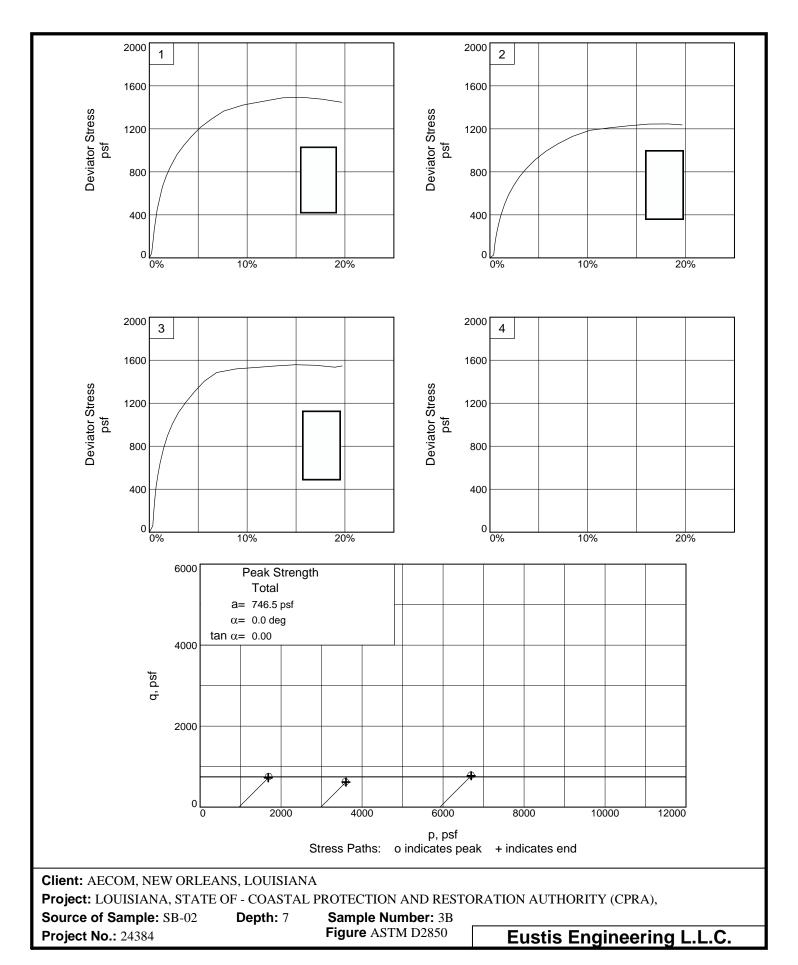
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-02 **Depth:** 7

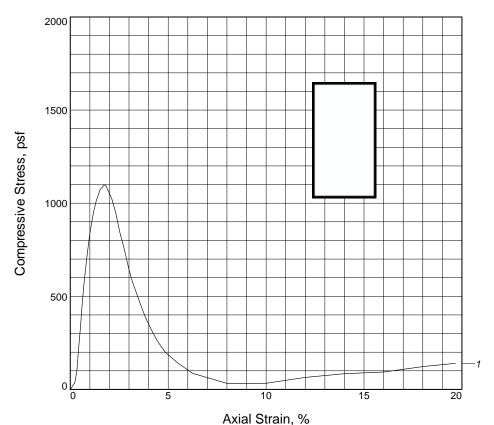
Sample Number: 3B

**Proj. No.:** 24384 **Date Sampled:** 10/7/20



Figure ASTM D2850





Specimen No.	1	
Unconfined strength, psf	1098.4	
Undrained shear strength, psf	549.2	
Failure strain, %	1.7	
Strain rate, %/min.	1.00	
Water content, %	55.4	
Wet density, pcf	103.8	
Dry density, pcf	66.8	
Saturation, %	97.6	
Void ratio	1.5433	
Specimen diameter, in.	1.390	
Specimen height, in.	2.755	
Height/diameter ratio	1.98	

Description: M GR CH4 W/ ARS ML, SL

**LL** = 104 **PL** = 30 **PI** = 74 **Assumed GS**= 2.72 **Type:** UNDISTURBED

Project No.: 24384

Date Sampled: 10/7/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

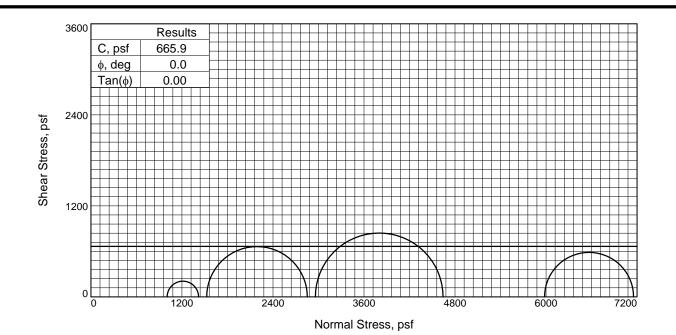
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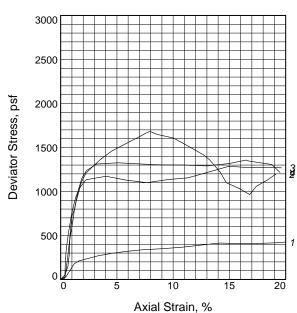
Source of Sample: SB-02 Depth: 11

Sample Number: 4B

EUSTIS ENGINEERING SINCE 1946

Figure ASTM D2166





Type of Test:

Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: M GR & T CH4 W/ ARS ML, SL, WD,

RT

**LL=** 94 **PL=** 25 **PI=** 69

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.375 TSF

	Spe	ecimen No.	1	2	3	4
		Water Content, %	56.3	58.4	54.7	54.3
	Initial	Dry Density, pcf	64.3	63.8	66.6	66.9
		Saturation, %	93.3	95.6	96.0	96.0
	<u>=</u>	Void Ratio	1.6413	1.6612	1.5506	1.5386
		Diameter, in.	1.406	1.422	1.418	1.396
		Height, in.	2.844	2.868	2.859	2.755
		Water Content, %	60.3	61.1	57.0	56.6
	+,,	Dry Density, pcf	64.3	63.8	66.6	66.9
3	At Test	Saturation, %	100.0	100.0	100.0	100.0
!		Void Ratio	1.6413	1.6612	1.5506	1.5386
		Diameter, in.	1.406	1.422	1.418	1.396
		Height, in.	2.844	2.868	2.859	2.755
	Strain rate, %/min.		1.00	1.00	1.00	1.00
1	Bad	ck Pressure, psi	0.000	0.000	0.000	0.000
	Cel	l Pressure, psi	6.980	20.550	41.530	10.600
	Fai	I. Stress, psf	413.9	1682.9	1173.3	1326.0
	8	Strain, %	14.2	7.9	4.0	5.1
	Ult.	Stress, psf	413.9	1101.3	1100.2	1290.2
	8	Strain, %	14.2	14.7	7.7	13.0
	σ <sub>1</sub>	Failure, psf	1419.0	4642.1	7153.6	2852.4
	$\sigma_3$	Failure, psf	1005.1	2959.2	5980.3	1526.4

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

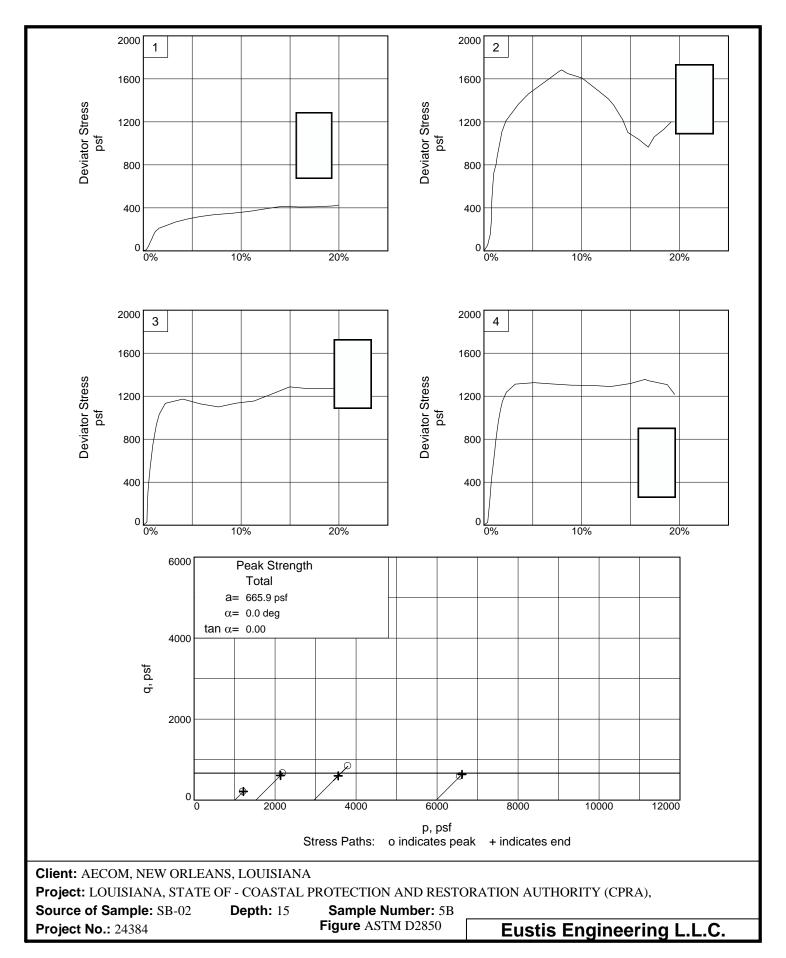
Source of Sample: SB-02 Depth: 15

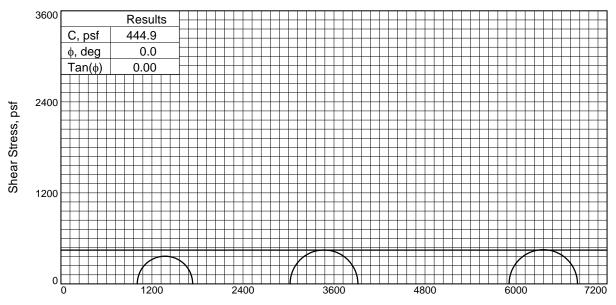
Sample Number: 5B

**Proj. No.:** 24384 **Date Sampled:** 10/7/20

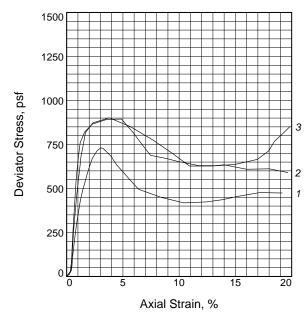


Figure ASTM D2850





Normal Stress, psf



_		_	
Type	<b>∩</b> t	ΙД	ct.
IVDE	vı		ЭL.

Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: SO GR & BR CHOB W/ WD

**LL=** 171 **PL=** 53 **PI=** 118

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.625 TSF

Sp	ecimen No.	1	2	3	
	Water Content, %	110.2	108.0	109.0	
	Dry Density, pcf	40.5	41.9	41.2	
Initial	Saturation, %	94.0	96.2	95.1	
:=	Void Ratio	3.1891	3.0519	3.1183	
	Diameter, in.	1.409	1.392	1.421	
	Height, in.	2.771	2.760	2.765	
	Water Content, %	117.2	112.2	114.6	
۱ بر	Dry Density, pcf	40.5	41.9	41.2	
At Test	Saturation, %	100.0	100.0	100.0	
'#	Void Ratio	3.1891	3.0519	3.1183	
`	Diameter, in.	1.409	1.392	1.421	
	Height, in.	2.771	2.760	2.765	
Str	ain rate, %/min.	1.00	1.00	1.00	
Ba	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	6.990	20.980	41.030	
Fa	il. Stress, psf	730.6	894.3	901.8	
;	Strain, %	3.0	4.8	3.8	
Ult	. Stress, psf	418.2	629.5	627.5	
;	Strain, %	10.4	13.2	10.9	
σ₁	Failure, psf	1737.2	3915.4	6810.1	
$\sigma_3$	Failure, psf	1006.6	3021.1	5908.3	

Client: AECOM, NEW ORLEANS, LOUISIANA

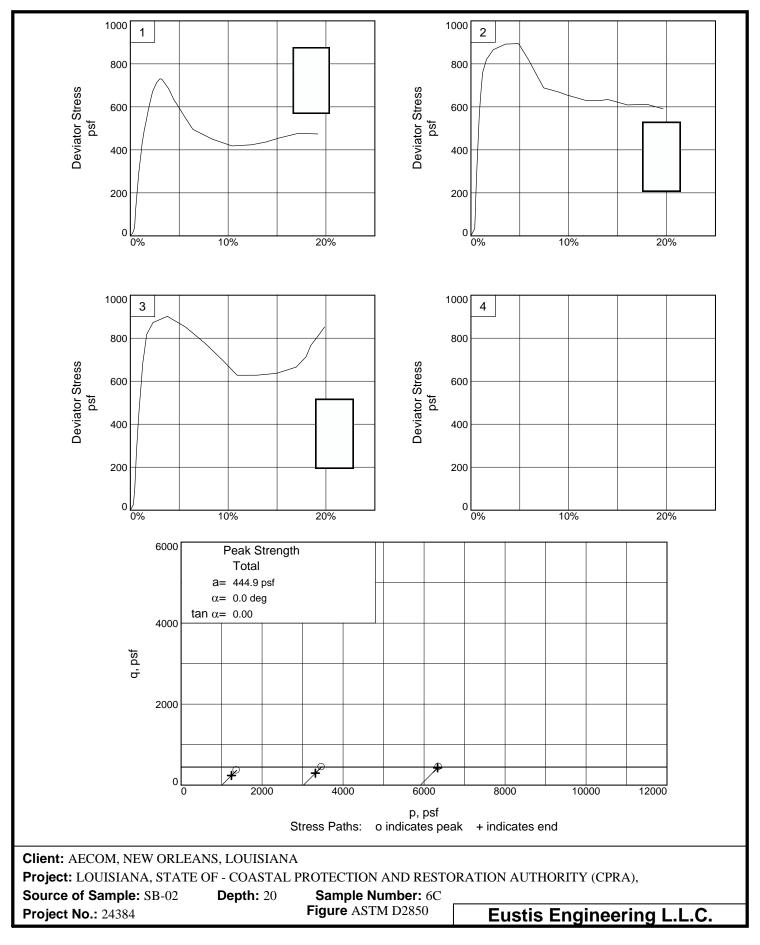
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

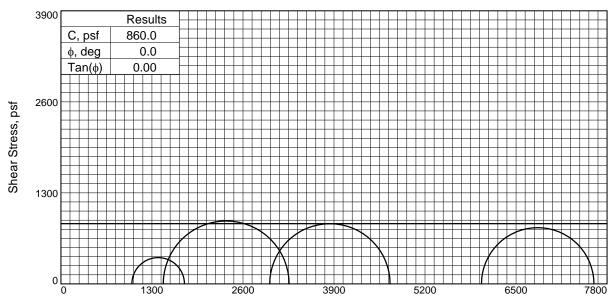
Sample Number: 6C

**Proj. No.:** 24384 **Date Sampled:** 10/7/20

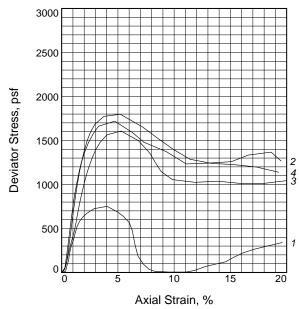


Figure ASTM D2850





Normal Stress, psf



Type	of	Test:
i ypc	O.	I COL.

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: M BR & GR CHOA

**LL=** 152 **PL=** 53 **PI=** 99

**Assumed Specific Gravity=** 2.70 **Remarks:** TORVANE = 0.625 TSF

	Spe	ecimen No.	1	2	3	4
		Water Content, %	95.3	96.0	96.7	94.0
		Dry Density, pcf	45.6	45.5	45.4	46.8
	nitial	Saturation, %	95.3	95.7	96.2	97.4
		Void Ratio	2.6982	2.7081	2.7141	2.6045
		Diameter, in.	1.414	1.417	1.403	1.402
		Height, in.	2.753	2.766	2.758	2.804
		Water Content, %	99.9	100.3	100.5	96.5
	;;	Dry Density, pcf	45.6	45.5	45.4	46.8
	<u>ĕ</u>	Saturation, %	100.0	100.0	100.0	100.0
!	At Test	Void Ratio	2.6982	2.7081	2.7141	2.6045
•		Diameter, in.	1.414	1.417	1.403	1.402
		Height, in.	2.753	2.766	2.758	2.804
	Strain rate, %/min.		0.99	1.00	1.00	1.00
	Back Pressure, psi		0.000	0.000	0.000	0.000
	Cel	l Pressure, psi	7.040	20.710	41.710	10.150
	Fail. Stress, psf		750.6	1718.7	1605.8	1796.8
	8	Strain, %	4.0	4.7	5.2	5.1
	Ult.	Stress, psf	5.1	1233.3	1024.3	1227.3
	5	Strain, %	11.0	11.1	11.9	15.2
	σ <sub>1</sub>	Failure, psf	1764.4	4700.9	7612.1	3258.4
	$\sigma_3$	Failure, psf	1013.8	2982.2	6006.2	1461.6

Client: AECOM, NEW ORLEANS, LOUISIANA

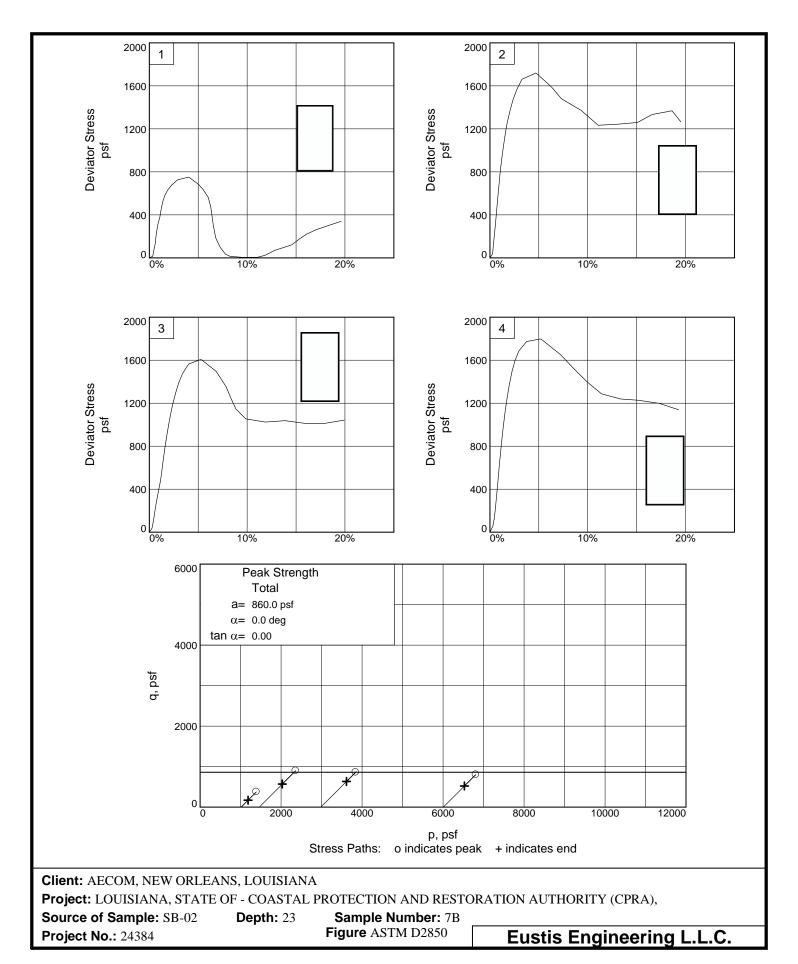
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

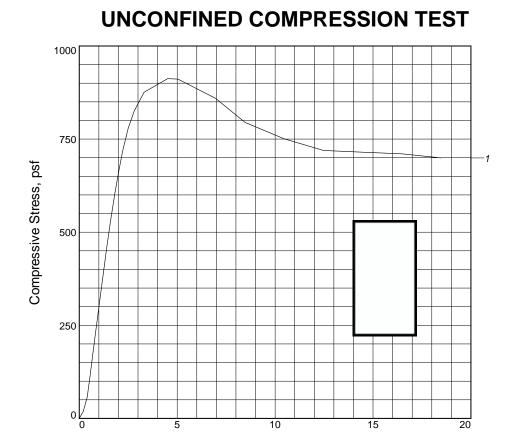
Sample Number: 7B

**Proj. No.:** 24384 **Date Sampled:** 10/7/20



Figure ASTM D2850





Specimen No.	1		
Unconfined strength, psf	912.3		
Undrained shear strength, psf	456.1		
Failure strain, %	4.5		
Strain rate, %/min.	1.00		
Water content, %	79.2		
Wet density, pcf	94.7		
Dry density, pcf	52.8		
Saturation, %	97.3		
Void ratio	2.2154		
Specimen diameter, in.	1.390		
Specimen height, in.	2.766		
Height/diameter ratio	1.99		

Axial Strain, %

Description: SO BR & GR CH4 W/ ARS ML, O

**LL** = 103 **PL** = 41 **PI** = 62 **Assumed GS**= 2.72 **Type:** UNDISTURBED

Project No.: 24384

Date Sampled: 10/7/20

Remarks:

TORVANE = 0.500 TSF

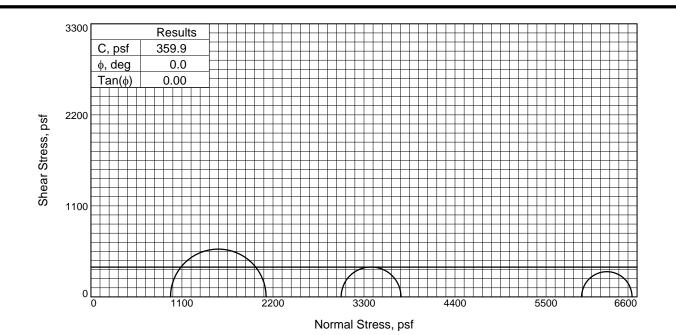
Client: AECOM, NEW ORLEANS, LOUISIANA

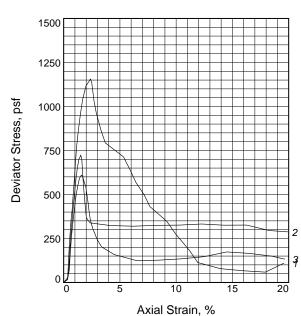
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

Sample Number: 7C

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Figure ASTM D2166





Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR CH3 W/ ARS ML, SIF

**PI=** 51 **PL=** 25 **LL=** 76

Assumed Specific Gravity= 2.72

Assumeu	Specific Gravity = 2.	12
Remarks:	TORVANE = 0.200 TS	SF

Spe	ecimen No.	1	2	3	
	Water Content, %	73.6	73.7	73.4	
	Dry Density, pcf	55.8	55.5	56.7	
lä.	Saturation, %	98.0	97.4	99.9	
Initia	Void Ratio	2.0435	2.0579	1.9972	
	Diameter, in.	1.402	1.403	1.397	
	Height, in.	2.763	2.760	2.769	
	Water Content, %	75.1	75.7	73.4	
ی ا	Dry Density, pcf	55.8	55.5	56.7	
At Test	Saturation, %	100.0	100.0	100.0	
=	Void Ratio	2.0435	2.0579	1.9972	
~	Diameter, in.	1.402	1.403	1.397	
	Height, in.	2.763	2.760	2.769	
Stra	ain rate, %/min.	1.00	1.00	1.00	
Bad	ck Pressure, psi	0.000	0.000	0.000	
Cel	l Pressure, psi	6.680	21.000	41.180	
Fai	I. Stress, psf	1155.4	723.1	609.3	
5	Strain, %	2.4	1.5	1.6	
Ult.	Stress, psf	78.5	319.9	123.8	
5	Strain, %	13.9	6.2	6.5	
σ <sub>1</sub>	Failure, psf	2117.3	3747.1	6539.2	
$\sigma_3$	Failure, psf	961.9	3024.0	5929.9	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

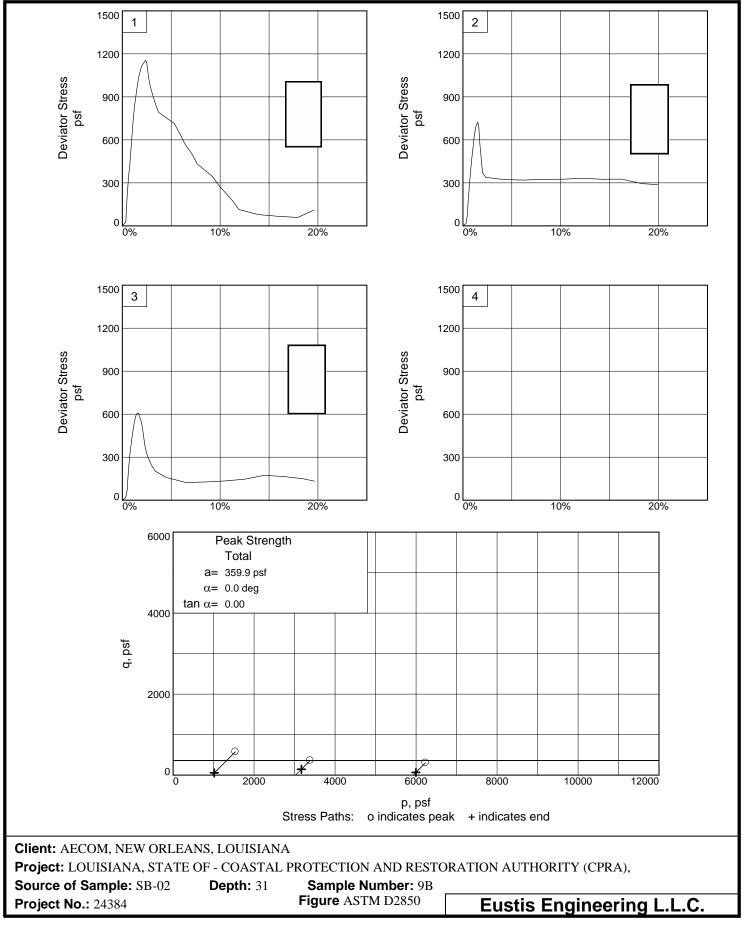
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-02 Depth: 31

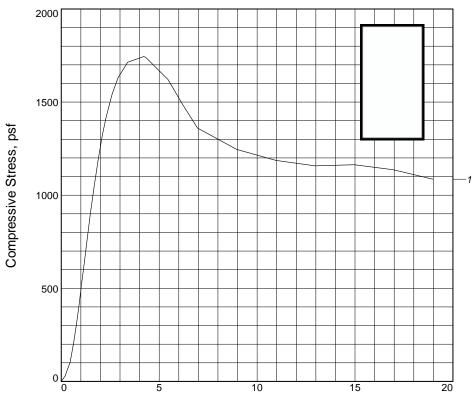
Sample Number: 9B

Proj. No.: 24384 **Date Sampled:** 10/4/20



Figure ASTM D2850





Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	1744.2	
Undrained shear strength, psf	872.1	
Failure strain, %	4.2	
Strain rate, %/min.	1.00	
Water content, %	87.9	
Wet density, pcf	89.9	
Dry density, pcf	47.9	
Saturation, %	93.8	
Void ratio	2.5475	
Specimen diameter, in.	1.424	
Specimen height, in.	2.760	
Height/diameter ratio	1.94	

Description: M GR & BR CHOA W/ WD

**PL** = 43 **LL =** 120 **PI** = 77 **Assumed GS=** 2.72 **Type:** UNDISTURBED

Project No.: 24384 **Date Sampled:** 10/7/20

Remarks:

TORVANE = 0.750 TSF

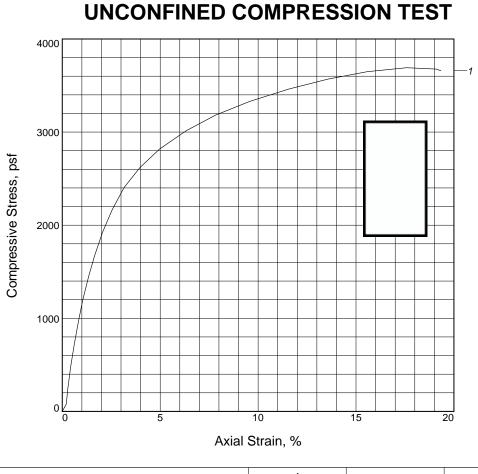
Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample: SB-02** Depth: 35

Sample Number: 10B

Figure ASTM D2166



1	
3569.3	
1784.6	
13.6	
1.00	
23.8	
124.2	
100.3	
93.4	
0.6930	
1.398	
2.742	
1.96	
	1784.6  13.6  1.00  23.8  124.2  100.3  93.4  0.6930  1.398  2.742

**Description:** ST GR & T CH3 W/ ARS ML

LL = PL = PI = Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/7/20

Remarks:

TORVANE = 1.000 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

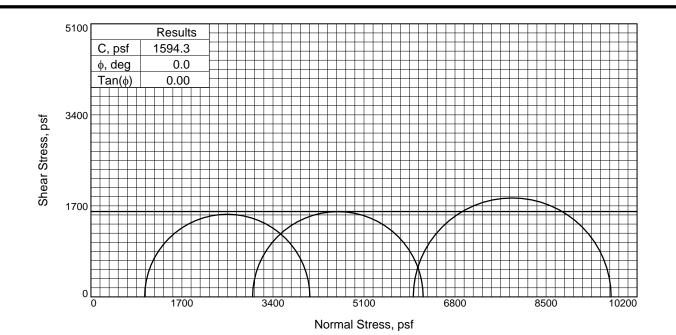
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

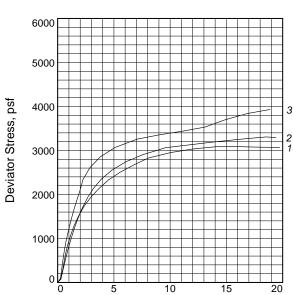
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-02 **Depth:** 39

Sample Number: 11B

EUSTIS ENGINEERING SINCE 1946

Figure ASTM D2166





Axial Strain, %

Type of Test:

Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: ST GR & T CH3 W/ ARS ML

**LL=** 59 **PL=** 15 **PI=** 44

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.875 TSF

	Spe	ecimen No.	1	2	3	
		Water Content, %	23.5	23.0	22.4	
		Dry Density, pcf	99.1	99.2	99.7	
	nitial	Saturation, %	89.5	88.0	86.7	
	<u>=</u>	Void Ratio	0.7134	0.7116	0.7035	
3		Diameter, in.	1.419	1.399	1.417	
		Height, in.	2.770	2.752	2.753	
2		Water Content, %	26.2	26.2	25.9	
1	+,	Dry Density, pcf	99.1	99.2	99.7	
	At Test	Saturation, %	100.0	100.0	100.0	
	=	Void Ratio	0.7134	0.7116	0.7035	
	1	Diameter, in.	1.419	1.399	1.417	
		Height, in.	2.770	2.752	2.753	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.970	20.970	41.810	
	Fai	I. Stress, psf	3085.0	3185.4	3694.7	
	5	Strain, %	14.1	13.6	14.8	
	Ult.	Stress, psf		3185.4	3694.7	
	5	Strain, %		13.6	14.8	
_	$\sigma_1$	Failure, psf	4088.6	6205.1	9715.4	
	$\sigma_3$	Failure, psf	1003.7	3019.7	6020.6	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

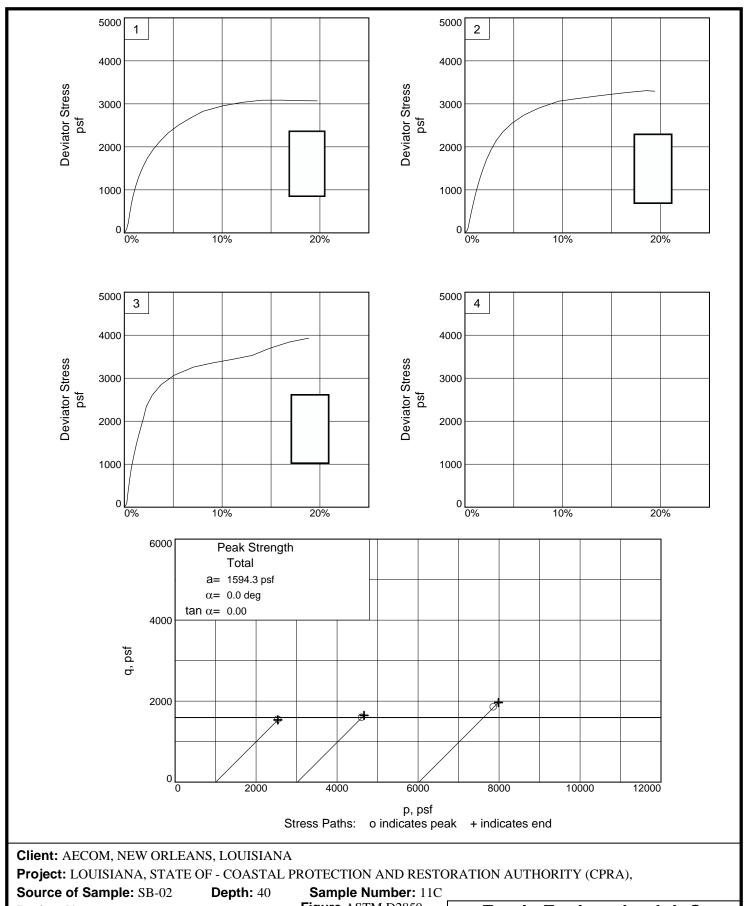
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-02 **Depth:** 40

Sample Number: 11C

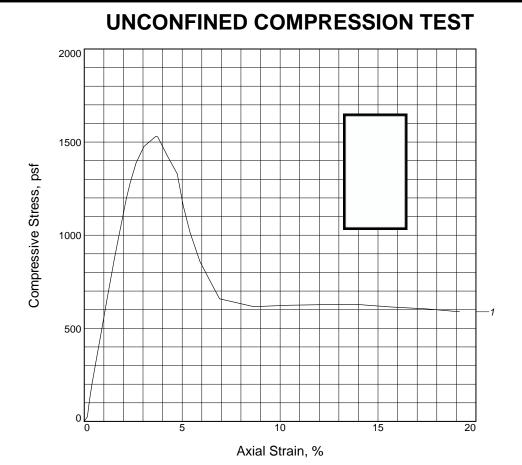
**Proj. No.:** 24384 **Date Sampled:** 10/7/20



Figure ASTM D2850



Project No.: 24384 Figure ASTM D2850 Eustis Engineering L.L.C.



- N		
Specimen No.	1	
Unconfined strength, psf	1530.1	
Undrained shear strength, psf	765.1	
Failure strain, %	3.6	
Strain rate, %/min.	1.00	
Water content, %	34.8	
Wet density, pcf	112.7	
Dry density, pcf	83.6	
Saturation, %	92.5	
Void ratio	1.0159	
Specimen diameter, in.	1.412	
Specimen height, in.	2.895	
Height/diameter ratio	2.05	
Description: M T & GR CL4 W/ CC		-

Project No.: 24384

Date Sampled: 10/7/20

**P**L = 20

**PI** = 14

Remarks:

**LL =** 34

TORVANE = 0.150 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

**Assumed GS=** 2.70

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

**Type:** UNDISTURBED

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-02 **Depth:** 48

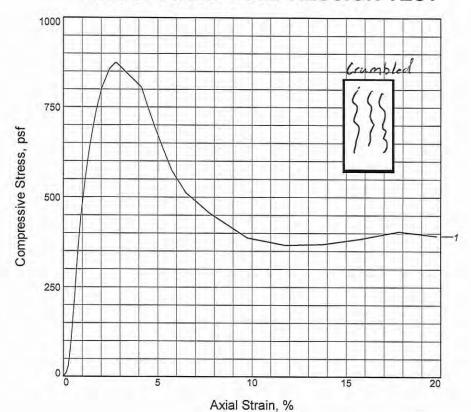
Sample Number: 13C

EUSTIS ENGINEERING SINCE 1946

Figure ASTM D2166

Tested By: CC

Checked By: RR



Specimen No.	1	
Unconfined strength, psf	874.3	
Undrained shear strength, psf	437.2	
Failure strain, %	2.7	
Strain rate, %/min.	1.00	
Water content, %	32.9	
Wet density, pcf	113.2	
Dry density, pcf	85.2	
Saturation, %	90.1	
Void ratio	0.9941	
Specimen diameter, in.	1.391	
Specimen height, in.	2.761	
Height/diameter ratio	1.98	

Description: M GR & BR CH3 W/ ARS SP, RT, SIF, SL

LL = 67PL = 25 PI = 42Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/20/20

Remarks: TORVANE = 0.375 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

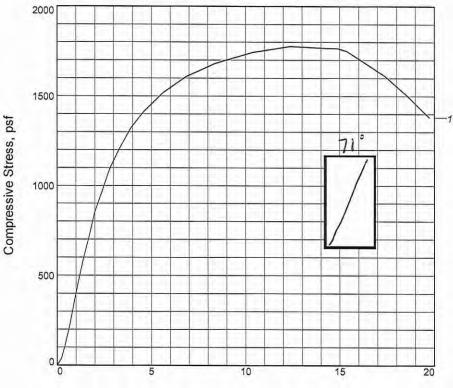
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 2

Sample Number: 1C

Figure ASTM D2166

Tested By: CC

Checked By: RR



Axial	Strain,	%
		100.0

Specimen No.	1	
Unconfined strength, psf	1777.5	
Undrained shear strength, psf	888.7	
Failure strain, %	12.4	
Strain rate, %/min.	1.00	
Water content, %	27.4	
Wet density, pcf	122.8	
Dry density, pcf	96.4	
Saturation, %	97.7	
Void ratio	0.7616	
Specimen diameter, in.	1.402	
Specimen height, in.	2.769	
Height/diameter ratio	1.98	

Description: M T & GR CH2 W/ CC, RT

PI = 31 LL = 51PL = 20 Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/20/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

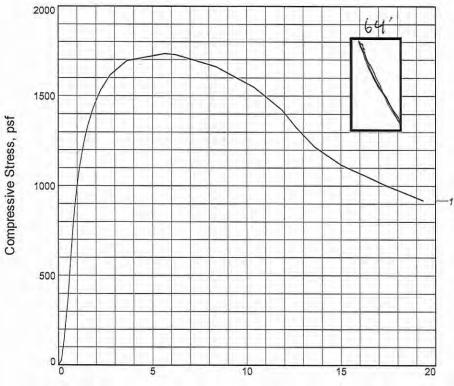
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 5

Sample Number: 2B

Figure ASTM D2166

Tested By: DE

Checked By: RR



Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	1733.9	
Undrained shear strength, psf	867.0	
Failure strain, %	5.6	
Strain rate, %/min.	1.00	
Water content, %	41.9	
Wet density, pcf	112.6	
Dry density, pcf	79.4	
Saturation, %	100.0	
Void ratio	1.1388	
Specimen diameter, in.	1.406	
Specimen height, in.	2.784	
Height/diameter ratio	1.98	

Description: M GR & T CH3 W/ ARS ML, CC

LL = 72 PL = 25 PI = 47 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/20/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

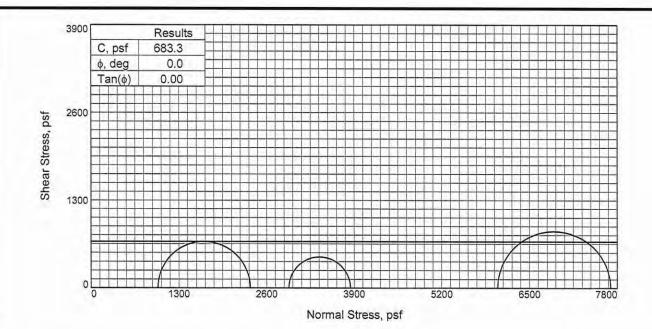
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

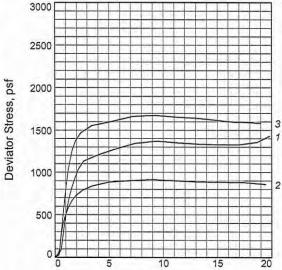
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 9

Sample Number: 3B

EUSTIS ENGINEERING SINCE 1946

Figure ASTM D2166





Axial Strain, %

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: M GR CH4 W/ ARS ML, WD

LL= 88 P

PL= 27

PI= 61

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.625 TSF

Proj. No.: 243

	Spi	ecimen No.	1	2	3	
	Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	45.9 74.7 98.1 1.2735 1.403 2.762	46.9 72.1 94.1 1.3565 1.386 2.783	45.0 75.0 96.8 1.2654 1.395 2.753	
3	AtTest	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	46.8 74.7 100.0 1.2735 1.403 2.762	49.9 72.1 100.0 1.3565 1.386 2.783	0.360 365 656	
	Bai Ce Fai Ult	ain rate, %/min. ck Pressure, psi Il Pressure, psi I. Stress, psf Strain, % . Stress, psf Strain, %	1.00 0.000 6.870 1369.1 9.4 1327.4 15.1	1.00 0.000 20.350 914.8 8.9 885.4 14.9	41.860 1675.2 9.2 1613.6 15.2	
		Failure, psf Failure, psf	2358.4 989.3	3845.2 2930.4		

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-03 Depth: 14

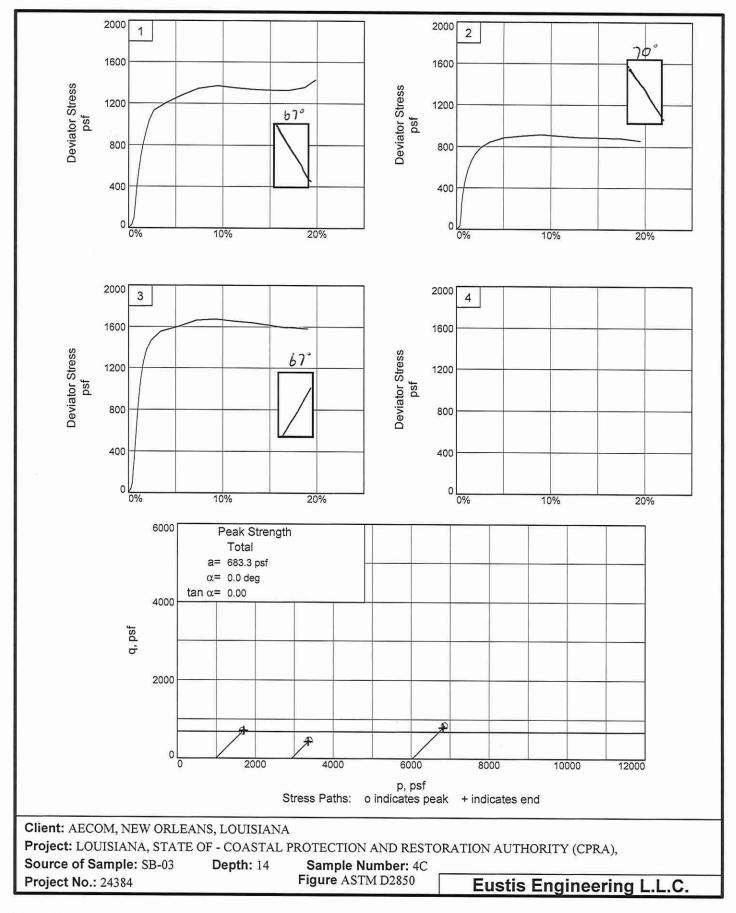
Sample Number: 4C

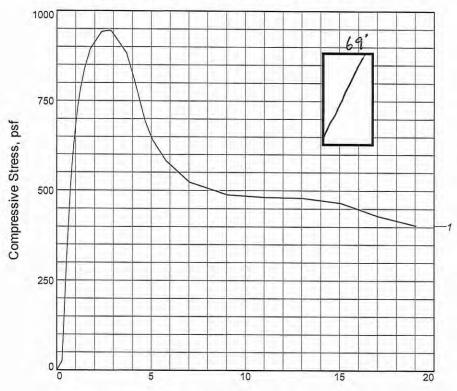
Proj. No.: 24384 Date Sampled: 10/20/20



Figure ASTM D2850

Tested By: MS





Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	946.0	
Undrained shear strength, psf	473.0	
Failure strain, %	2.8	
Strain rate, %/min.	1.00	
Water content, %	89.8	
Wet density, pcf	91.4	
Dry density, pcf	48.2	
Saturation, %	96.8	
Void ratio	2.5249	
Specimen diameter, in.	1,409	
Specimen height, in.	2.766	
Height/diameter ratio	1.96	

Description: SO GR CHOA

LL = 142 PL = 41 PI = 101 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/20/20

Remarks:

TORVANE = 0.375 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

Source of Sample: SB-03

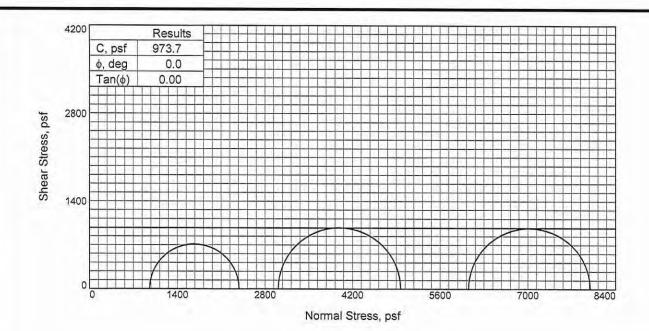
Depth: 18

Sample Number: 5C

EUSTIS ENGINEERING

Figure ASTM D2166

Tested By: DE



Water Content, %

Water Content, %

Dry Density, pcf

Saturation, %

Diameter, in.

Void Ratio

Height, in.

Strain, %

Strain, %

Ult. Stress, psf

σ<sub>1</sub> Failure, psf

σ<sub>3</sub> Failure, psf

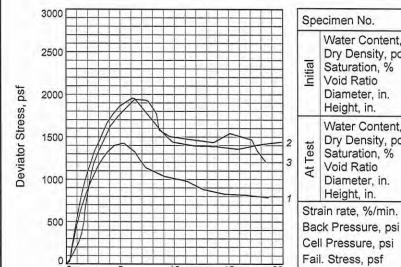
Dry Density, pcf

Saturation, %

Diameter, in.

Void Ratio

Height, in.



Axial Strain, %

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED Description: M GR & DGR CHOC

LL= 224 PL= 59 PI= 165

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.375 TSF

Client: AECOM, NEW ORLEANS, LOUISIAN	IA
--------------------------------------	----

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

1

135.7

35.6

98.3

3.7304

1.409

2.757

138.2

100.0

1.409

2.757

1.00

0.000

6.660

5.3

826.2

14.6

2385.1

959.0

1426.0

3.7304

35.6

2

142.1

34.7

99.6

3.8509

1.408

2.747

142.6

34.7

100.0

1.408

2.747

1.00

0.000

20.920

1956.6

1385.5

4969.1

3012.5

6.1

13.9

3.8509

3

131.5

37.0

99.9

3.5533

1.405

2.750

131.6

100.0

1.405

2.750

0.000

42.040

1940.5

1433.7

7994.2

6053.8

6.3

13.6

1.00

3.5533

37.0

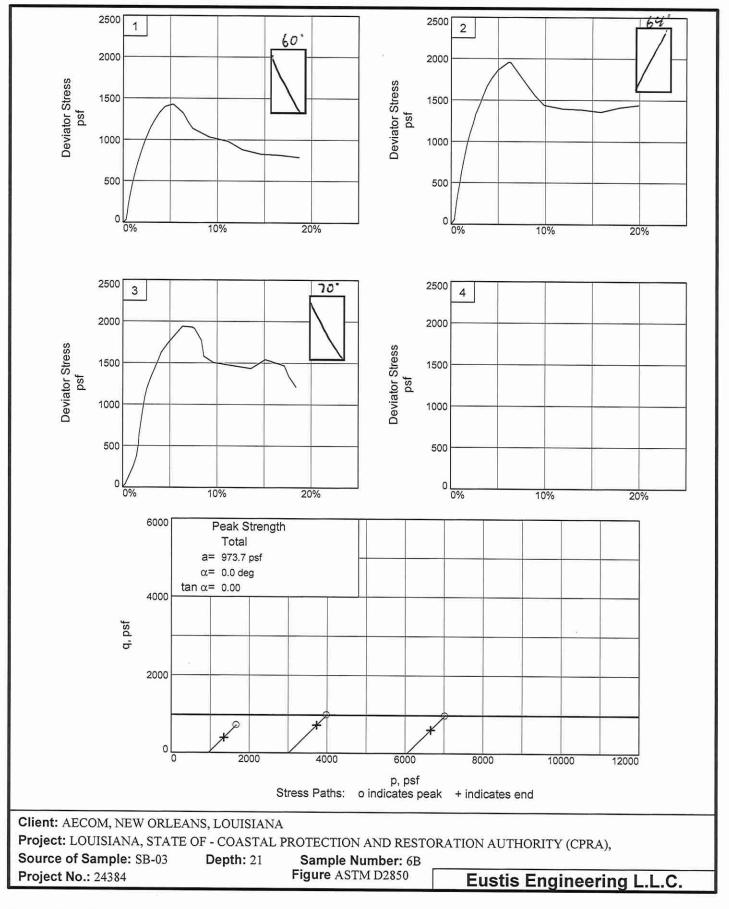
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 21

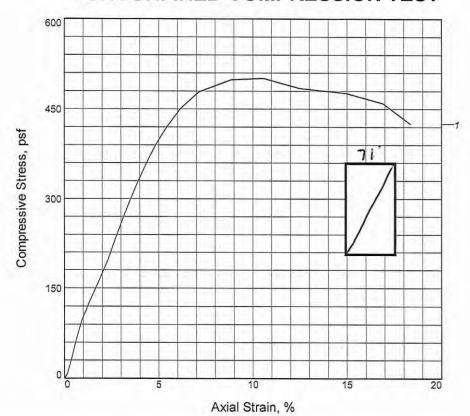
Sample Number: 6B

Proj. No.: 24384 Date Sampled: 10/20/20



Figure ASTM D2850





1	
501.3	
250.6	
10.6	
1.00	
36.3	
116.1	
85.2	
100.0	
0.9787	
1.390	
2.789	
2.01	
	250.6 10.6 1.00 36.3 116.1 85.2 100.0 0.9787 1.390 2.789

Description: SO GR CL4 W/ ARS SP, SIF

PI = 14 LL = 35PL = 21Assumed GS= 2.70 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/20/20

Remarks:

TORVANE = 0.200 TSF

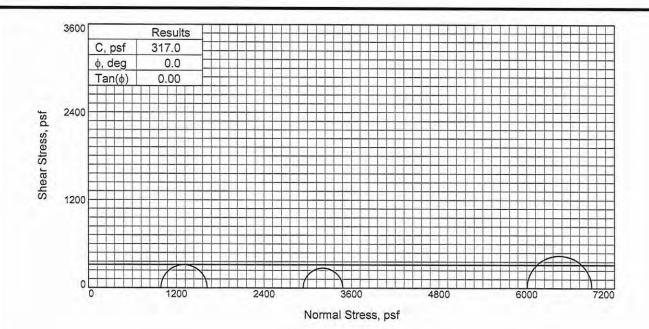
Client: AECOM, NEW ORLEANS, LOUISIANA

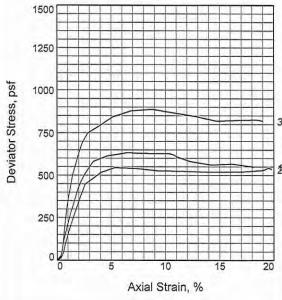
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 26

Sample Number: 7C

Figure ASTM D2166





Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED

Description: SO GR CH2 W/ ARS SP, SIF

LL= 50

PL= 18

PI= 32

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.150 TSF

Spe	ecimen No.	1	2	3
	Water Content, %	40.9	40.6	40.2
_	Dry Density, pcf	80.4	80.7	81.1
Initial	Saturation, %	100.0	100.0	100.0
드	Void Ratio	1.1129		1.0945
	Diameter, in.	1.392		1.401
	Height, in.	2.835	2.818	2.804
	Water Content, %	40.9	40.6	40.2
ts.	Dry Density, pcf	80.4	80.7	81.1
ě	Saturation, %	100.0	100.0	
At Test	Void Ratio	1.1129	1.1046	1.0945
-	Diameter, in.	1.392	1.403	1.401
	Height, in.	2.835	2.818	2.804
Str	ain rate, %/min.	1.00	1.00	1.00
Bad	ck Pressure, psi	0.000	0.000	0.000
Ce	l Pressure, psi	6.870	20.400	41.690
Fai	I. Stress, psf	633.1	544.4	886.7
5	Strain, %	6.4	5.3	8.8
Ult.	Stress, psf	560.3	519.6	818.6
5	Strain, %	14.2	15.1	14.7
σ1	Failure, psf	1622.4	3482.0	6890.0
$\sigma_3$	Failure, psf	989.3	2937.6	6003.4

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 31

Sample Number: 8D

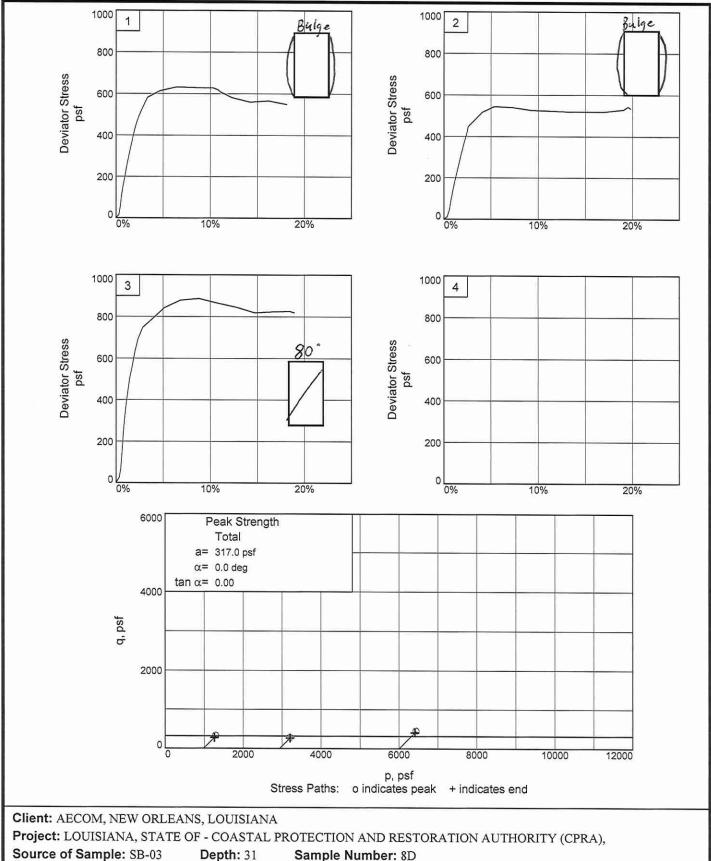
Proj. No.: 24384

Date Sampled: 10/20/20



Figure ASTM D2850

Tested By: MS



Source of Sample: SB-03

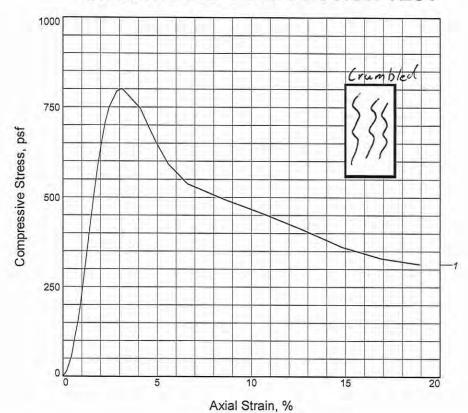
Sample Number: 8D

Figure ASTM D2850

**Eustis Engineering L.L.C.** 

Tested By: MS

Project No.: 24384



Specimen No.	1	
Unconfined strength, psf	800.3	
Undrained shear strength, psf	400.1	
Failure strain, %	3.1	
Strain rate, %/min.	1.00	
Water content, %	20.6	
Wet density, pcf	126.7	
Dry density, pcf	105.1	
Saturation, %	92.1	
Void ratio	0.6042	
Specimen diameter, in.	1.382	
Specimen height, in.	2.759	
Height/diameter ratio	2.00	

Description: SO GNG CL6

LL = 41PI = 28PL = 13 Assumed GS= 2.70 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/20/20

Remarks:

TORVANE = 0.150 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

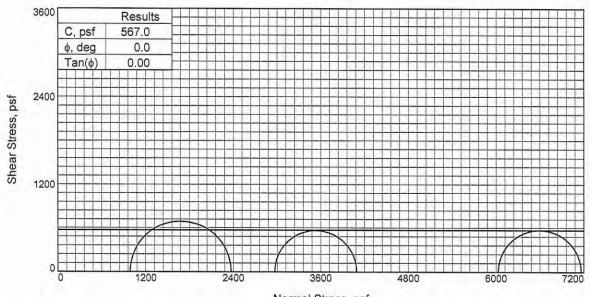
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03

Sample Number: 9C

Depth: 34

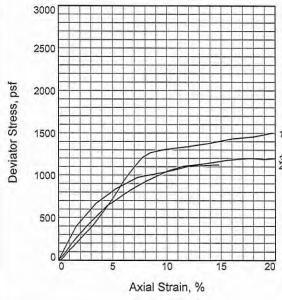
Figure ASTM D2166



Normal Stress, psf

Water Content, %

Specimen No.



		Dry Density, pcf	93.3	92.6	89.7	
	Initial	Saturation, %	96.6	99.8	100.0	
	Ξ	Void Ratio	0.8070	0.8193	0.8799	
4	11	Diameter, in.	1.441	1.392	1.406	
		Height, in.	2.696	2.791	2.766	
		Water Content, %	29.9	30.3	32.6	
1	±2	Dry Density, pcf	93.3	92.6	89.7	
	es	Saturation, %	100.0	100.0	100.0	
3	At Test	Void Ratio	0.8070	0.8193	0.8799	
	1	Diameter, in.	1.441	1.392	1.406	
		Height, in.	2.696	2.791	2.766	
	Str	ain rate, %/min.	1.00	1.00	1.00	
	Back Pressure, psi		0.000	0.000	0.000	
	Ce	Il Pressure, psi	6.860	20.610	41.860	
	Fail. Stress, psf		1381.3	1123.2	1136.2	
	Strain, % Ult. Stress, psf			14.8 1123.2	13.6	
					1136.2	
	5	Strain, %	14.0	14.8	13.6	
	$\sigma_1$	Failure, psf	2369.1	4091.0	7164.0	
	03	Failure, psf	987.8	2967.8	6027.8	

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: M T & GR CL4 W/ CC

LL= 31 PL= 18 PI= 13

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.150 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 42

Sample Number: 11C

Proj. No.: 24384

Date Sampled: 10/20/20

2

30.3

3

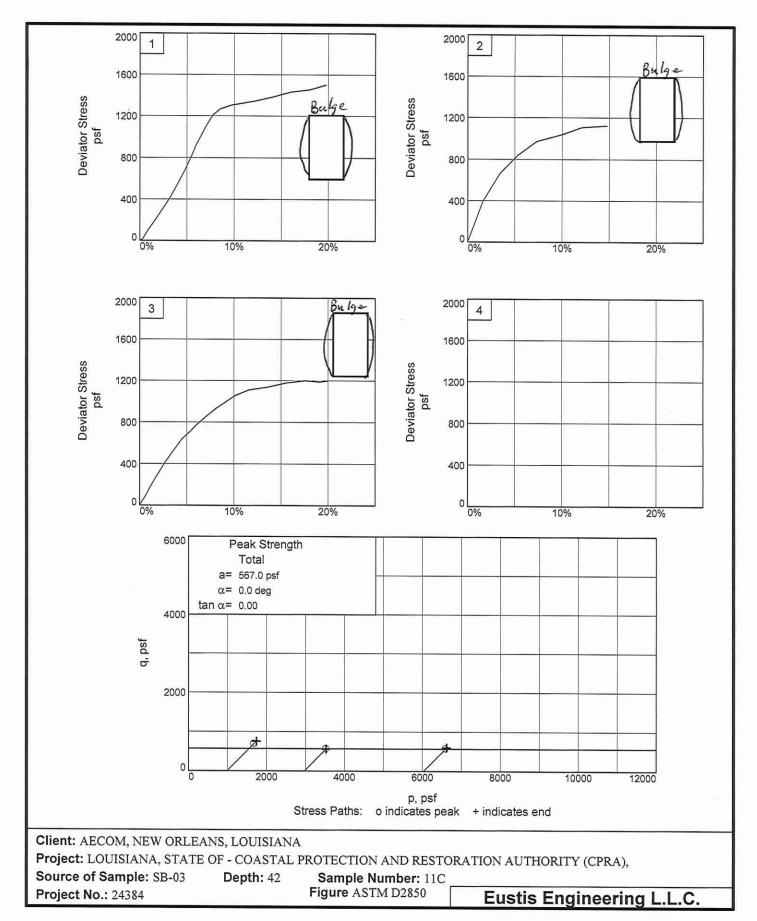
32.6

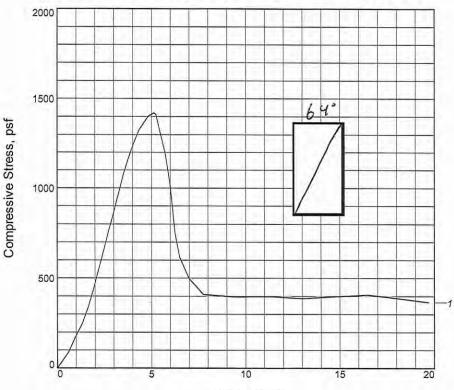
1

28.9



Figure ASTM D2850





Axial	Strain.	0/0
MAIGH	Ottaill,	10

Specimen No.	1	
Unconfined strength, psf	1419.9	
Undrained shear strength, psf	710.0	
Failure strain, %	5.1	
Strain rate, %/min.	1.00	
Water content, %	33.9	
Wet density, pcf	113.2	
Dry density, pcf	84.5	
Saturation, %	92.0	
Void ratio	0.9940	
Specimen diameter, in.	1.412	
Specimen height, in.	2.884	
Height/diameter ratio	2.04	

Description: M T & GR CL6

LL = 45 PL = 25 PI = 20Assumed GS= 2.70 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/20/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

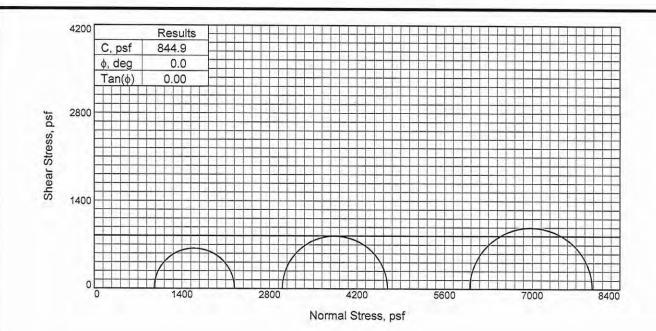
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

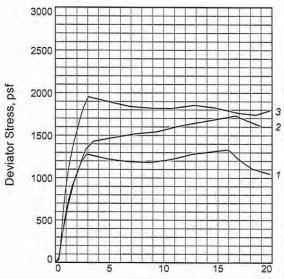
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 57

Sample Number: 15B

SINCE 1946

Figure ASTM D2166





Specimen No. 1 3 Water Content, % 43.8 44.7 42.6 Dry Density, pcf 76.9 75.0 77.0 Saturation, % 98.6 96.1 96.0 Void Ratio 1.2076 1.2653 1.2061 Diameter, in. 1.421 1.415 1.410 Height, in. 2.752 2.764 2.759 Water Content, % 44.4 46.5 44.3 Dry Density, pcf 76.9 75.0 77.0 100.0 Saturation, % 100.0 100.0 Void Ratio 1.2076 1.2653 1.2061 F Diameter, in. 1.421 1.415 1.410 2.764 Height, in. 2.752 2.759 Strain rate, %/min. 1.00 1.00 1.00 Back Pressure, psi 0.000 0.000 0.000 Cell Pressure, psi 6.650 20.860 41.690 Fail. Stress, psf 1281.0 1684.1 1954.3 Strain, % 2.9 14.6 3.0 Ult. Stress, psf 1184.2 1684.1 1819.4 Strain, % 8.9 14.6 8.9 σ<sub>1</sub> Failure, psf 2238.6 4688.0 7957.7 σ<sub>3</sub> Failure, psf 957.6 3003.8 6003.4

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED

Description: M GR & T CH3 W/ ARS & LNS SM

Axial Strain, %

LL= 69 PL= 22 PI= 47

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 61

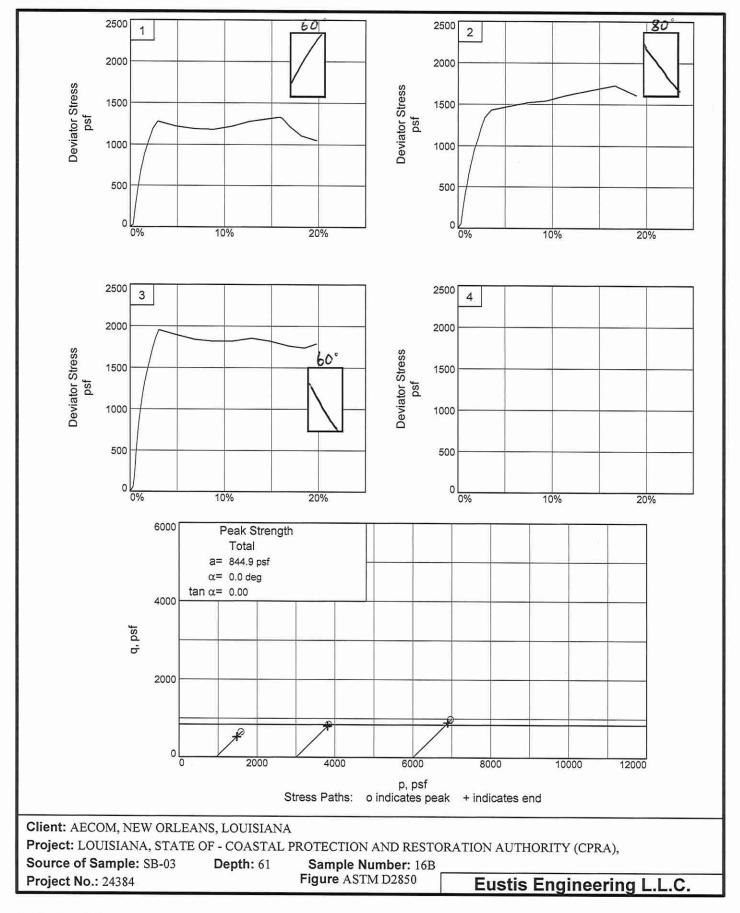
Sample Number: 16B

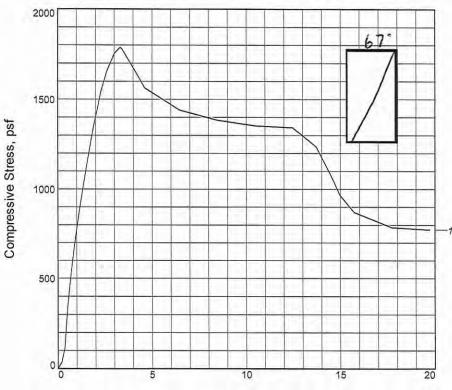
Proj. No.: 24384 Date Sampled: 10/20/20



Figure ASTM D2850

Tested By: MS





Avial	Strain.	0/2
Axiai	oliani,	/0

Specimen No.	1	
Unconfined strength, psf	1787.6	
Undrained shear strength, psf	893.8	
Failure strain, %	3.3	
Strain rate, %/min.	1.00	
Water content, %	35.0	
Wet density, pcf	115.4	
Dry density, pcf	85.5	
Saturation, %	96.5	
Void ratio	0.9868	
Specimen diameter, in.	1.401	
Specimen height, in.	2.759	
Height/diameter ratio	1.97	

Description: M GR & T CH3 W/ ARS & LNS ML, CC

LL = 51 PL = 15 PI = 36 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/20/20

Date Sampled. 10/20/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

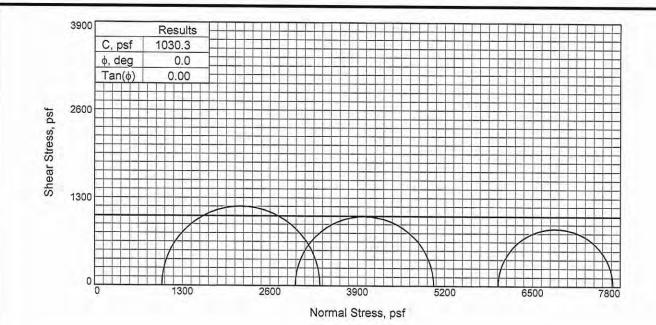
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

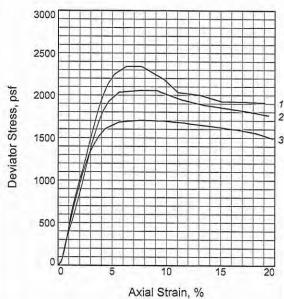
RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-03 Depth: 66

Sample Number: 17C

EUSTIS ENGINEERING

Figure ASTM D2166





Sp	ecimen No.	1	2	3	
ial	Water Content, % Dry Density, pcf Saturation, %	29.8 91.8 95.4	28.7 94.2 97.4	29.6 93.5 98.7	
Initial	Void Ratio Diameter, in. Height, in.	0.8505 1.415 2.757	0.8017 1.413 2.742	0.8167 1.402 2.763	
At Test	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	31.3 91.8 100.0 0.8505 1.415 2.757	29.5 94.2 100.0 0.8017 1.413 2.742	30.0 93.5 100.0 0.8167 1.402 2.763	
Str	ain rate, %/min.	1.00	1.00	1.00	
Ba	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	6.930	20.680	41.590	
	il. Stress, psf Strain, %	2346.1 7.6	2055.7 7.6	1703.7 7.6	
	. Stress, psf Strain, %	1925.0 15.1	1854.1 15.0	1635.5 14.1	
σ,	Failure, psf Failure, psf	3344.0 997.9	5033.6 2977.9		

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: ST GR CL6 W/ ARS SM

LL= 40 PL= 15 PI= 25

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.625 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 70

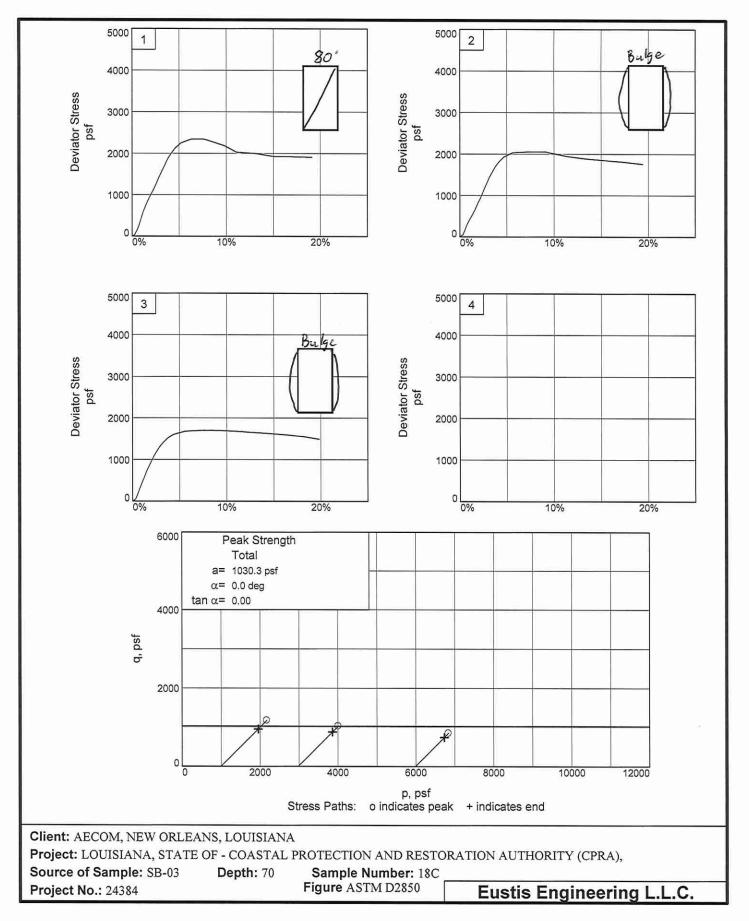
Sample Number: 18C

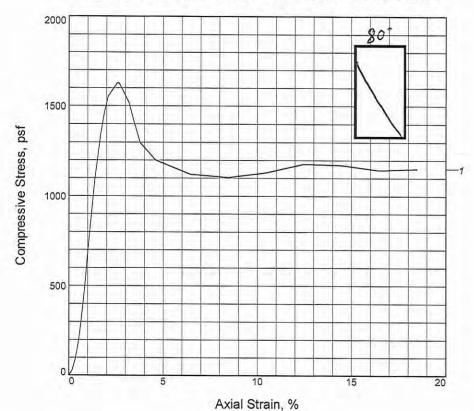
Proj. No.: 24384 Date Sampled: 10/20/20



Figure ASTM D2850

Tested By: MS





Specimen No.	1	
Unconfined strength, psf	1628.6	
Undrained shear strength, psf	814.3	
Failure strain, %	2.6	
Strain rate, %/min.	1.00	
Water content, %	38.1	
Wet density, pcf	112.2	
Dry density, pcf	81.2	
Saturation, %	95.0	
Void ratio	1.0901	
Specimen diameter, in.	1.418	
Specimen height, in.	2.753	
Height/diameter ratio	1.94	

Description: M GR CL6 W/ ARS SM

LL = 44 PL = 14 PI = 30Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/20/20

Remarks:

TORVANE = 0.700 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

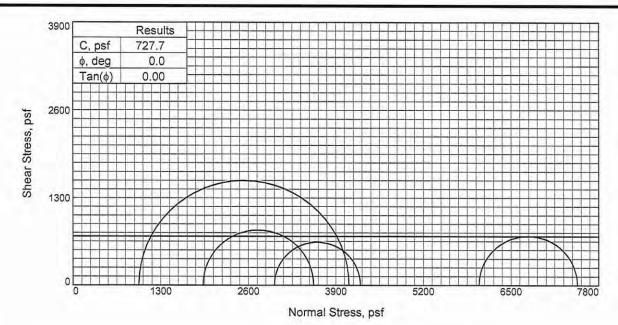
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03

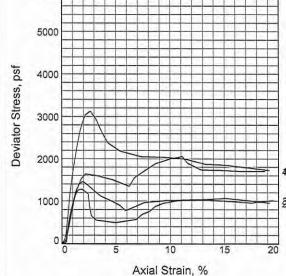
Sample Number: 19C

Depth: 74

Figure ASTM D2166



6000 Specimen No.



-			-	No Wall
Typ	0	OT.	1	act.
. 7 1	•	O.		COL.

Unconsolidated Undrained
Sample Type: UNDISTURBED

Description: ST GR CH3 W/ ARS ML, SL

LL= 72 PL= 23 PI= 49

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.700 TSF

Sp	ecimen No.	1	2	3	4
Initial	Water Content, % Dry Density, pcf Saturation, %	43.3 77.6 99.2	43.2 76.4 96.2	44.5 76.1 98.4	43.5 77.3 98.8
Ξ	Void Ratio Diameter, in. Height, in.	1.1873 1.414 2.804	1.2215 1.406 2.759	1.2302 1.418 2.785	1.1967 1.405 2.805
	Water Content, % Dry Density, pcf	43.7 77.6	44.9 76.4	45.2 76.1	44.0 77.3
At Test	Saturation, % Void Ratio	100.0 1.1873	100.0 1.2215	100.0 1.2302	100.0 1.1967
	Diameter, in. Height, in.	1.414 2.804	1.406 2.759	1.418 2.785	1.405 2.805
Str	ain rate, %/min.	1.00	1.00	1.00	1.00
Ва	ck Pressure, psi	0.000	0.000	0.000	0.000
Ce	Il Pressure, psi	6.780	20.750	41.840	13.390
Fai	I. Stress, psf	3118.2	1276.4	1459.1	1640.5
5	Strain, %	2.6	1.8	1.9	2.2
Ult	. Stress, psf	1849.4	494.5	775.3	1345.4
5	Strain, %	15.0	5.0	5.9	6.2
σ1	Failure, psf	4094.6	4264.4	7484.1	3568.7
$\sigma_3$	Failure, psf	976.3	2988.0	6025.0	1928.2

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-03 Depth: 81

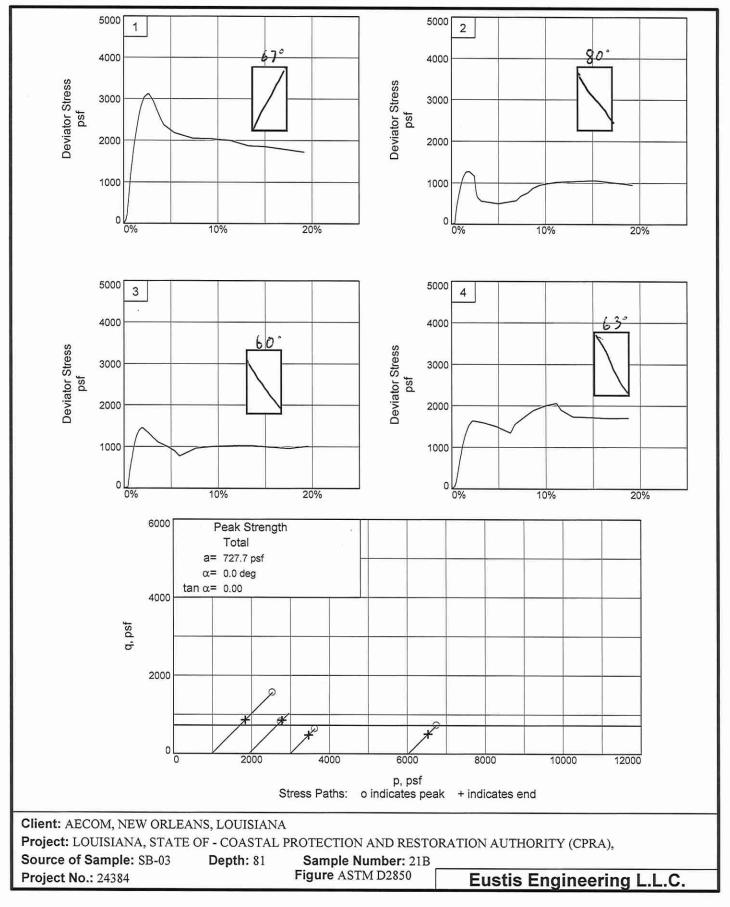
Sample Number: 21B

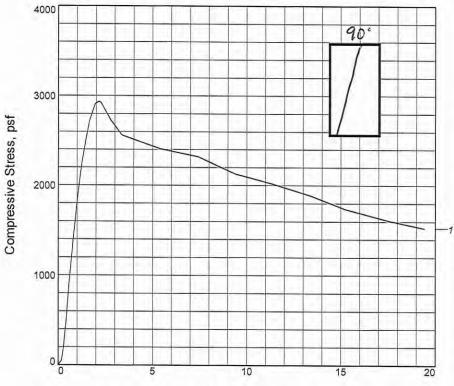
Proj. No.: 24384

Date Sampled: 10/20/20



Figure ASTM D2850





A	O	07
Axial	Strain,	%

Specimen No.	1	
Unconfined strength, psf	2935.5	
Undrained shear strength, psf	1467.7	
Failure strain, %	2.2	
Strain rate, %/min.	1.00	
Water content, %	42.7	
Wet density, pcf	110.6	
Dry density, pcf	77.5	
Saturation, %	97.6	
Void ratio	1.1899	
Specimen diameter, in.	1.413	
Specimen height, in.	2.769	
Height/diameter ratio	1.96	

Description: ST GR CH4 W/ ARS ML, WD

LL = 74PL = 18PI = 56Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/20/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03

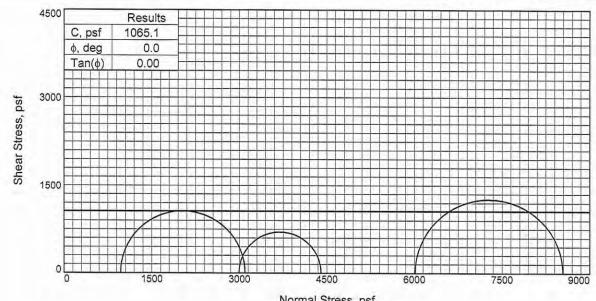
Depth: 86

Sample Number: 22C

SINCE 1946

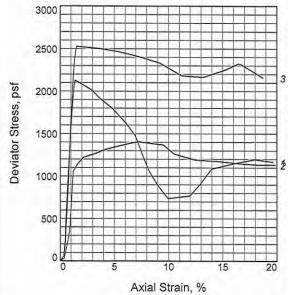
Figure ASTM D2166

Tested By: DE



Normal Stress, psf

Specimen No.



	Water Content, %	45.0	44.6	45.1	
-	Dry Density, pcf	75.3	75.7	74.4	
Initial	Saturation, %	97.6	97.5	95.7	
=		1.2551	1.2428	1.2830	
	Diameter, in.	1.401	1.392	1.419	
	Height, in.	2.753	2.761	2.750	
	Water Content, %	46.1	45.7	47.2	
+×	Dry Density, pcf	75.3	75.7	74.4	
At Test	Saturation, %	100.0	100.0	100.0	
7	Void Ratio	1.2551	1.2428	1.2830	
	Diameter, in.	1.401	1.392	1.419	
	Height, in.	2.753	2.761	2.750	
St	rain rate, %/min.	1.00	0.20	1.00	
Ва	ick Pressure, psi	0.000	0.000	0.000	
Ce	ell Pressure, psi	6.720	20.790	41.750	
Fa	il. Stress, psf	2128.6	1405.8	2528.5	
	Strain, %	1.4	7.3	1.5	
Ult	t. Stress, psf	738.0	1176.7	2167.0	
	Strain, %	10.0	14.5	13.2	
<b>σ</b> <sub>1</sub>	Failure, psf	3096.3	4399.6	8540.5	
$\sigma_3$	Failure, psf	967.7	2993.8	6012.0	

1

2

3

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: ST GR CH4 W/ ARS ML, SL

LL= 84

PL= 22

PI= 62

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.625 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 90

Sample Number: 23C

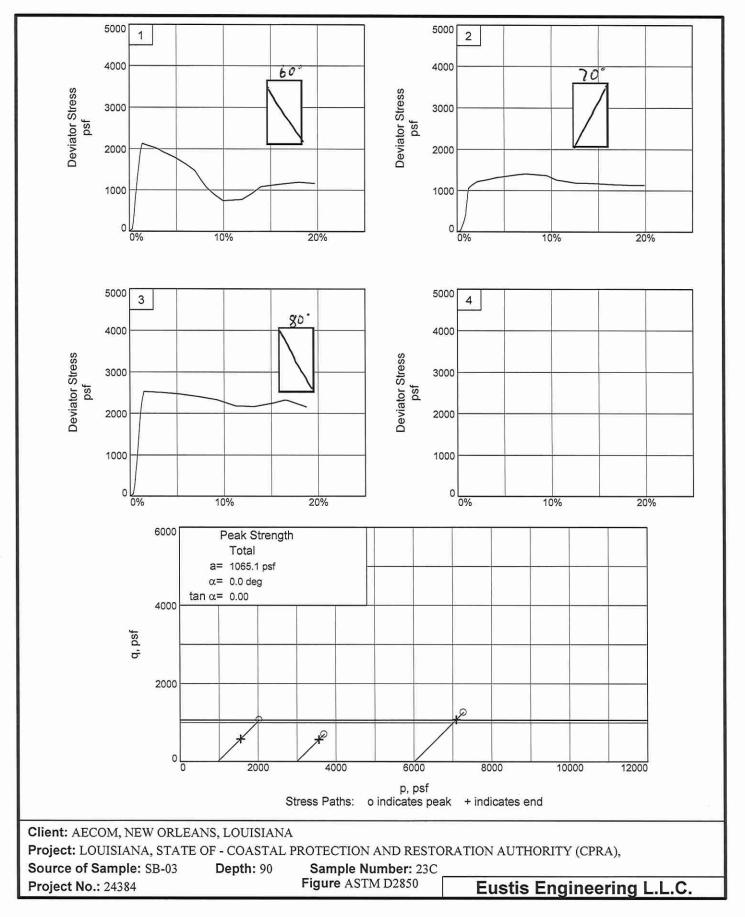
Proj. No.: 24384

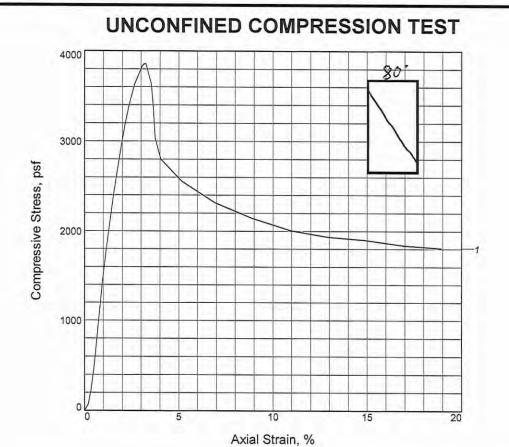
Date Sampled: 10/20/20



Figure ASTM D2850

Tested By: MS





Specimen No.	1	
Unconfined strength, psf	3858.5	
Undrained shear strength, psf	1929.3	
Failure strain, %	3.2	
Strain rate, %/min.	1.00	
Water content, %	47.3	
Wet density, pcf	107.7	
Dry density, pcf	73.1	
Saturation, %	97.2	
Void ratio	1.3230	
Specimen diameter, in.	1.404	
Specimen height, in.	2.769	
Height/diameter ratio	1.97	

Description: ST GR CH4 W/ ARS ML, SL

LL = 87 PL = 25 PI = 62 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/20/20

Date Sampled. 10/20

Remarks: TORVANE = 0.625 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

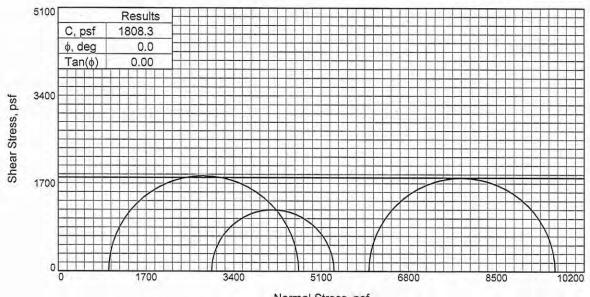
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 94

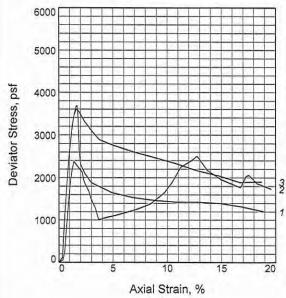
Sample Number: 24C

EUSTIS ENGINEERING

Figure ASTM D2166



Normal Stress, psf



_		- 1
Type	Ot.	Loct.

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: ST GR CH4 W/ ARS & LNS ML, SL

LL= 91

PL= 24

PI= 67

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.625 TSF

Sp	ecimen No.	1	2	3	
	Water Content, % Dry Density, pcf	47.1 73.0	46.9 73.0	46.1 74.2	
<u>a</u>	Saturation, %	96.4	96.3	97.4	
Initial	Void Ratio	1.3275	1.3247	1.2871	
	Diameter, in.	1.413	1.422	1.411	
	Height, in.	2.759	2.760	2.781	
	Water Content, %	48.8	48.7	47.3	
+2	Dry Density, pcf	73.0	73.0	74.2	
At Test	Saturation, %	100.0	100.0	100.0	
7	Void Ratio	1.3275	1.3247	1.2871	
	Diameter, in.	1.413	1.422	1.411	
	Height, in.	2.759	2.760	2.781	
Str	ain rate, %/min.	1.00	1.00	1.00	
Ba	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	6.780	20.640	41.850	
Fa	il. Stress, psf	3685.3	2377.1	3610.1	
1	Strain, %	1.7	1.4	1.6	
Ult	. Stress, psf	1400.4	1011.4	2047.8	
;	Strain, %	14.9	3.7	14.7	
σ <sub>1</sub>	Failure, psf	4661.6	5349.3	9636.5	
$\sigma_3$	Failure, psf	976.3	2972.2	6026.4	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 98

Sample Number: 25C

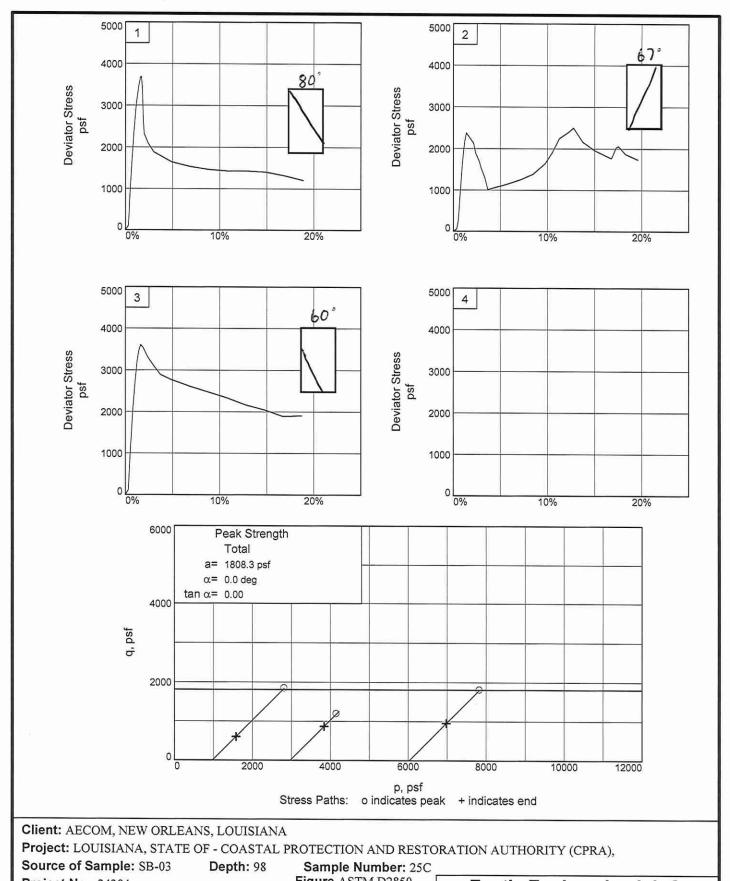
Proj. No.: 24384

Date Sampled: 10/20/20



Figure ASTM D2850

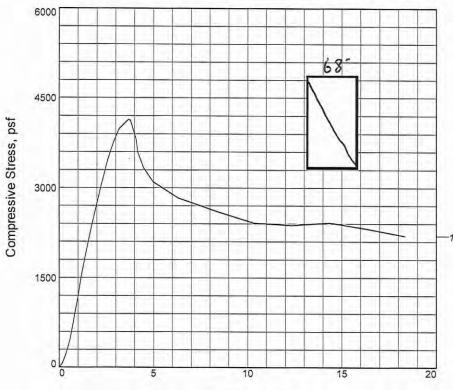
Tested By: MS



Project No.: 24384 Figure ASTM D2850 Eustis Engineering L.L.C.

Checked By: RR

Tested By: MS



Avia	Strain.	0/
Axia	Suaii.	70

Specimen No.	1	
Unconfined strength, psf	4138.4	
Undrained shear strength, psf	2069.2	
Failure strain, %	3.7	
Strain rate, %/min.	1.00	
Water content, %	44.3	
Wet density, pcf	108.7	
Dry density, pcf	75.3	
Saturation, %	96.1	
Void ratio	1.2542	
Specimen diameter, in.	1.411	
Specimen height, in.	2.767	
Height/diameter ratio	1.96	

Description: VST GR CH4 W/ ARS & LNS ML

LL = 87 PL = 22 PI = 65 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/20/20

Remarks:

TORVANE = 0.750 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

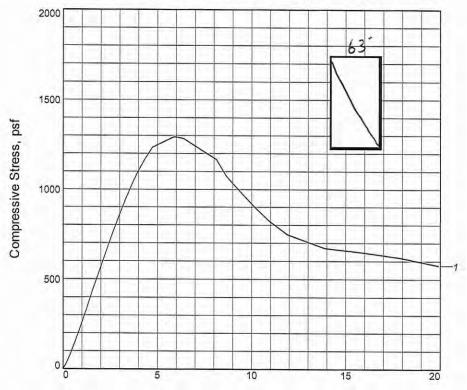
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-03 Depth: 102

Sample Number: 26C

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Figure ASTM D2166



Axial Strain, %

1	
1292.0	
646.0	
5.9	
1.00	
21.6	
128.3	
105.5	
97.6	
0.5981	
1.407	
2.749	
1.95	
	646.0 5.9 1.00 21.6 128.3 105.5 97.6 0.5981 1.407 2.749

Description: M GNG CL4 W/ ARS SP

LL = 28 PL = 16 PI = 12 Assumed GS = 2.70 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/20/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

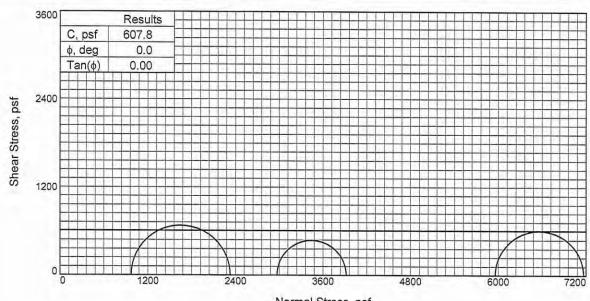
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 106

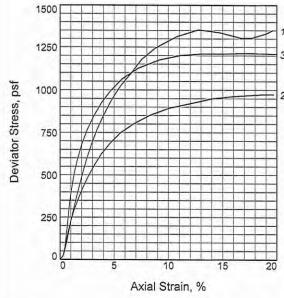
Sample Number: 27C

EUSTIS ENGINEERING

Figure ASTM D2166



Normal Stress, psf



	Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED

Description: M GNG CL4 W/ ASR SP, O

LL= 32

PL= 13

PI= 19

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.375 TSF

Sp	ecimen No.	1	2	3	
Initial	Water Content, % Dry Density, pcf Saturation, %	27.5 96.7 100.0	27.2 97.2 100.0	27.6 96.6 100.0	
<u>:</u>	Void Ratio Diameter, in. Height, in.	0.7424 1.405 2.789	0.7338 1.405 2.775	0.7447 1.404 2.773	
At Test	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	27.5 96.7 100.0 0.7424 1.405 2.789	27.2 97.2 100.0 0.7338 1.405 2.775	27.6 96.6 100.0 0.7447 1.404 2.773	
	ain rate, %/min.	1.00	1.00	1.00	
	ck Pressure, psi	0.000	0.000	0.000	
Fai	ll Pressure, psi I. Stress, psf Strain, %	6.760 1353.8 12.8	20.620 949.4 14.1	41.370 1211.0 13.2	
Ult.	Stress, psf Strain, %	1336.6 14.7	949.4 14.1	1210.5 15.2	
σ1		2327.2 973.4	3918.7 2969.3	7168.3	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-03 Depth: 113

Sample Number: 29B

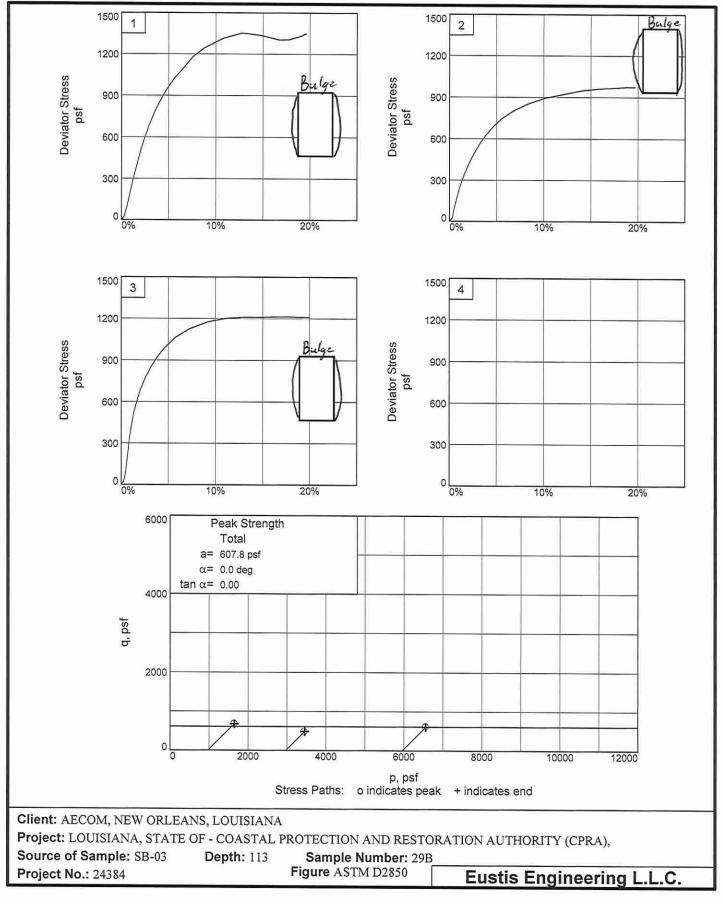
Proj. No.: 24384

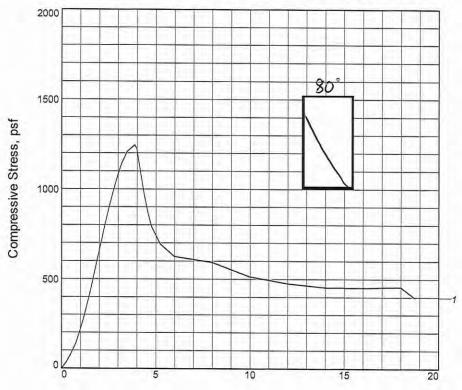
Date Sampled: 10/20/20



Figure ASTM D2850

Tested By: MS





Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	1245.1	
Undrained shear strength, psf	622.6	
Failure strain, %	3.9	
Strain rate, %/min.	1.00	
Water content, %	30.8	
Wet density, pcf	117.3	
Dry density, pcf	89.7	
Saturation, %	94.6	
Void ratio	0.8787	
Specimen diameter, in.	1.401	
Specimen height, in.	2.802	
Height/diameter ratio	2.00	

Description: M GR & T CL4

LL = 30 PL = 14 PI = 16 Assumed GS = 2.70 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/20/20

Date Sampled: 10/20/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

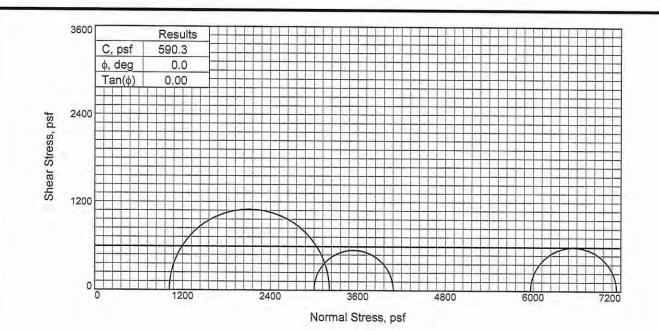
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

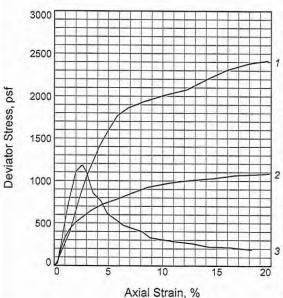
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-03 Depth: 118

Sample Number: 30C

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Figure ASTM D2166





Specimen No. 3 Water Content, % 33.1 34.1 32.7 Dry Density, pcf 88.1 86.9 88.9 Saturation, % 99.1 99.0 99.7 Void Ratio 0.89200.9187 0.8747 Diameter, in. 1.425 1.415 1.409 Height, in. 2.862 2.727 2.830 Water Content, % 33.4 34.4 32.8 Dry Density, pcf 88.1 86.9 88.9 Saturation, % 100.0 100.0 100.0 Void Ratio 0.8920 0.9187 0.8747 Diameter, in. 1.425 1.415 1.409 2.727 Height, in. 2.862 2.830 Strain rate, %/min. 1.00 1.00 1.00 Back Pressure, psi 0.000 0.000 0.000 Cell Pressure, psi 7.080 20.840 41.410 Fail. Stress, psf 2193.0 1086.0 1179.4 Strain, % 14.0 19.8 2.6 Ult. Stress, psf 2193.0 221.0 Strain, % 14.0 14.3 σ<sub>1</sub> Failure, psf 3212.5 4086.9 7142.5 σ<sub>3</sub> Failure, psf 1019.5 3001.0 5963.0

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: M GNG & T CL4

LL= 28

PL= 15

PI= 13

Assumed Specific Gravity= 2.67 Remarks: TORVANE = 0.375 TSF

Sample Number: 32B Proj. No.: 24384

Source of Sample: SB-03

Date Sampled: 11/3/20

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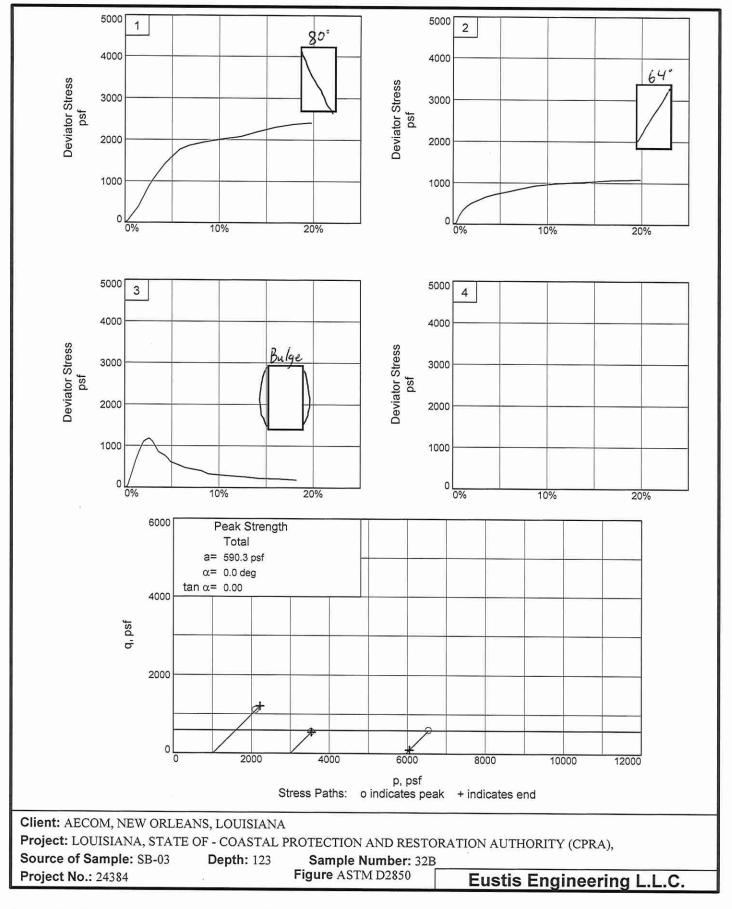
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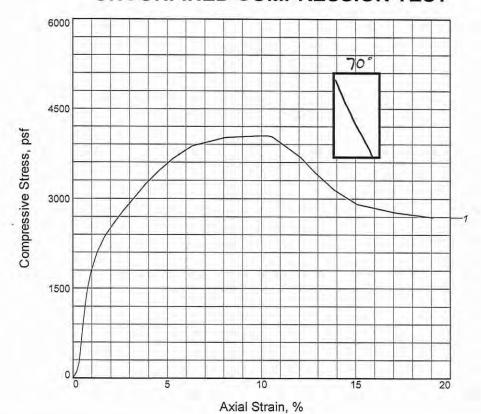
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

Client: AECOM, NEW ORLEANS, LOUISIANA

RESTORATION AUTHORITY (CPRA),

Figure ASTM D2850





Specimen No.	1	
Unconfined strength, psf	4046.9	
Undrained shear strength, psf	2023.5	
Failure strain, %	10.1	
Strain rate, %/min.	1.00	
Water content, %	33.8	
Wet density, pcf	113.7	
Dry density, pcf	85.0	
Saturation, %	92.0	
Void ratio	0.9984	
Specimen diameter, in.	1.397	
Specimen height, in.	2.765	
Height/diameter ratio	1.98	

Description: VST GR & T CH3 W/ ARS ML, WD

LL = 76 PI = 45 PL = 31 Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/7/20

Remarks:

TORVANE = 0.750 TSF

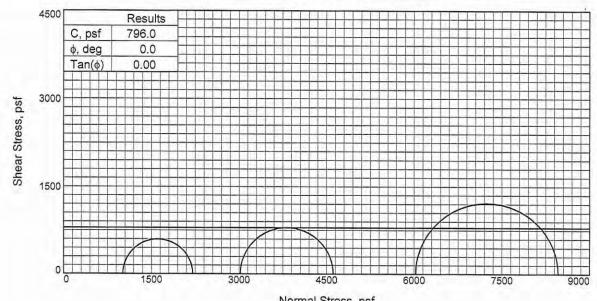
Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

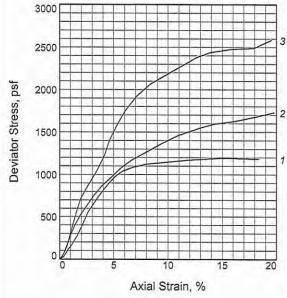
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-04 Depth: 5

Sample Number: 2B

Figure ASTM D2166



Normal Stress, psf



Sp	ecimen No.	1	2	3	
Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	30.3 90.3 94.5 0.8664 1.412 2.783	0.9241		
AtTest	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	32.1 90.3 100.0 0.8664 1.412 2.783	100.0 0.9241	34.9 86.8 100.0 0.9425 1.431 2.753	
Ba Ce Fai Ult	ain rate, %/min. ck Pressure, psi Il Pressure, psi Il. Stress, psf Strain, % . Stress, psf Strain, %	1.00 0.000 7.040 1192.8 15.0 1192.8 15.0	1.00 0.000 20.980 1592.5 14.2 1592.5 14.2	2437.0 13.9	
σ <sub>1</sub> Failure, psf σ <sub>3</sub> Failure, psf		2206.5 1013.8			

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: M GR & T CL4 W/ CC

LL= 34

PL= 16

PI= 18

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.200 TSF

Sample Number: 4B

Source of Sample: SB-04

Client: AECOM, NEW ORLEANS, LOUISIANA

RESTORATION AUTHORITY (CPRA),

Proj. No.: 24384

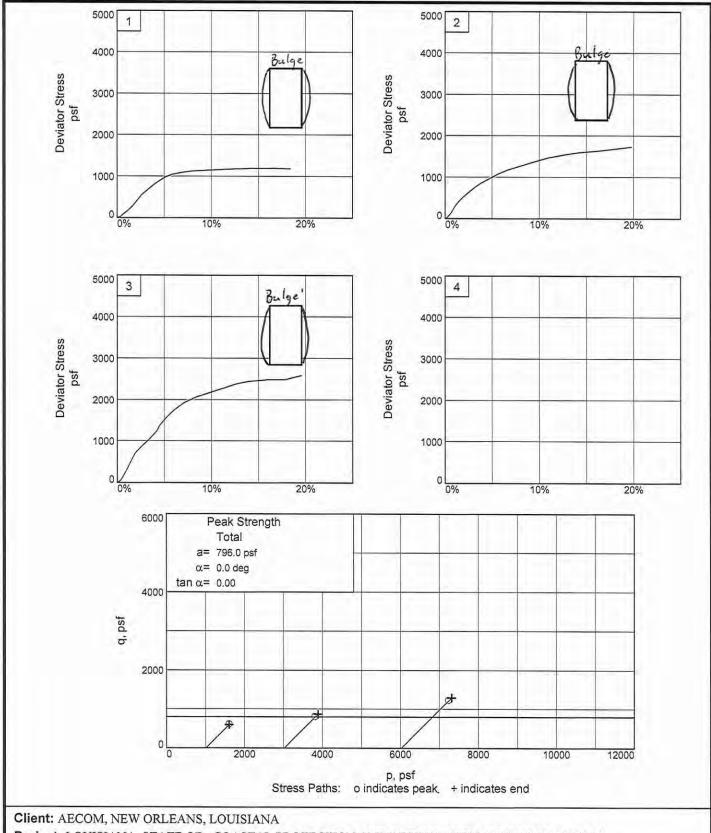
Date Sampled: 10/7/20



Depth: 13

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

Figure ASTM D2850



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA),

Source of Sample: SB-04

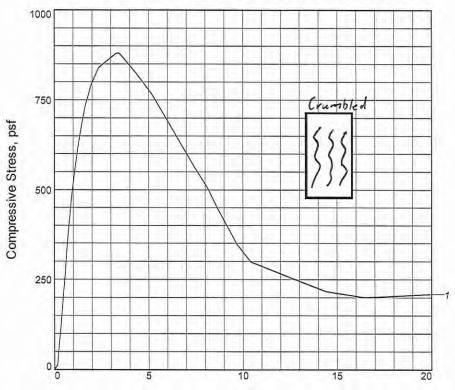
Project No.: 24384

Depth: 13

Sample Number: 4B Figure ASTM D2850

Eustis Engineering L.L.C.

Tested By: DE



Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	880.5	
Undrained shear strength, psf	440.3	
Failure strain, %	3.4	
Strain rate, %/min.	1.00	
Water content, %	70.3	
Wet density, pcf	95.1	
Dry density, pcf	55.9	
Saturation, %	93.7	
Void ratio	2.0390	
Specimen diameter, in.	1.413	
Specimen height, in.	2.777	
Height/diameter ratio	1.97	

Description: SO GR CHOA W/RT

LL = 128 PL = 35 PI = 93 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/7/20

Remarks:

TORVANE = 0.200 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

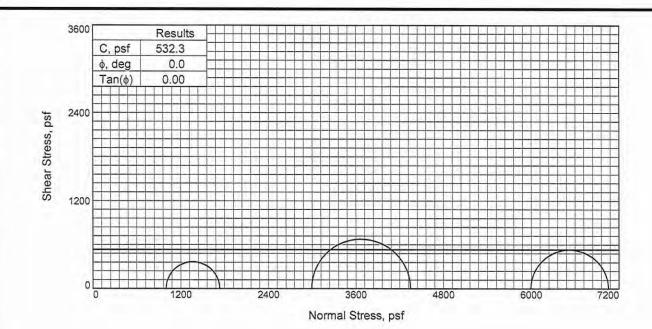
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

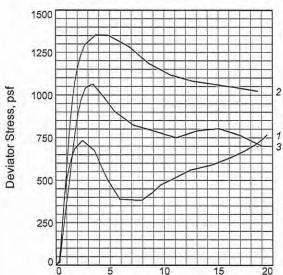
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-04 Depth: 18

Sample Number: 5C

EUSTIS

Figure ASTM D2166





Axial Strain, %

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: M GR & BR CHOB

LL= 167 PL= 47 PI= 120

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.150 TSF

Figure	ASTM	D2850
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Sp	ecimen No.	1	2	3	
Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	101.6 43.2 94.7 2.8974 1.390 2.760	2.8999	1.402	
AtTest	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	107.3 43.2 100.0 2.8974 1.390 2.760	2.8999 1.402	44.8 100.0 2.7663	
Ba Ce Fa Ult	ain rate, %/min. ck Pressure, psi Il Pressure, psi il. Stress, psf Strain, % . Stress, psf Strain, %	1.00 0.000 6.940 733.0 2.5 382.2 7.9	20.780 1355.0 3.7 1062.7 14.6	0.000	
	Failure, psf Failure, psf	1732.4 999.4			

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

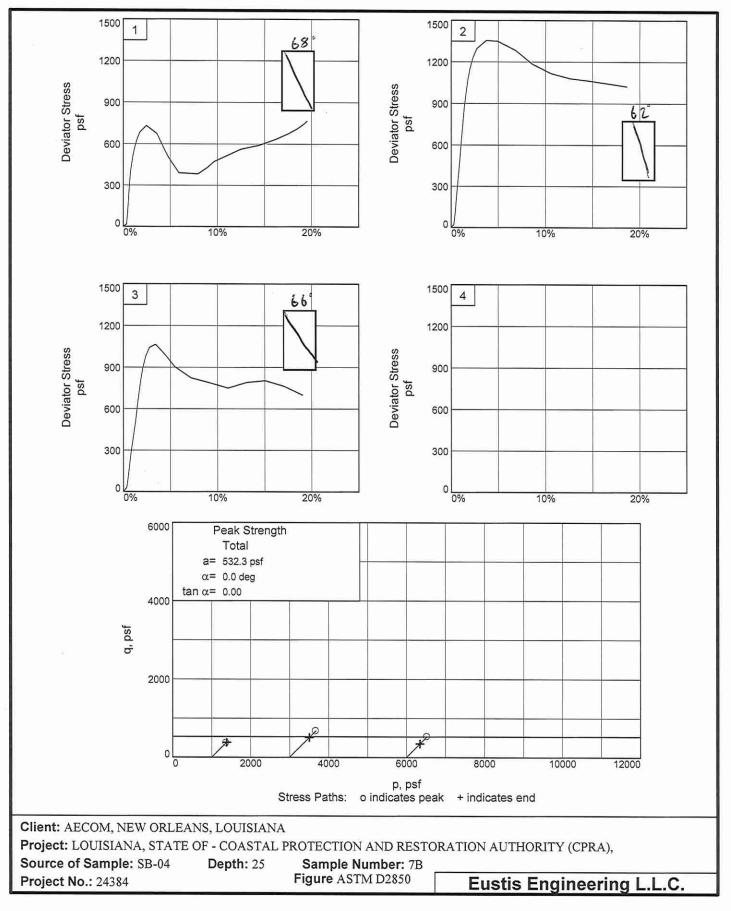
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-04 Depth: 25

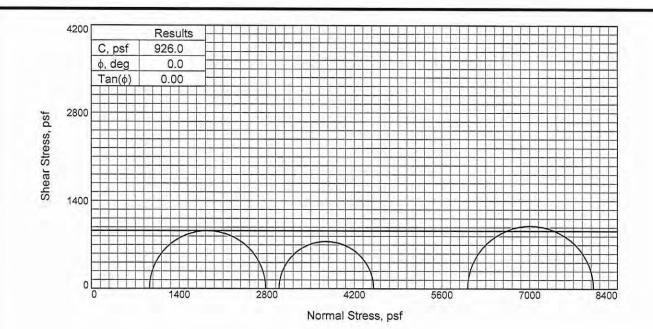
Sample Number: 7B

Proj. No.: 24384 Date Sampled: 10/7/20

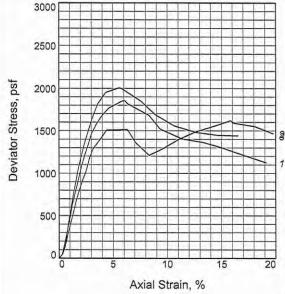


Tested By: CC





Specimen No.



- 4						
	Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	124.8 36.4 93.4 3.5414 1.404 2.760		3.8011	
<b>3</b> 8	AtTest	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	133.6 36.4 100.0 3.5414 1.404 2.760	139.7 35.2 100.0 3.7030 1.402 2.762	143.4 34.5 100.0 3.8011 1.388 2.765	
	Bai Ce Fai Ult	ain rate, %/min. ck Pressure, psi Il Pressure, psi il. Stress, psf Strain, % . Stress, psf Strain, %	1.00 0.000 6.430 1855.3 6.1 1300.9 15.0	0.000 20.800	2007.9 5.6 1446.5	
j	$\sigma_1$ $\sigma_3$	Failure, psf Failure, psf	2781.2 925.9	4508.5 2995.2	775 10 10 70	

Client: AECOM, NEW ORLEANS, LOUISIANA

RESTORATION AUTHORITY (CPRA),

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: M BR & GR CHOC

LL= 195

PL= 62

PI= 133

Assumed Specific Gravity= 2.65 Remarks: TORVANE = 0.200 TSF

Sample Number: 7C

Source of Sample: SB-04

Proj. No.: 24384

Date Sampled: 10/7/20

2

3

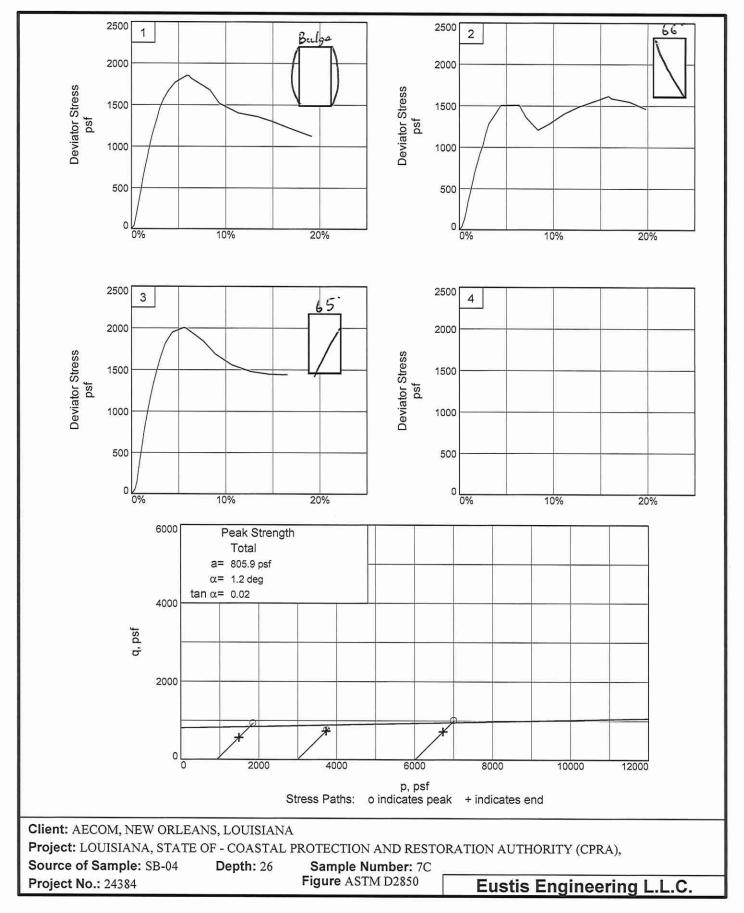


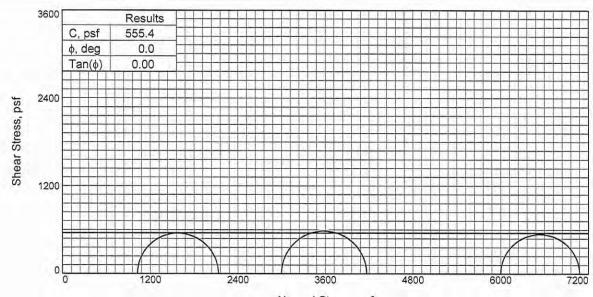
Depth: 26

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

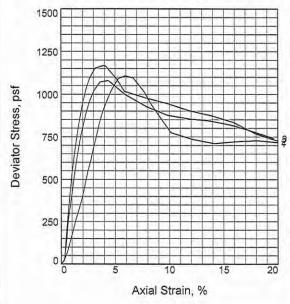
Figure ASTM D2850

Tested By: CC





Normal Stress, psf



_				_		2
1.	In	0	of		20	••
4.3	w	C	O.		53	٠.

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: M GR CH4 W/ ARS & LNS ML, RT

LL= 99

PL= 29

PI= 70

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.200 TSF

Spe	ecimen No.	1	2	3	
	Water Content, %	69.6	73.8	75.8	
	Dry Density, pcf	58.5	55.9	55.4	
Initial	Saturation, %	99.5		99.8	
ī	Void Ratio	1.9014	2.0387	2.0675	
	Diameter, in.	1.388			
١.,	Height, in.	2.778	2.746	2.815	
	Water Content, %	69.9	75.0	76.0	-
+	Dry Density, pcf	58.5	55.9	55.4	
At Test	Saturation, %	100.0	100.0	100.0	
1	Void Ratio	1.9014	2.0387	2.0675	
1	Diameter, in.	1.388	1.401	1.401	
	Height, in.	2.778	2.746	2.815	
Str	ain rate, %/min.	1.00	1.00	1.00	
Bad	ck Pressure, psi	0.000	0.000	0.000	
Cel	l Pressure, psi	7.120	20.810	41.660	
Fai	I. Stress, psf	1106.8	1168.0	1079.5	
5	Strain, %	5.9	3.9	4.3	
Ult.	Stress, psf	710.0	872.7	842.8	
5	Strain, %	14.1	13.9	13.8	
<b>0</b> 1	Failure, psf	2132.1	4164.6	7078.5	
$\sigma_3$	Failure, psf	1025.3	2996.6	5999.0	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-04 Depth: 30

Sample Number: 8C

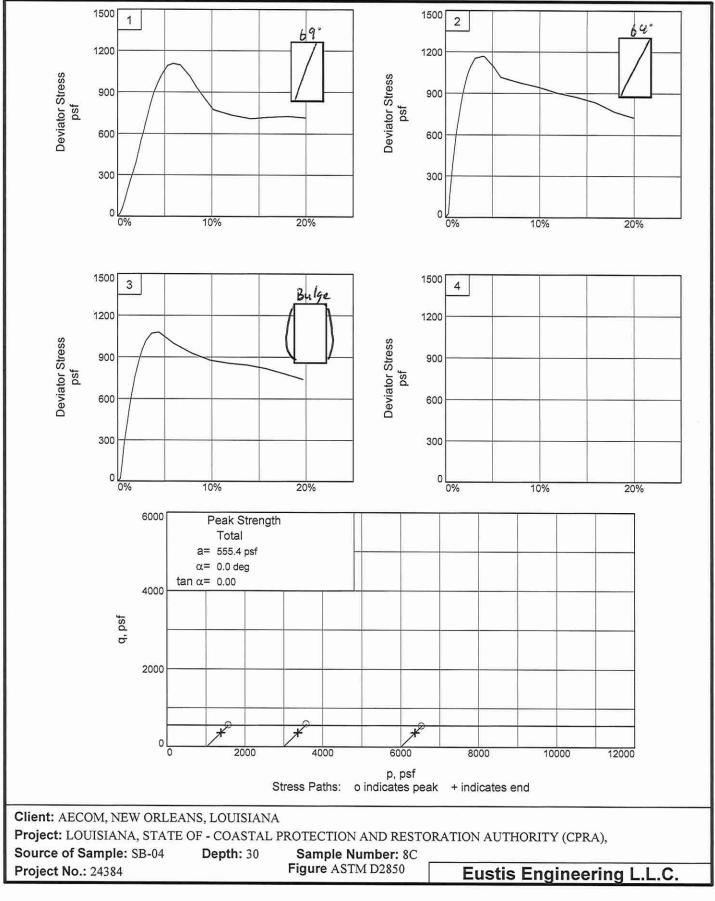
Proj. No.: 24384

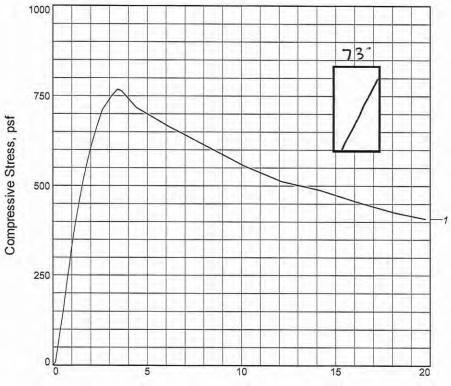
Date Sampled: 10/7/20



Figure ASTM D2850

Tested By: CC





Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	768.2	
Undrained shear strength, psf	384.1	
Failure strain, %	3.4	
Strain rate, %/min.	1.00	
Water content, %	63.1	
Wet density, pcf	98.0	
Dry density, pcf	60.1	the second second
Saturation, %	94.1	
Void ratio	1.8256	
Specimen diameter, in.	1.411	
Specimen height, in.	2.765	
Height/diameter ratio	1.96	

Description: SO GR CH4 W/ ARS & LNS SP, RT

LL = 83 PL = 23 PI = 60 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/7/20

Remarks:

TORVANE = 0.200 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

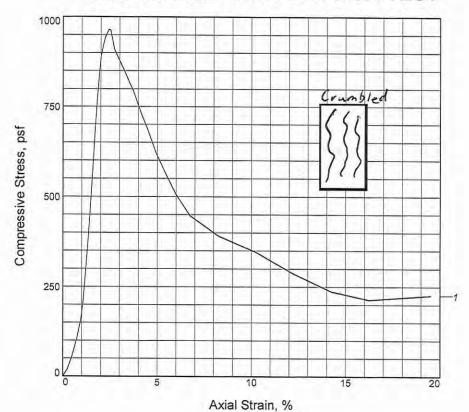
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-04 Depth: 33

Sample Number: 9B

EUSTIS ENGINEERING

Figure ASTM D2166



Specimen No.	1	
Unconfined strength, psf	965.2	
Undrained shear strength, psf	482.6	
Failure strain, %	2.4	
Strain rate, %/min.	1.00	
Water content, %	20.7	
Wet density, pcf	128.6	
Dry density, pcf	106.6	
Saturation, %	94.7	
Void ratio	0.5935	
Specimen diameter, in.	1.403	
Water content, % Wet density, pcf Dry density, pcf Saturation, % Void ratio	20.7 128.6 106.6 94.7 0.5935	

Description: SO GR & T CH2 W/ ARS ML, SL

LL = 52 PL = 20 PI = 32 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/7/20

Specimen height, in.

Height/diameter ratio

Remarks:

TORVANE = 0.750 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

2.753

1.96

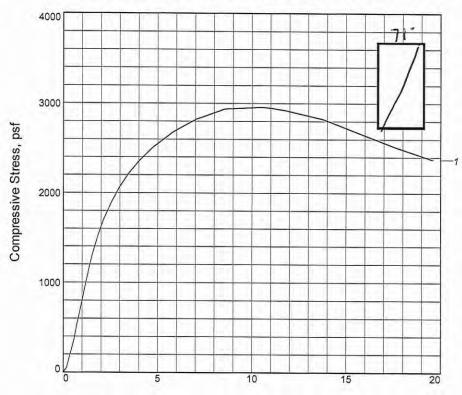
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-04 Depth: 38

Sample Number: 10C

EUSTIS

Figure ASTM D2166



Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	2957.8	
Undrained shear strength, psf	1478.9	
Failure strain, %	10.6	
Strain rate, %/min.	1.00	
Water content, %	28.8	
Wet density, pcf	119.1	
Dry density, pcf	92.5	
Saturation, %	93.6	
Void ratio	0.8365	
Specimen diameter, in.	1.408	
Specimen height, in.	2.742	
Height/diameter ratio	1.95	

Description: ST LGR & T CH3 W/ ARS ML, CC, SL

LL = 65 PL = 21PI = 44Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/7/20

Remarks:

TORVANE = 1.000 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

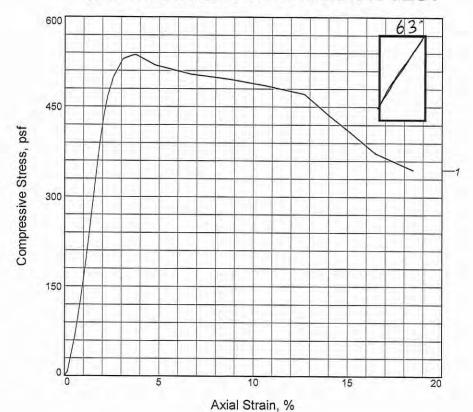
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-04

Depth: 42

Sample Number: 11C

Figure ASTM D2166



Specimen No.	1	
Unconfined strength, psf	536.9	
Undrained shear strength, psf	268.4	
Failure strain, %	3.7	
Strain rate, %/min.	1.00	
Water content, %	32.9	
Wet density, pcf	118.7	
Dry density, pcf	89.3	
Saturation, %	100.0	
Void ratio	0.8869	
Specimen diameter, in.	1.400	
Specimen height, in.	2.782	
Height/diameter ratio	1.99	

Description: SO GR & T CL6

LL = 41PI = 24 PL = 17 Assumed GS= 2.70 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 10/7/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

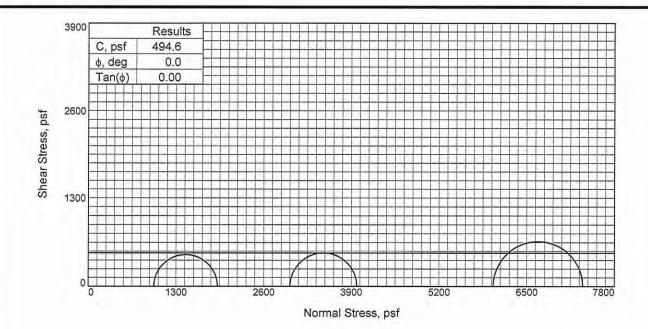
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-04

Sample Number: 12C

Depth: 46

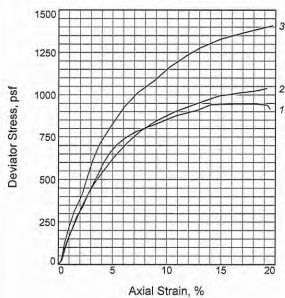
Figure ASTM D2166

Tested By: CC



Specimen No.

Water Content, %



Dry Density, pcf 90.3 89.4 90.8 Saturation, % 100.0 100.0 99.4 Void Ratio 0.8861 0.8566 0.8666 Diameter, in. 1.407 1.414 1.406 Height, in. 2.780 2.775 2.753 Water Content, % 32.8 31.7 32.1 90.3 Dry Density, pcf 89.4 90.8 Saturation, % 100.0 100.0 100.0 Void Ratio 0.8861 0.8566 0.8666 Diameter, in. 1.407 1.414 1.406 Height, in. 2.780 2.775 2.753 1.00 Strain rate, %/min. 1.00 1.00 Back Pressure, psi 0.000 0.000 0.000 Cell Pressure, psi 6.750 20.760 41.670 Fail. Stress, psf 942.3 992.4 1329.8 Strain, % 14.7 14.1 15.0 Ult. Stress, psf 942.3 992.4 1329.8 Strain, % 14.1 14.7 15.0 σ<sub>1</sub> Failure, psf 1914.3 3981.8 7330.3 σ<sub>3</sub> Failure, psf 972.0 2989.4 6000.5

1

32.8

2

31.7

3

31.9

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: SO T & GR CL4 W/ CC

LL= 41

PL= 20

PI= 21

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.625 TSF

Sample Number: 2B

Source of Sample: SB-05

Client: AECOM, NEW ORLEANS, LOUISIANA

RESTORATION AUTHORITY (CPRA),

Proj. No.: 24384

Date Sampled: 9/14/20

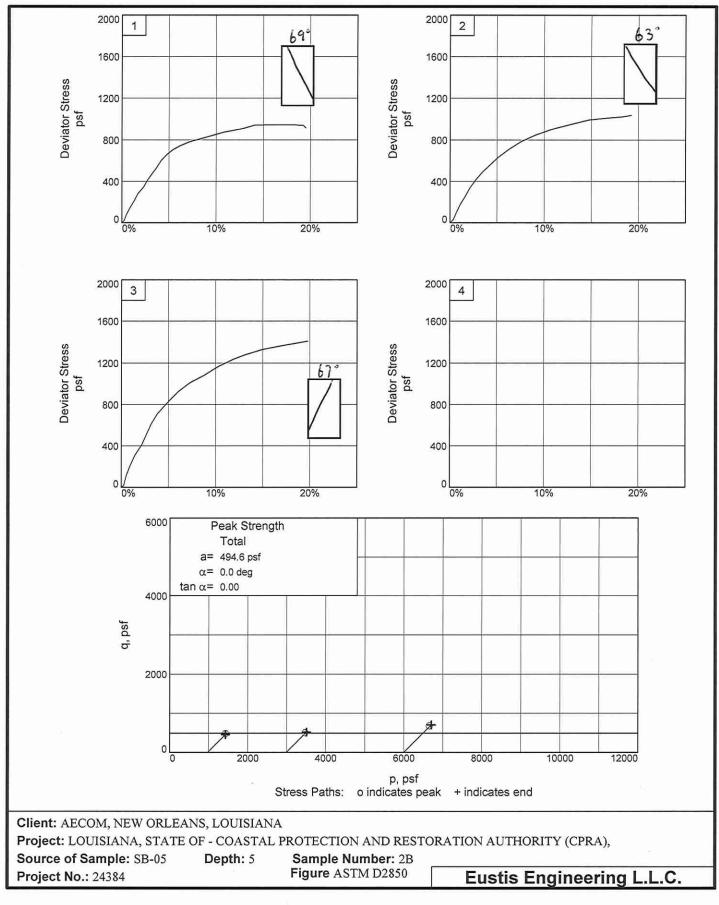


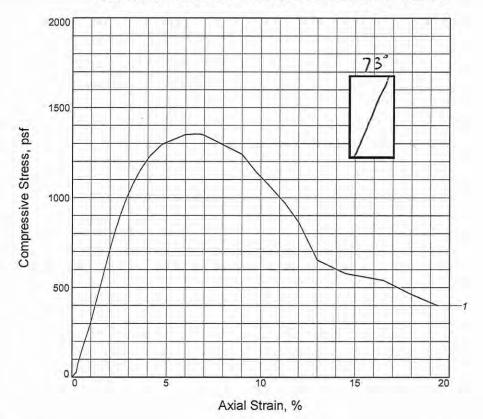
Depth: 5

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

Figure ASTM D2850

Tested By: DE





Specimen No.	1	
Unconfined strength, psf	1352.9	
Undrained shear strength, psf	676.5	
Failure strain, %	6.7	
Strain rate, %/min.	0.50	
Water content, %	47.8	
Wet density, pcf	107.3	
Dry density, pcf	72.6	
Saturation, %	97.1	
Void ratio	1.3391	
Specimen diameter, in.	1.409	
Specimen height, in.	2.755	
Height/diameter ratio	1.96	

Description: M T & GR CH4 W/ ARS ML, CC

LL = 80PL = 33PI = 47Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/17/20

Remarks:

TORVANE = 0.750 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

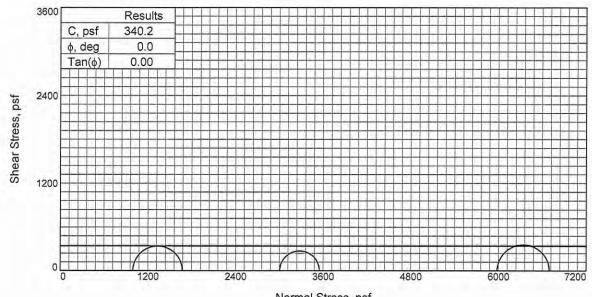
RESTORATION AUTHORITY (CPRA), Depth: 10

Source of Sample: SB-05

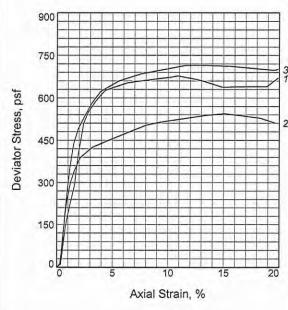
Sample Number: 3C



Figure ASTM D2166



Normal Stress, psf



Sp	ecimen No.	1	2	3	
	Water Content, %	76.3	75.3	77.0	
Initial	Dry Density, pcf	54.4	55.0	53.6	
	Saturation, %	97.8	98.1	96.6	
=	Void Ratio	2.1217	2.0877		
	Diameter, in.	1.423			
	Height, in.	2.753	2.754	2.765	
	Water Content, %	78.0	76.8	79.7	
+	Dry Density, pcf	54.4	55.0	53.6	
At Test	Saturation, %	100.0	100.0	100.0	
T+	Void Ratio	2.1217	2.0877	2.1678	
1	Diameter, in.	1.423	1.413	1.414	
	Height, in.	2.753	2.754	2.765	
Str	ain rate, %/min.	1.00	1.00	1.00	
Ва	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	6.900	20.860	41.530	
Fai	I. Stress, psf	677.8	544.2	715.3	
5	Strain, %	10.9	15.0	11.6	
Ult	Stress, psf	637.9	544.2	714.3	
5	Strain, %	14.9	15.0	13.6	
σ1	Failure, psf	1671.4	3548.0	6695.6	
$\sigma_3$	Failure, psf	993.6	3003.8	5980.3	

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED Description: SO GR & T CHOA

LL= 117 PL= 35 PI= 82

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-05 Depth: 14

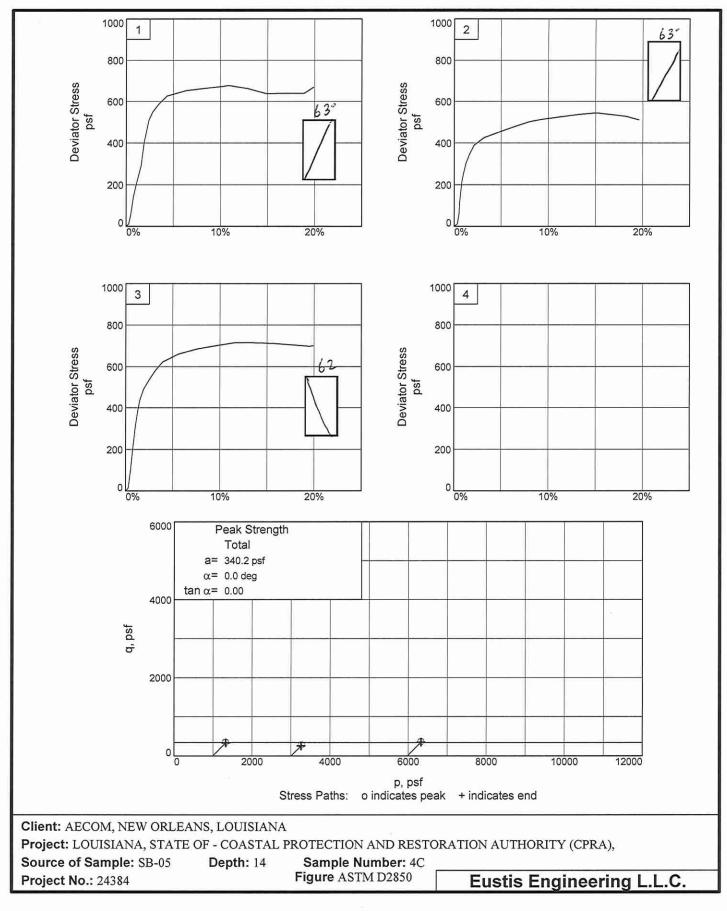
Sample Number: 4C

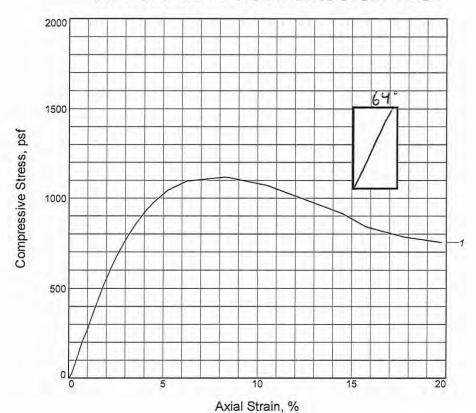
Proj. No.: 24384 Date Sampled: 9/14/20



Figure ASTM D2850

Tested By: DE





Specimen No.	1	
Unconfined strength, psf	1118.7	
Undrained shear strength, psf	559.3	
Failure strain, %	8.2	
Strain rate, %/min.	1.00	
Water content, %	114.3	
Wet density, pcf	88.4	
Dry density, pcf	41.3	
Saturation, %	100.0	
Void ratio	3.0861	
Specimen diameter, in.	1.394	
Specimen height, in.	2.760	
Height/diameter ratio	1.98	

Description: M GR & DGR CHOA

LL = 137 PL = 37 PI = 100 Assumed GS = 2.70 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/14/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

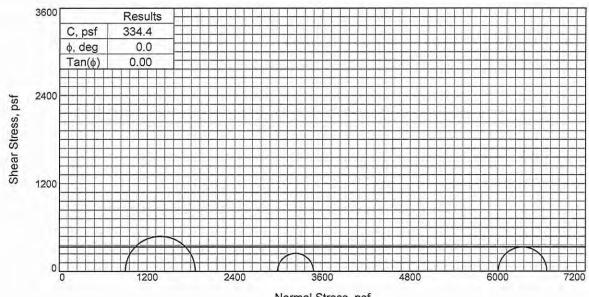
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-05 Depth: 19

Comple Number 5D

Sample Number: 5D

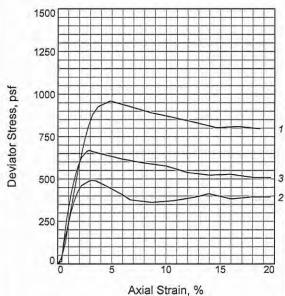
EUSTIS ENGINEERING

Figure ASTM D2166



Normal Stress, psf

Specimen No.



-	Conficients.				
Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	51.0 70.9 99.6 1.3934 1.399 2.737	52.9 69.6 99.9 1.4399 1.397 2.751	55.6 65.9 95.8 1.5772 1.414 2.763	
AtTest	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	51.2 70.9 100.0 1.3934 1.399 2.737	52.9 69.6 100.0 1.4399 1.397 2.751	58.0 65.9 100.0 1.5772 1.414 2.763	
7.7	ain rate, %/min.	1.00	1.00	1.00	
Ba	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	6.260	20.740	41.710	
Fai	I. Stress, psf	960.1	493.1	668.0	
1 5	Strain, %	4.8	3.1	3.0	
Ult.	Stress, psf	802.7	362.0	523.4	
	Strain, %	14.6	8.7	14.0	
σ1	Failure, psf	1861.5	3479.6	6674.3	
03	Failure, psf	901.4	2986.6	6006.2	

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR CH3 W/ ARS & LNS ML

LL= 58

PL= 22

PI= 36

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.375 TSF

Sample Number: 7B

Source of Sample: SB-05

Proj. No.: 24384

Date Sampled: 9/14/20

2

3



Client: AECOM, NEW ORLEANS, LOUISIANA

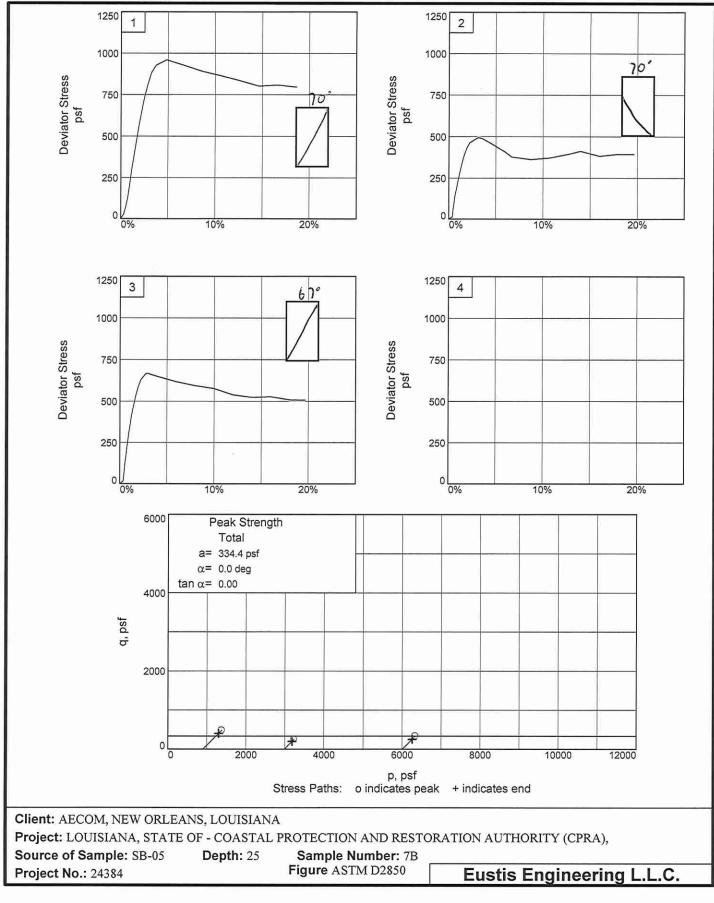
RESTORATION AUTHORITY (CPRA),

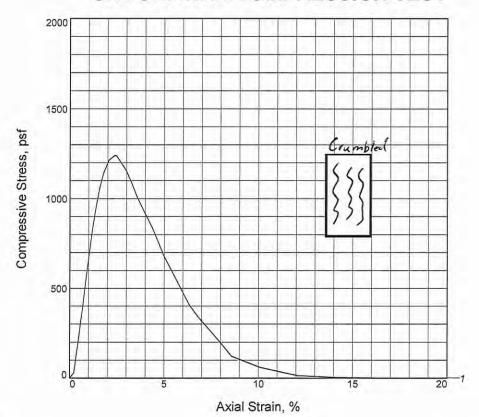
ENGINEERING

Depth: 25

Figure ASTM D2850

Tested By: DE





Specimen No.	1	
Unconfined strength, psf	1240.2	
Undrained shear strength, psf	620.1	
Failure strain, %	2.4	
Strain rate, %/min.	0.50	
Water content, %	34.9	
Wet density, pcf	111.7	
Dry density, pcf	82.8	
Saturation, %	90.4	
Void ratio	1.0508	
Specimen diameter, in.	1.390	
Specimen height, in.	2.755	
Height/diameter ratio	1.98	

Description: M GR & BR CH3 W/ ARS ML, SL

LL = 79 PL = 26 PI = 53 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/17/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

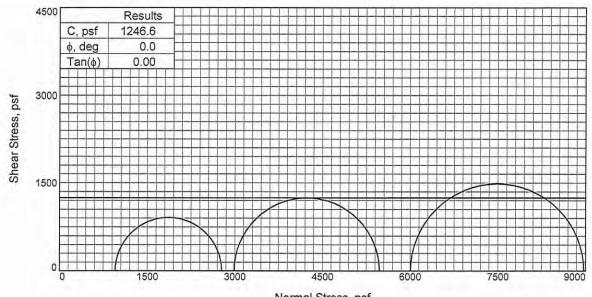
RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-05 Depth: 30

Sample Number: 8C

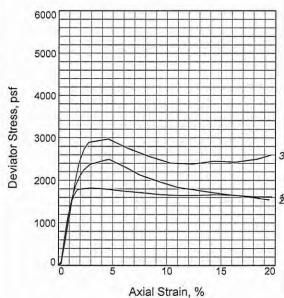
EUSTIS ENGINEERING

Figure ASTM D2166

Tested By: CC



Normal Stress, psf



Spe	ecimen No.	1	2	3	
F	Water Content, % Dry Density, pcf	37.2 80.3	37.2 82.0	35.6 83.0	
ā	Saturation, %	90.7	94.5	92.5	
Initial	Void Ratio	1.1150	1.0703	1.0466	
	Diameter, in.	1.404	1.405	1.412	
	Height, in.	2.763	2.981	2.764	
	Water Content, %	41.0	39.3	38.5	
4	Dry Density, pcf	80.3	82.0	83.0	
At Test	Saturation, %	100.0	100.0	100.0	
1	Void Ratio	1.1150	1.0703	1.0466	
_	Diameter, in.	1.404	1.405	1.412	
	Height, in.	2.763	2.981	2.764	
Str	ain rate, %/min.	0.30	0.30	0.30	
Bad	ck Pressure, psi	0.000	0.000	0.000	
Cel	l Pressure, psi	6.600	20.730	41.680	
Fai	I. Stress, psf	1824.3	2487.9	2968.4	
5	Strain, %	2.9	4.5	4.6	
Ult.	Stress, psf	1639.4	1703.2	2382.7	
	Strain, %	11.3	14.5	12.3	
σ1	Failure, psf	2774.7	5473.0	8970.4	
σ3	Failure, psf	950.4	2985.1	6001.9	

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: ST GR & T CH4 W/ ARS & LNS ML

LL= 82

PL= 25

PI= 57

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.625 TSF

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-05 Depth: 41

Client: AECOM, NEW ORLEANS, LOUISIANA

Sample Number: 11B

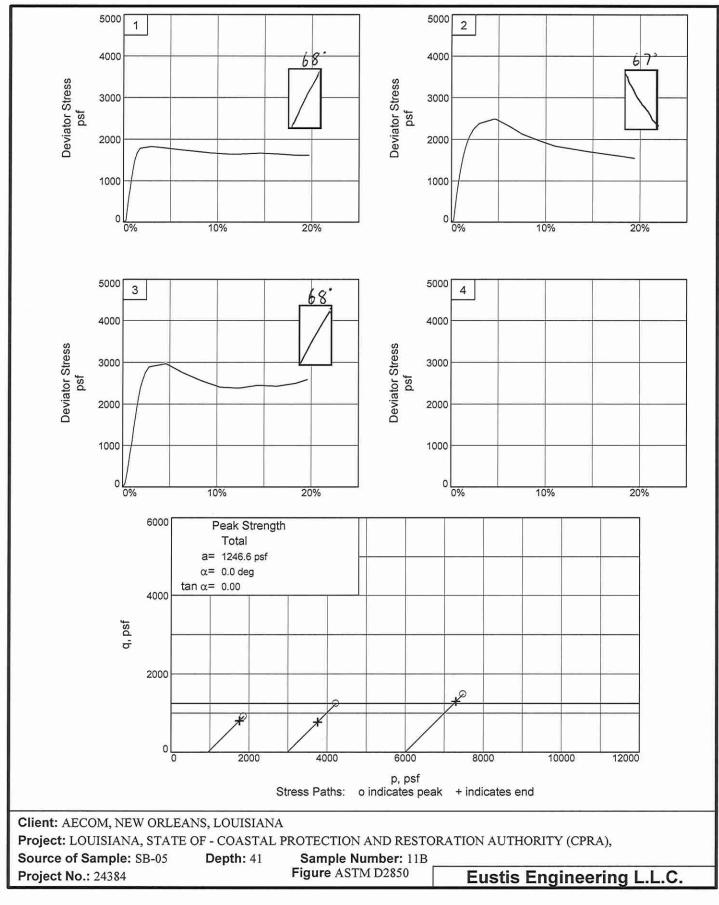
Proj. No.: 24384

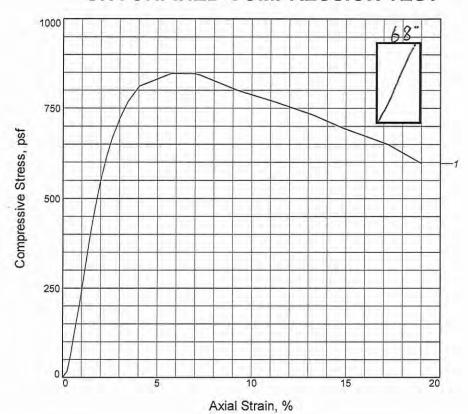
Date Sampled: 9/14/20



Figure ASTM D2850

Tested By: MS





Specimen No.	1	
Unconfined strength, psf	847.6	
Undrained shear strength, psf	423.8	
Failure strain, %	5.7	
Strain rate, %/min.	1.00	
Water content, %	30.5	
Wet density, pcf	117.2	
Dry density, pcf	89.8	
Saturation, %	93.1	
Void ratio	0.8915	
Specimen diameter, in.	1.400	
Specimen height, in.	2.756	
Height/diameter ratio	1.97	

Description: SO GR & BR CH2 W/ CC, WD

LL = 50 PL = 20 PI = 30 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 9/15/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

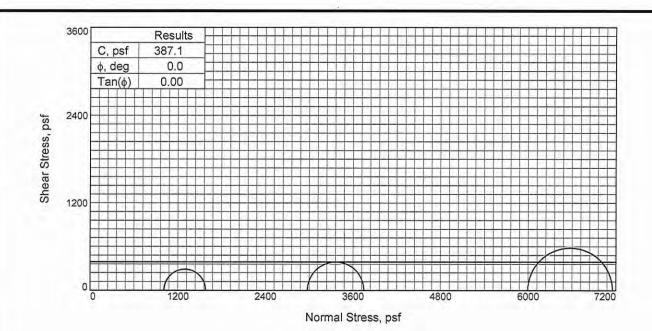
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-06 Depth: 5

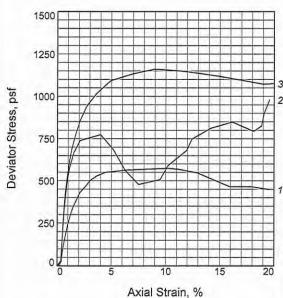
Sample Number: 2B

EUSTIS ENGINEERING SINCE 1946

Figure ASTM D2166



Specimen No.



Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio	50.3 71.2 98.9 1.3837	46.8 74.2 98.6 1.2898		
-	Diameter, in. Height, in.	1.402 2.801	1.403 2.797		
est	Water Content, % Dry Density, pcf Saturation, %	50.9 71.2 100.0	47.4 74.2 100.0	48.2 73.5 100.0	
At Test	Void Ratio Diameter, in. Height, in.	1.3837 1.402 2.801	1.2898 1.403 2.797		
Str	ain rate, %/min.	1.00	1.00	1.00	
Bad	ck Pressure, psi	0.000	0.000	0.000	
Cel	Il Pressure, psi	7.000	20.660	41.620	
Fai	I. Stress, psf	573.6	773.8	1161.1	
5	Strain, %	10.2	4.0	9.0	
Ult.	Stress, psf	504.9	477.8	1117.9	
5	Strain, %	14.4	7.5	15.0	
σ1	Failure, psf	1581.6	3748.8	7154.4	
$\sigma_3$	Failure, psf	1008.0	2975.0	5993.3	

2

1

3

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR CH4 W/ ARS ML, SL

LL= 97 PL= 33 PI= 64

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.625 TSF

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-06 Depth: 10

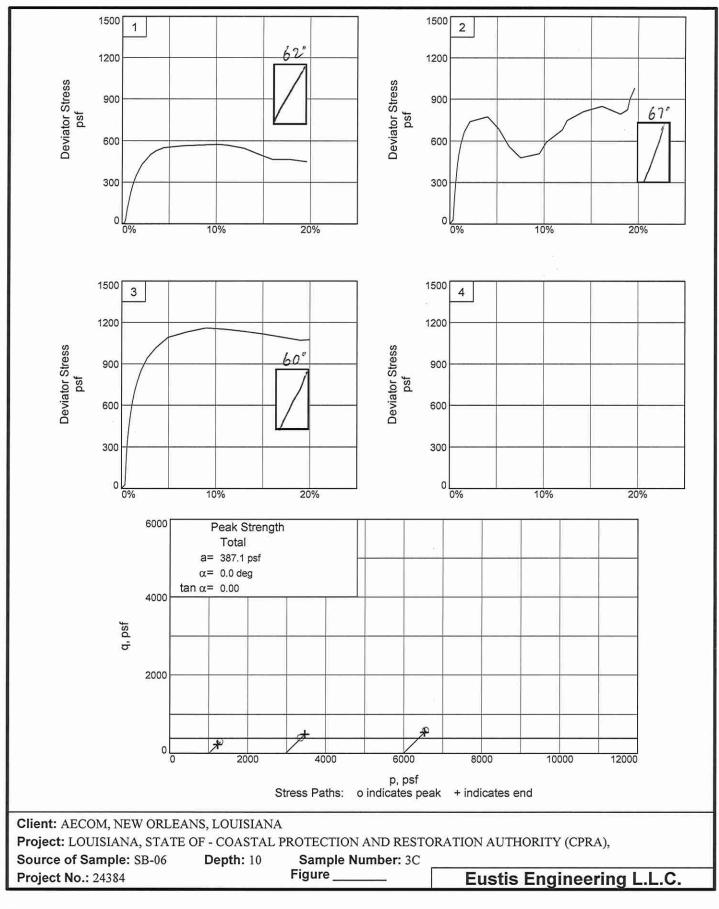
Client: AECOM, NEW ORLEANS, LOUISIANA

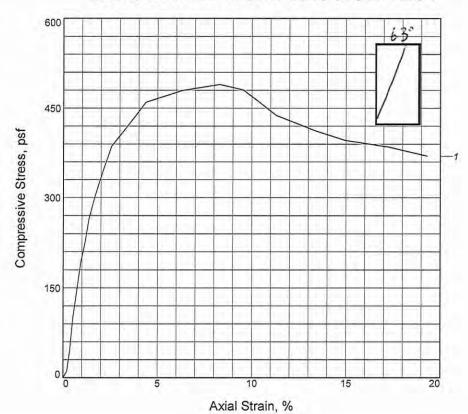
Sample Number: 3C

Proj. No.: 24384 Date Sampled: 9/15/20



**Figure** 





Specimen No.	1	
Unconfined strength, psf	489.6	
Undrained shear strength, psf	244.8	
Failure strain, %	8.3	
Strain rate, %/min.	1.00	
Water content, %	94.5	
Wet density, pcf	92.4	
Dry density, pcf	47.5	
Saturation, %	99.9	
Void ratio	2.5753	
Specimen diameter, in.	1.375	
Specimen height, in.	2.767	
Height/diameter ratio	2.01	

Description: VSO GR CH4 W/ ARS ML, WD, RT, SL

LL = 132 PL = 32 PI = 100 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/15/20

Remarks:

TORVANE = 0.150 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

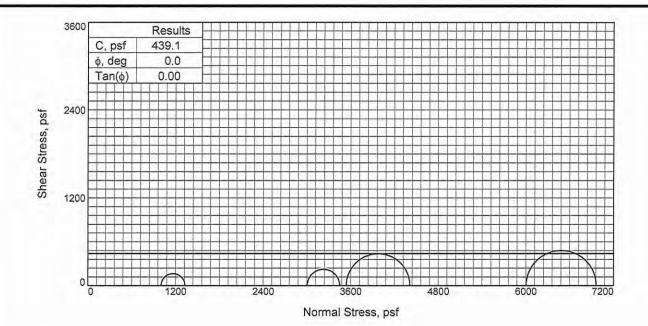
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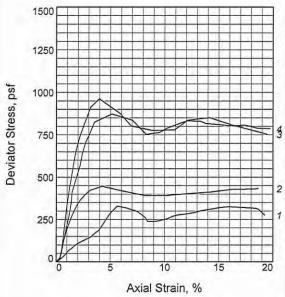
RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-06 Depth: 14

Sample Number: 4C

EUSTIS ENGINEERING SINCE 1944

Figure ASTM D2166





	Sp	ecimen No.	1	2	3	4
	Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	70.4 57.5 98.0 1.9522 1.399 2.746	70.0 58.5 100.1 1.9035 1.399 2.735	71.8 57.4 99.7 1.9594 1.397 2.756	70.1 58.2 99.6 1.9152 1.394 2.767
2	At Test	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	71.8 57.5 100.0 1.9522 1.399 2.746	70.0 58.5 100.0 1.9035 1.399 2.735	72.0 57.4 100.0 1.9594 1.397 2.756	70.4 58.2 100.0 1.9152 1.394 2.767
	Str	ain rate, %/min.	1.00	1.00	1.00	1.00
	Ba	ck Pressure, psi	0.000	0.000	0.000	0.000
	Ce	Il Pressure, psi	6.950	20.850	41.670	24.570
	Fai	I. Stress, psf	329.0	446.5	962.3	873.0
	5	Strain, %	5.6	4.2	4.0	5.1
	Ult.	Stress, psf	238.4	391.5	775.6	753.1
		Strain, %	8.4	10.1	8.9	8.3
-	σ1	Failure, psf	1329.8	3448.9	6962.8	4411.0
	$\sigma_3$	Failure, psf	1000.8	3002.4	6000.5	3538.1

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR CH4 W/ ARS ML, RT

LL= 98

PL= 27

PI= 71

Assumed Specific Gravity= 2.72 Remarks: TORVANE =0.150 TSF

Sample Number: 6B

Source of Sample: SB-06

Proj. No.: 24384

Date Sampled: 9/15/20



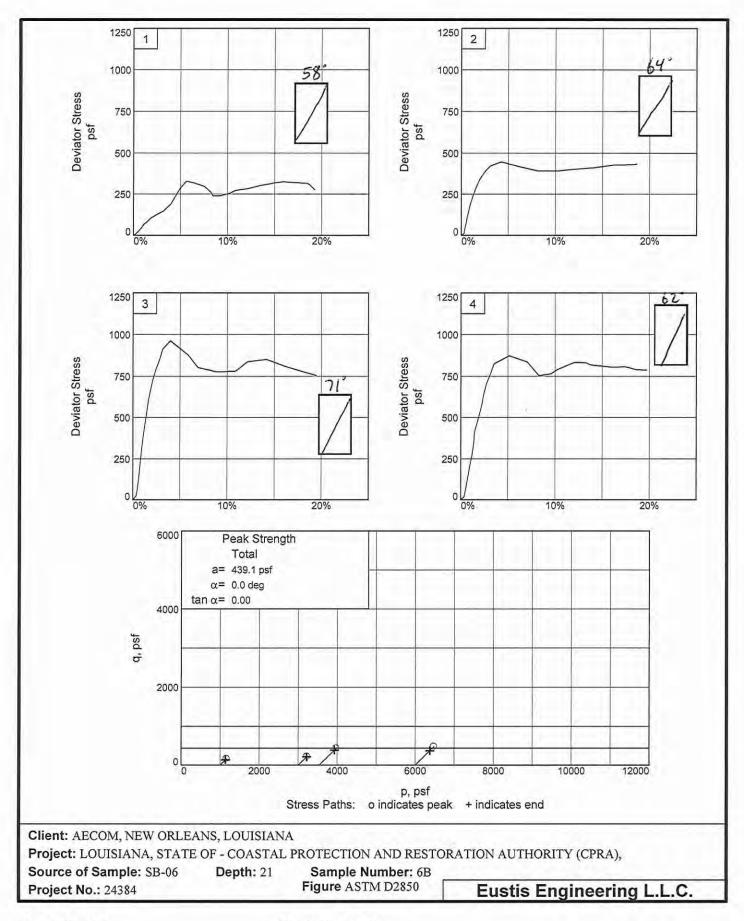
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

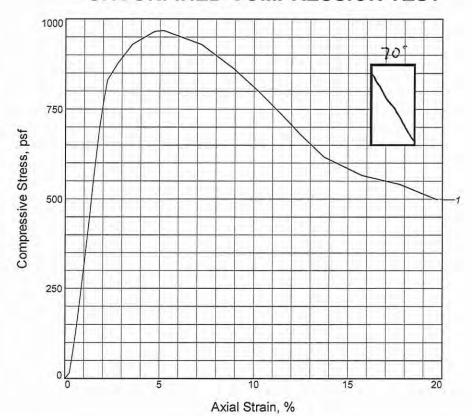
Depth: 21

Client: AECOM, NEW ORLEANS, LOUISIANA

RESTORATION AUTHORITY (CPRA),

Figure ASTM D2850





Specimen No.	1	
Unconfined strength, psf	967.8	
Undrained shear strength, psf	483.9	
Failure strain, %	5.2	
Strain rate, %/min.	1.00	
Water content, %	25.2	
Wet density, pcf	121.6	
Dry density, pcf	97.1	
Saturation, %	92.5	
Void ratio	0.7361	
Specimen diameter, in.	1.389	
Specimen height, in.	2.760	
Height/diameter ratio	1.99	

Description: SO LGR CL6

LL = 41 PL = 17 PI = 24 Assumed GS = 2.70 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/15/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

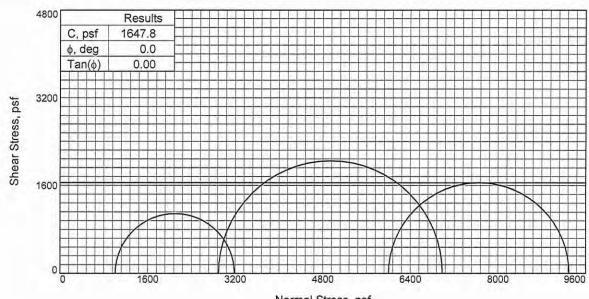
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-06 Depth: 26

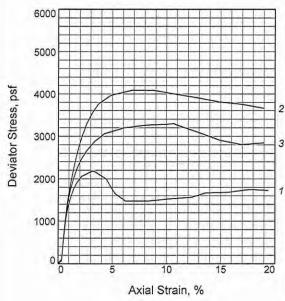
Sample Number: 7C

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Figure ASTN D2166



Normal Stress, psf



Type	-6.	T4-
IVDE	OT	I PST

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: ST T & LGR CH4 W/ ARS ML, SL, CC

LL= 66

PL= 25

PI= 41

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 1.000 TSF

Sp	ecimen No.	1	2	3	
a	Water Content, % Dry Density, pcf Saturation, %	27.4 95.4 95.4	26.6 96.0 94.0	28.2 95.3 98.0	
Initial	Void Ratio Diameter, in.	0.7803 1.396 2.736	0.7694	0.7817 1.395	
+	Height, in.  Water Content, %  Dry Density, pcf	28.7 95.4	28.3 96.0	28.7 95.3	
At Test	Saturation, % Void Ratio Diameter, in. Height, in.	100.0 0.7803 1.396 2.736	100.0 0.7694 1.403 2.762	100.0 0.7817 1.395 2.755	
Str	ain rate, %/min.	1.00	1.00	1.00	_
17 24 55	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	7.000	20.100	41.660	
Fai	I. Stress, psf	2178.7	4090.7	3298.6	
5	Strain, %	3.3	6.9	10.7	
Ult.	Stress, psf	1474.7	3806.2	2907.0	
5	Strain, %	8.2	14.9	14.9	
$\sigma_1$	Failure, psf	3186.7	6985.1	9297.6	
$\sigma_3$	Failure, psf	1008.0	2894.4	5999.0	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-06 Depth: 34

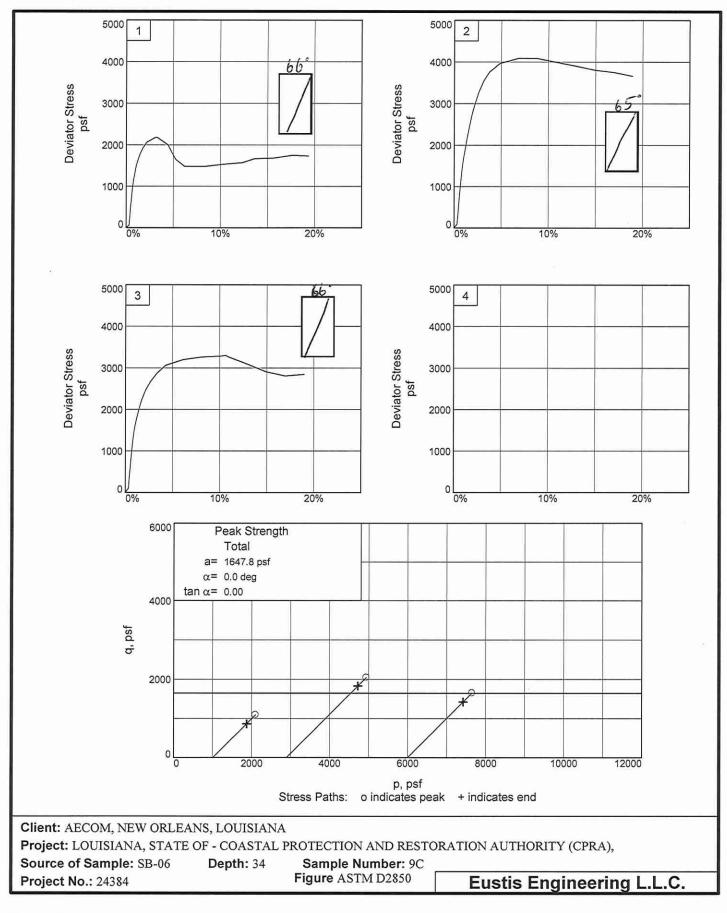
Sample Number: 9C

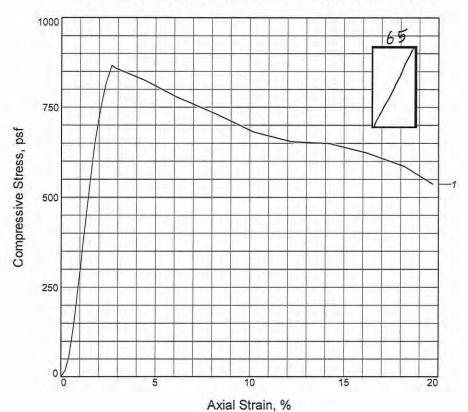
Proj. No.: 24384

Date Sampled: 9/15/20



Figure ASTN D2850





Specimen No.	1	
Unconfined strength, psf	867.2	
Undrained shear strength, psf	433.6	
Failure strain, %	2.7	
Strain rate, %/min.	1.00	
Water content, %	32.7	
Wet density, pcf	117.3	
Dry density, pcf	88.4	
Saturation, %	97.4	
Void ratio	0.9070	
Specimen diameter, in.	1.423	
Specimen height, in.	2.750	
Height/diameter ratio	1.93	

Description: SO GR & T CL6

LL = 44 PL = 16 PI = 28 Assumed GS = 2.70 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/15/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

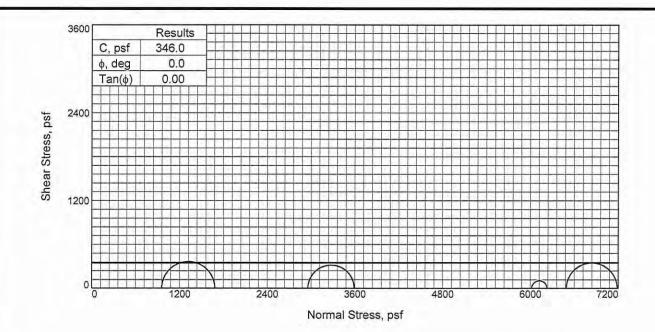
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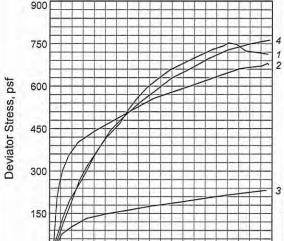
RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-06 Depth: 38

Sample Number: 10C

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Figure ASTM D2166





Axial Strain, %

2 Specimen No. 1 3 4 Water Content, % 33.2 32.8 32.9 31.9 Dry Density, pcf 88.0 87.9 86.8 90.0 Saturation, % 97.8 96.3 94.3 98.7 0.9151 Void Ratio 0.9185 0.9411 0.8722 Diameter, in. 1.408 1.406 1.388 1.412 Height, in. 2.742 2.833 2.759 2.820 33.9 34.0 34.9 32.3 Water Content, % Dry Density, pcf 88.0 87.9 86.8 90.0 Saturation, % 100.0 100.0 100.0 100.0 Void Ratio 0.9151 0.9185 0.9411 0.8722 Diameter, in. 1.408 1.406 1.388 1.412 Height, in. 2.742 2.833 2.759 2.820 Strain rate, %/min. 1.00 1.00 1.00 1.00 Back Pressure, psi 0.000 0.000 0.000 0.000 6.630 20.530 45.070 Cell Pressure, psi 41.780 Fail. Stress, psf 729.5 634.5 210.4 696.1 Strain, % 14.6 15.1 15.0 14.3 Ult. Stress, psf 729.5 634.5 210.4 696.1 Strain, % 14.6 15.1 15.0 14.3 σ<sub>1</sub> Failure, psf 1684.2 3590.8 6226.7 7186.2 σ<sub>3</sub> Failure, psf 954.7 2956.3 6016.3 6490.1

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED Description: SO GR CL4 W/ O

LL= 37

PL= 20

PI= 17

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.150 TSF

Sample Number: 2B

Source of Sample: SB-07

Proj. No.: 24384

Date Sampled: 9/15/20



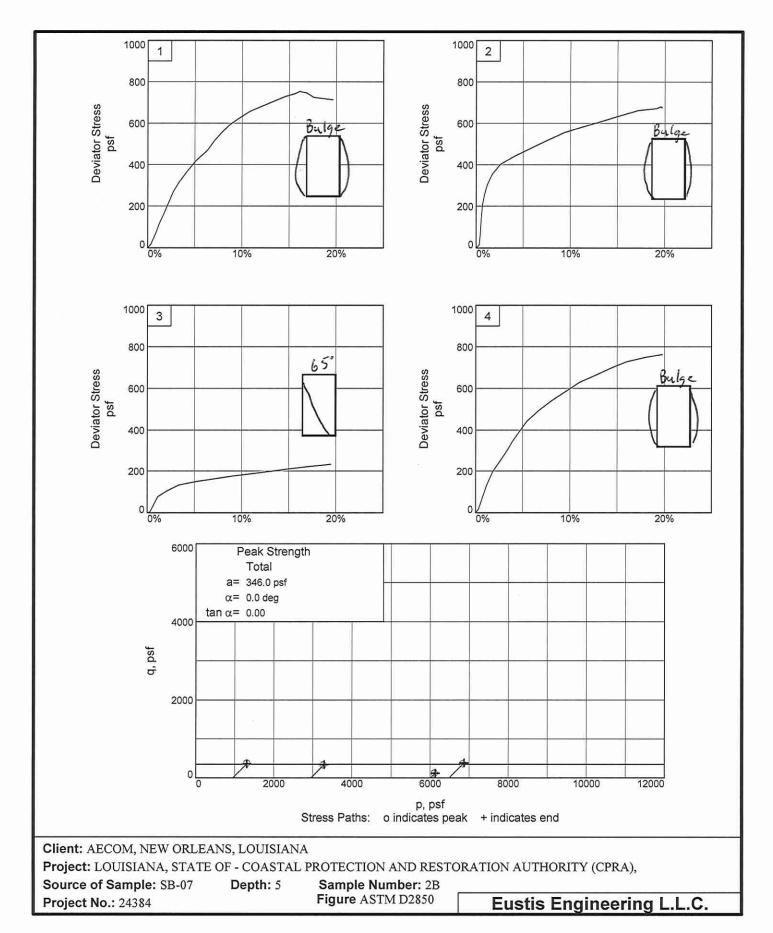
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

Depth: 5

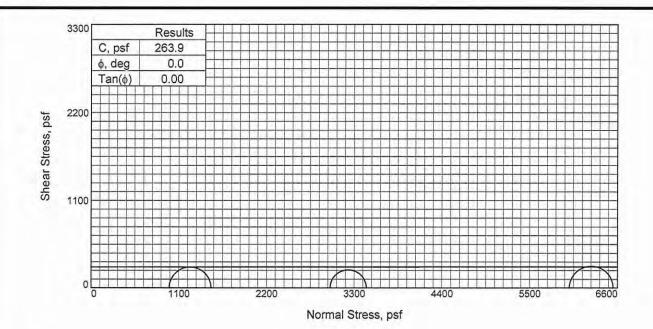
Client: AECOM, NEW ORLEANS, LOUISIANA

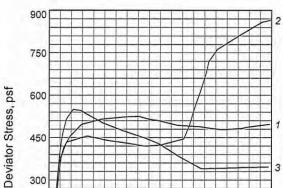
RESTORATION AUTHORITY (CPRA),

Figure ASTM D2850



 Tested By: DE
 Checked By: RR





Axial Strain, %

Type of Test:

450

300

150

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR CH4 W/ ARS ML, RT

PI= 70 LL= 101 PL= 31

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.500 TSF

Specimen No.		1	2	3	
Initial	Water Content, % Dry Density, pcf	58.4 65.0	58.2 65.8	58.1 65.2	
	Saturation, % Void Ratio	98.4 1.6133	100.0 1.5821	98.6 1.6036	
	Diameter, in. Height, in.	1.433 2.766	1.388	1.429	
sst	Water Content, %	59.3	58.2	59.0	
	Dry Density, pcf Saturation, %	65.0 100.0	65.8 100.0	65.2 100.0	
At Test	Void Ratio	1.6133	1.5821	1.6036	
4	Diameter, in. Height, in.	1.433 2.766	1.388 2.758	1.429 2.736	
Str	ain rate, %/min.	1.00	1.00	1.00	
Back Pressure, psi		0.000	0.000	0.000	
Cell Pressure, psi		6.800	20.810	41.640	
Fail. Stress, psf		525.4	455.5	550.1	
Strain, %		8.0	3.4	2.2	
Ult.	Stress, psf	488.4	419.7	339.6	
Strain, %		13.6	8.6	13.7	
σ1	Failure, psf	1504.6	3452.1	6546.2	
σ <sub>3</sub> Failure, psf		979.2	2996.6	5996.2	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

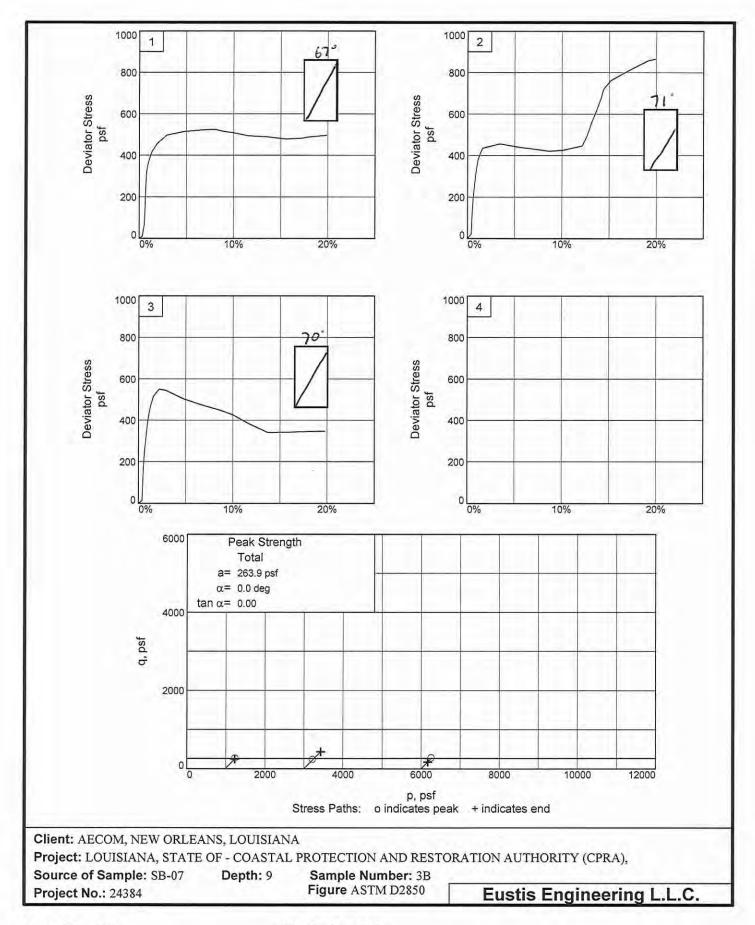
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-07 Depth: 9

Sample Number: 3B

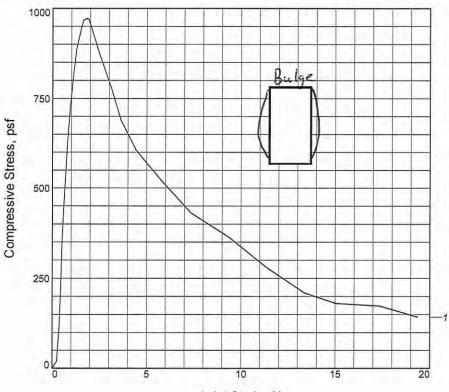
Proj. No.: 24384 Date Sampled: 9/21/20



Figure ASTM D2850



## **UNCONFINED COMPRESSION TEST**



Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	971.7	
Undrained shear strength, psf	485.8	
Failure strain, %	1.8	
Strain rate, %/min.	1.00	
Water content, %	43.9	
Wet density, pcf	109.7	
Dry density, pcf	76.2	
Saturation, %	97.3	
Void ratio	1.2284	
Specimen diameter, in.	1.400	
Specimen height, in.	2.752	
Height/diameter ratio	1.97	

Description: SO GR CH4 W/ ARS ML

LL = 82 PL = 23 PI = 59 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 9/15/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

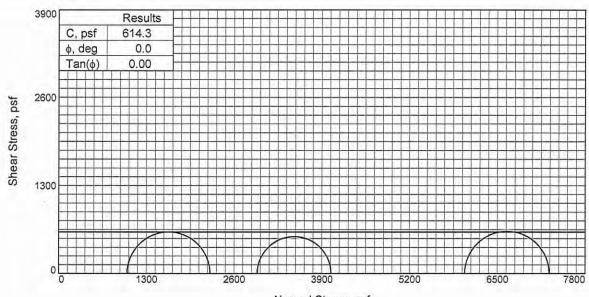
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-07 Depth: 10

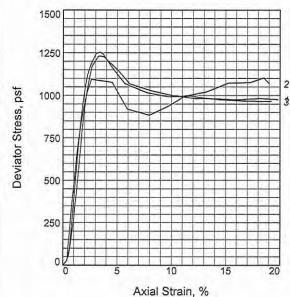
Sample Number: 3C

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Figure ASTM D2166



Normal Stress, psf



Sp	ecimen No.	1	2	3	
Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	117.9 40.1 99.9 3.1277 1.370 2.765		3.2342	
At Test	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	118.0 40.1 100.0 3.1277 1.370 2.765		3.2342	
Ва	ain rate, %/min. ck Pressure, psi Il Pressure, psi	1.00 0.000 7.030	1.00 0.000 20.430	1.00 0.000 41.770	
Fa	il. Stress, psf Strain, %	1231.2 3.5	1094.1 2.6	1253.0 3.4	
1	. Stress, psf Strain, % Failure, psf	980.1 14.2 2243.5	883.0 8.0 4036.1	974.2 14.5 7267.8	
$\sigma_3$	Failure, psf	1012.3	2941.9	6014.9	

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED Description: M DGR CHOB

LL= 180

PL= 63

PI= 117

Assumed Specific Gravity= 2.65 Remarks: TORVANE = 0.375 TSF Source of Sample: SB-07 Sample Number: 5B

Proj. No.: 24384

Date Sampled: 9/15/20



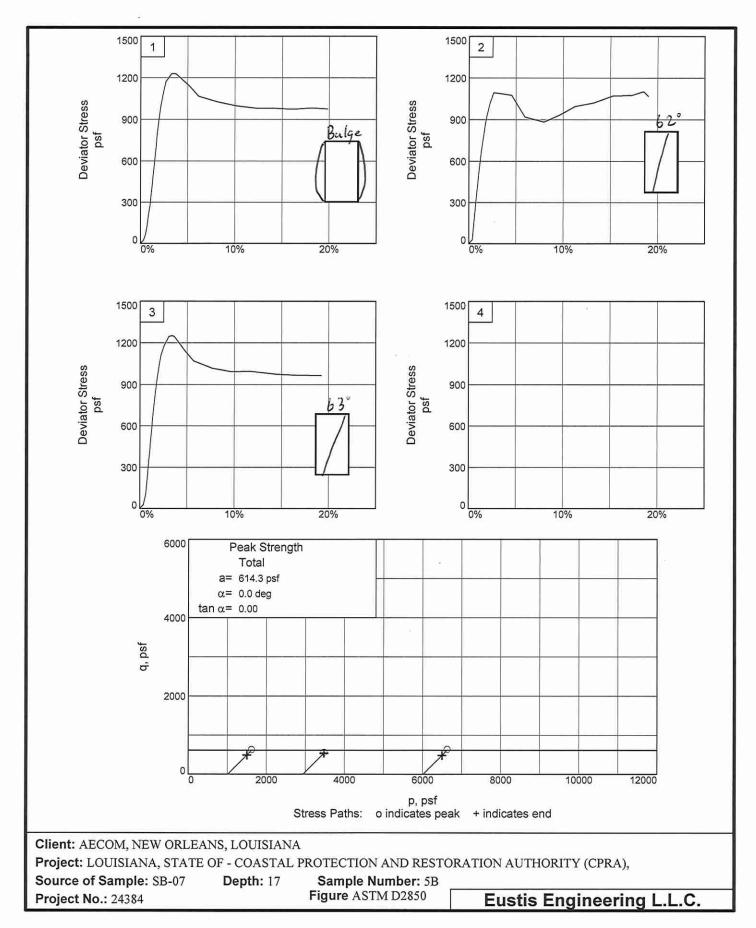
Client: AECOM, NEW ORLEANS, LOUISIANA

RESTORATION AUTHORITY (CPRA),

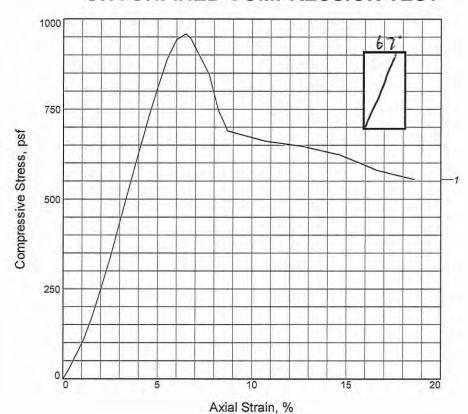
Depth: 17

Figure ASTM D2850

Tested By: CC



## **UNCONFINED COMPRESSION TEST**



Specimen No.	1	
Unconfined strength, psf	958.5	
Undrained shear strength, psf	479.3	
Failure strain, %	6.5	
Strain rate, %/min.	1.00	
Water content, %	32.7	
Wet density, pcf	118.8	
Dry density, pcf	89.5	
Saturation, %	100.0	
Void ratio	0.8830	
Specimen diameter, in.	1.376	
Specimen height, in.	2.754	
Height/diameter ratio	2.00	

Description: SO GR CL4 W/ WD

LL = 28 PL = 17 PI = 11 Assumed GS = 2.70 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 9/15/20

Bate Gampiea. 3/13/20

Remarks:

TORVANE = 0.450 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

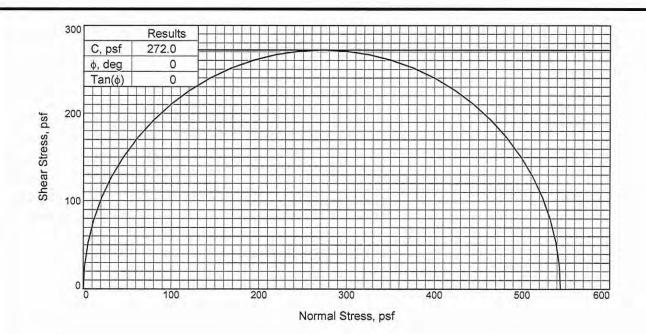
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

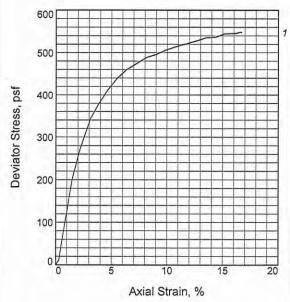
RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-07 Depth: 21

Sample Number: 6B

EUSTIS ENGINEERING

Figure ASTM D2166





Sp	ecimen No.	1	
Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	34.4 87.4 99.9 0.9285 2.882 5.683	
At Test	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	34.4 87.4 100.0 0.9285 2.882 5.683	
Str	ain rate, %/min.	0.50	
Ba	ck Pressure, psi	0.000	
Ce	Il Pressure, psi	0.000	
Fai	I. Stress, psf	544.0	
5	Strain, %	15.1	
Ult.	Stress, psf	544.0	
5	Strain, %	15.1	
$\sigma_1$	Failure, psf	544.0	
$\sigma_3$	Failure, psf	0.0	

Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: SO GR CL6 W/ SIF

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.150 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-07 Depth: 25

Sample Number: 7B

Proj. No.: 24384 Date Sampled: 9/17/20

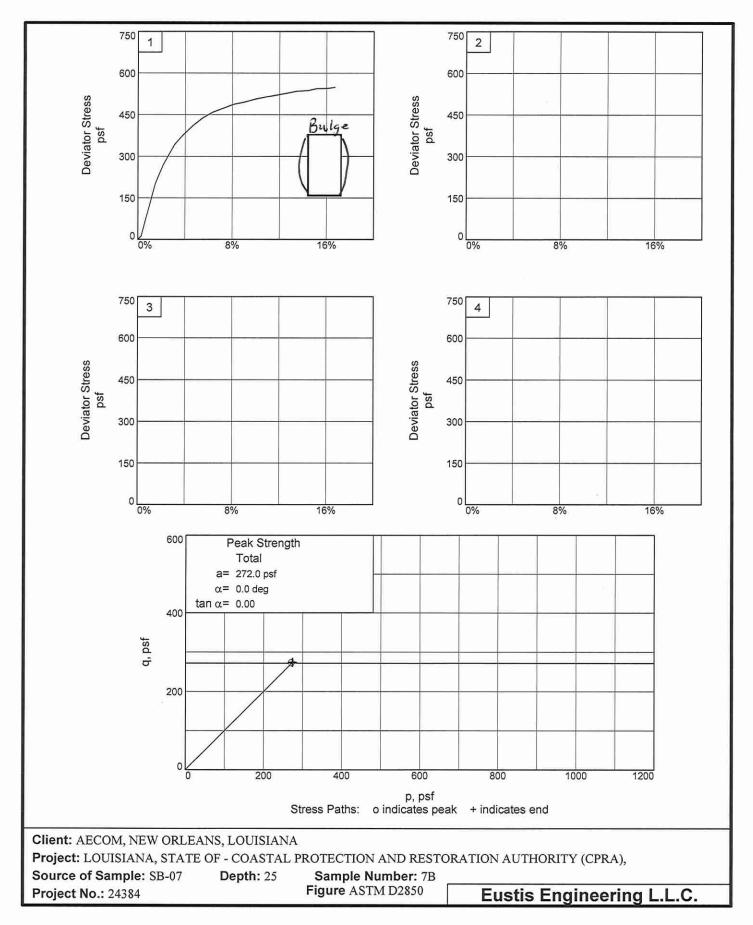


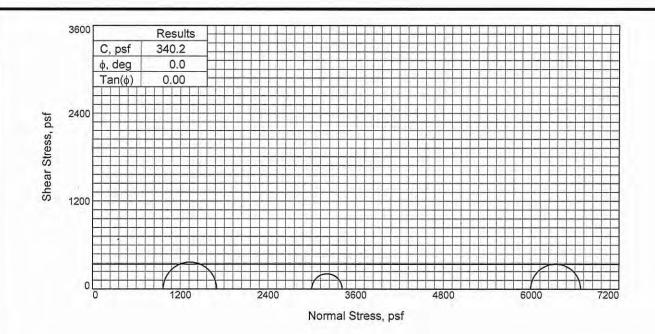
EUSTIS

SINCE 1946

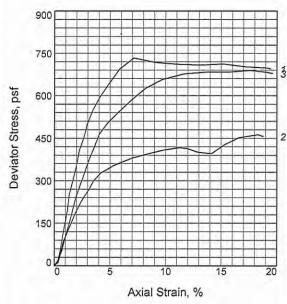
Figure ASTM D2850

Tested By: BH





Specimen No.



37.0 35.2 34.8 Water Content, % Dry Density, pcf 84.3 86.4 86.9 Saturation, % 100.0 100.0 100.0 Void Ratio 0.9997 0.9502 0.9391 Diameter, in. 1.392 1.385 1.388 Height, in. 2.740 2.766 2.774 Water Content, % 37.0 35.2 34.8 Dry Density, pcf 86.9 84.3 86.4 Saturation, % 100.0 100.0 100.0 Void Ratio 0.9997 0.9502 0.9391 Diameter, in. 1.388 1.392 1.385 Height, in. 2.740 2.766 2.774 Strain rate, %/min. 1.00 1.00 1.00 Back Pressure, psi 0.000 0.000 0.000 Cell Pressure, psi 6.700 20.820 41.610 Fail. Stress, psf 733.2 416.6 683.3 Strain, % 7.1 11.2 13.7 Ult. Stress, psf 708.4 395.6 683.3 Strain, % 13.2 14.1 13.7 σ<sub>1</sub> Failure, psf 1698.0 3414.7 6675.1 σ<sub>3</sub> Failure, psf 964.8 2998.1 5991.8

2

1

3

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: SO GR CL6 W/ SIF

LL= 41 PL= 18 PI= 23

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.150 TSF

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-07 Depth: 26

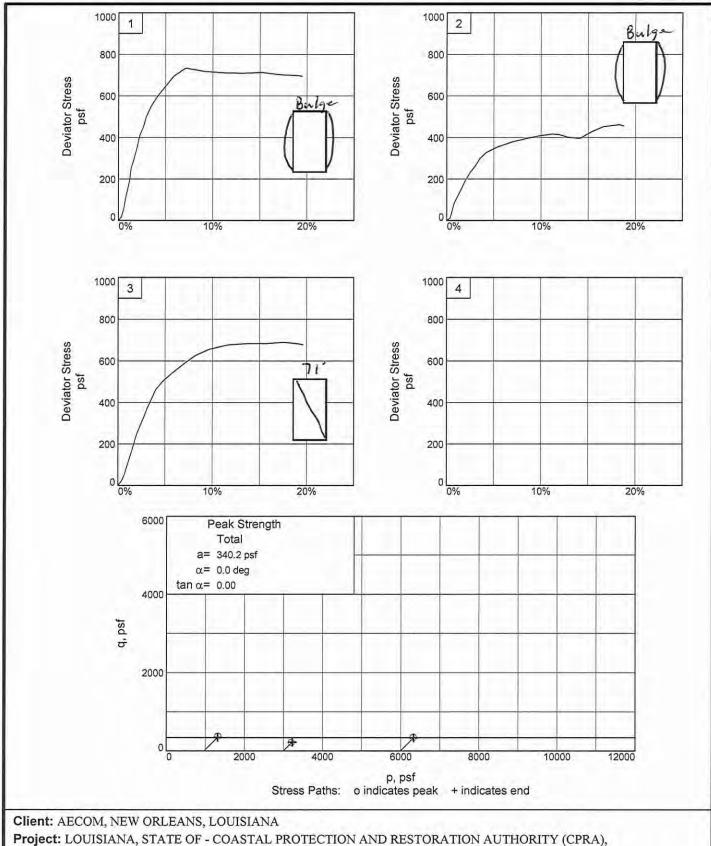
Client: AECOM, NEW ORLEANS, LOUISIANA

Sample Number: 7C

Proj. No.: 24384 Date Sampled: 9/15/20



Figure ASTM D2850



Source of Sample: SB-07

Depth: 26

Sample Number: 7C

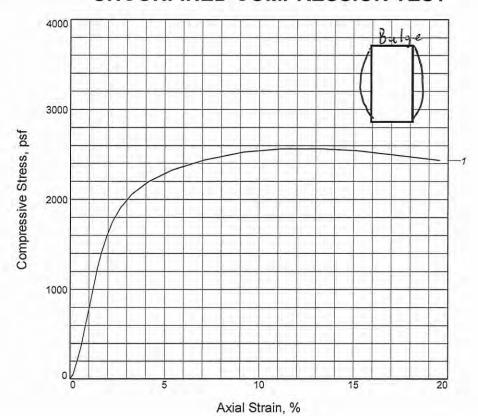
Project No.: 24384

Figure ASTM D2850

Eustis Engineering L.L.C.

Tested By: CC

## **UNCONFINED COMPRESSION TEST**



Specimen No.	1	
Unconfined strength, psf	2566.0	
Undrained shear strength, psf	1283.0	1.
Failure strain, %	13.2	
Strain rate, %/min.	1.00	
Water content, %	24.7	
Wet density, pcf	126.6	
Dry density, pcf	101.5	
Saturation, %	100.1	
Void ratio	0.6725	
Specimen diameter, in.	1.386	
Specimen height, in.	2.726	
Height/diameter ratio	1.97	

Description: ST LGR & T CH3 W/ ARS ML

LL = 56 PL = 20 Pl = 36 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 9/17/20

Date Sampled. 9/1//20

Remarks: TORVANE = 0.750 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

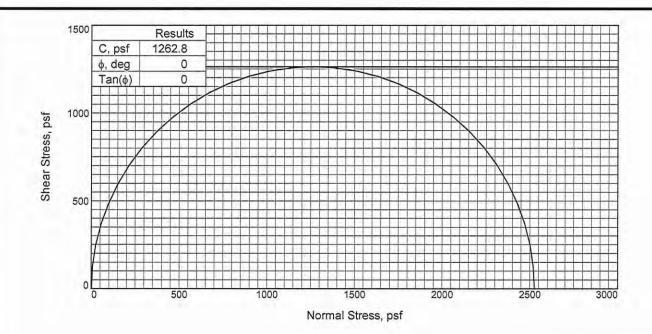
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-07 Depth: 30

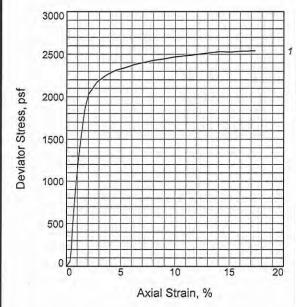
Sample Number: 8C1

EUSTIS ENGINEERING SINGL 1946

Figure ASTM D2166



Specimen No.



-6	••••••		
Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	23.9 100.9 95.0 0.6836 2.858 5.772	
AtTest	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	25.1 100.9 100.0 0.6836 2.858 5.772	
Str	ain rate, %/min.	0.50	
Ba	ck Pressure, psi	0.000	
Ce	l Pressure, psi	0.000	
Fai	I. Stress, psf	2525.6	
5	Strain, %	15.1	
Ult.	Stress, psf	2525.6	
5	Strain, %	15.1	
σ1	Failure, psf	2525.6	
$\sigma_3$	Failure, psf	0.0	

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

1

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: ST LGR & T CH3 W/ ARS ML

LL= 56

PL= 20

PI= 36

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.775 TSF

Sample Number: 8C2

Source of Sample: SB-07

Proj. No.: 24384

Date Sampled: 9/17/20



Client: AECOM, NEW ORLEANS, LOUISIANA

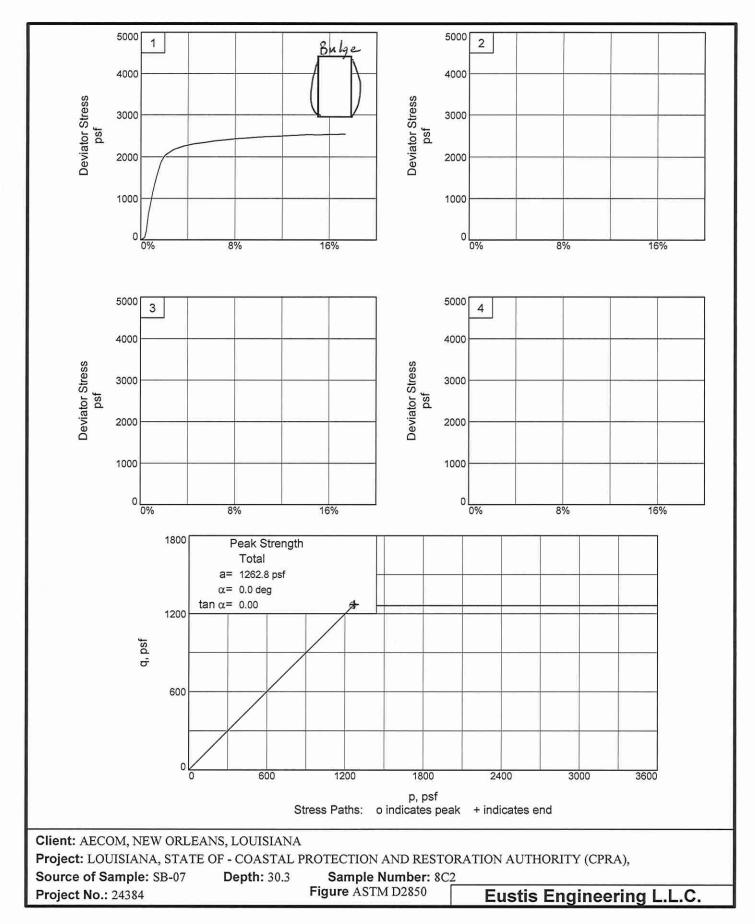
RESTORATION AUTHORITY (CPRA),

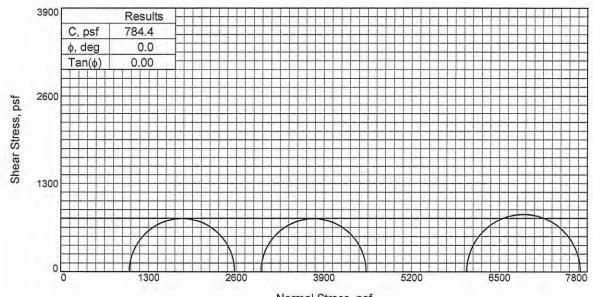
EUSTIS

SINCE 1946

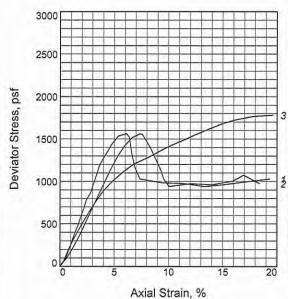
Depth: 30.3

Figure ASTM D2850





Normal Stress, psf



			_	
Type	of	Test:		

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: M GR & T CL6

LL= 41 PL= 13 PI= 28

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.375 TSF

Spe	ecimen No.	1	2	3	
	Water Content, %	28.1	29.7	30.0	
	Dry Density, pcf	95.9	93.5	93.2	
Initial	Saturation, %	100.0	100.0	100.0	
Ξ	Void Ratio	0.7581	0.8022	0.8091	
	Diameter, in.	1.394			
	Height, in.	2.743	2.783	2.727	
	Water Content, %	28.1	29.7	30.0	
44	Dry Density, pcf	95.9	93.5	93.2	
At Test	Saturation, %	100.0	100.0	100.0	
1	Void Ratio	0.7581	0.8022	0.8091	
•	Diameter, in.	1.394	1.380		
	Height, in.	2.743	2.783	2.727	
Str	ain rate, %/min.	1.00	1.00	1.00	
Bad	ck Pressure, psi	0.000	0.000	0.000	
Cel	Il Pressure, psi	7.040	20.590	41.760	
Fai	I. Stress, psf	1562.5	1560.2	1684.3	
5	Strain, %	6.1	7.4	15.0	
Ult.	Stress, psf	940.4	940.4	1684.3	
5	Strain, %	13.3	10.0	15.0	
σ1	Failure, psf	2576.3	4525.1	7697.8	
σ3	Failure, psf	1013.8	2965.0	6013.4	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-07 Depth: 37

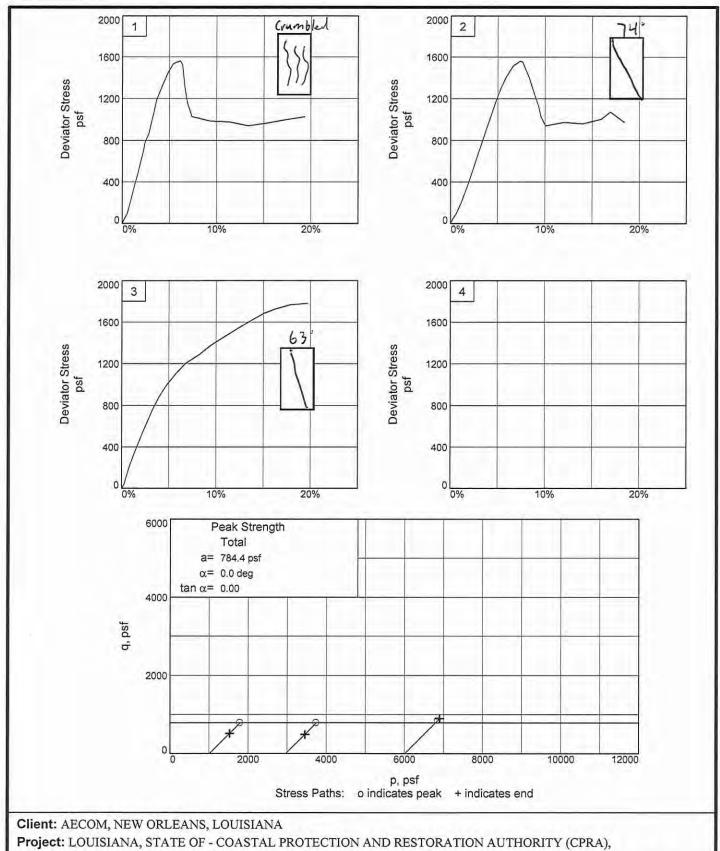
Sample Number: 10B

Proj. No.: 24384 Date Sampled: 9/15/20



Figure ASTM D2850

Tested By: CC



Source of Sample: SB-07

Depth: 37

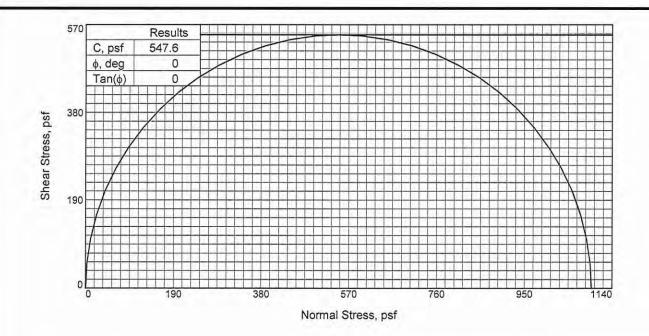
Sample Number: 10B

Project No.: 24384

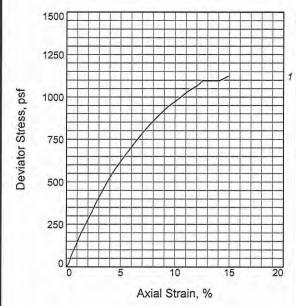
Figure ASTM D2850

Eustis Engineering L.L.C.

Tested By: CC



Specimen No.



	Water Content, %	30.9	
_	Dry Density, pcf	91.5	
Initial	Saturation, %	98.9	
=	Void Ratio	0.8429	
	Diameter, in.	2.879	
	Height, in.	5.750	
	Water Content, %	31.2	
4	Dry Density, pcf	91.5	
es	Saturation, %	100.0	
At Test	Void Ratio	0.8429	
4	Diameter, in.	2.879	
	Height, in.	5.750	
Str	ain rate, %/min.	0.50	
Ba	ck Pressure, psi	0.000	
Ce	Il Pressure, psi	0.000	
Fai	il. Stress, psf	1095.2	
1	Strain, %	12.5	
Ult	. Stress, psf	1095.2	
1	Strain, %	12.5	
σ1	Failure, psf	1095.2	
$\sigma_3$	Failure, psf	0.0	

1

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED

Description: MTCL4

LL= 38 PL= 19 Pl= 19

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.350 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

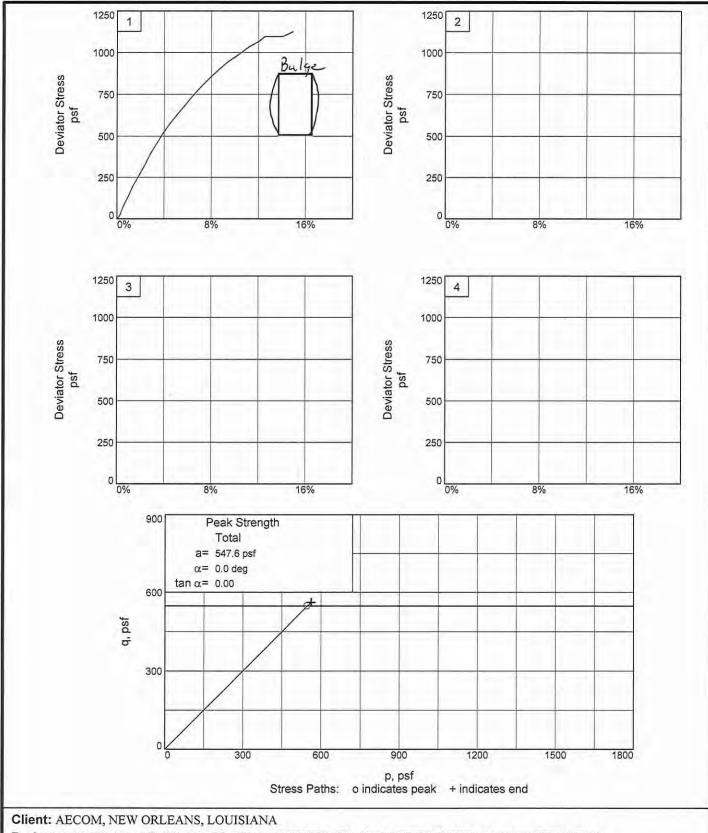
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-07 Depth: 38

Sample Number: 10C

Proj. No.: 24384 Date Sampled: 10/7/20



Figure ASTM D2850



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA),

Source of Sample: SB-07

Depth: 38

Sample Number: 10C

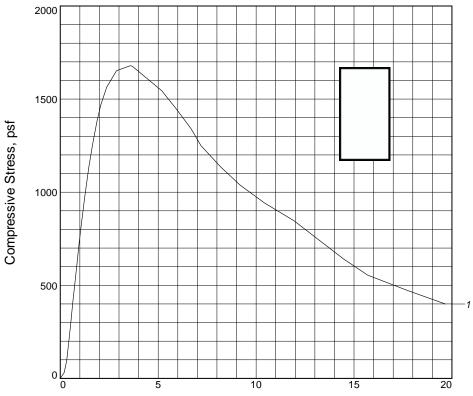
Figure ASTM D2850

Eustis Engineering L.L.C.

Tested By: BH

Project No.: 24384

## UNCONFINED COMPRESSION TEST



Specimen No.	1		
Unconfined strength, psf	1679.0		
Undrained shear strength, psf	839.5		
Failure strain, %	3.6		
Strain rate, %/min.	1.00		
Water content, %	28.8		
Wet density, pcf	117.0		
Dry density, pcf	90.9		
Saturation, %	90.2		
Void ratio	0.8685		
Specimen diameter, in.	1.393		
Specimen height, in.	2.769		
Height/diameter ratio	1.99		

Axial Strain, %

**Description:** M GR & T CH3 W/ ARS ML, RT, SL

LL = 61 PL = 19 PI = 42 Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

**Date Sampled:** 9/21/20

Remarks:

TORVANE = 0.750 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

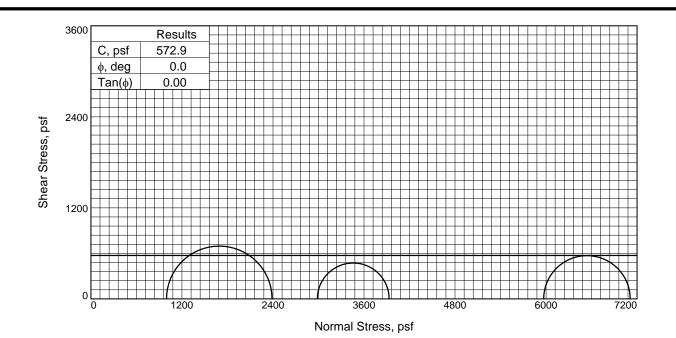
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-08 **Depth:** 5

Sample Number: 2B

EUSTIS ENGINEERING SINCE 1946

Figure ASTM D2166





Deviator Stress, pg. 1000

Axial Strain, %

Type of Test:

250

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: M GR & BR CH0A

**LL=** 144 **PL=** 45 **PI=** 99

**Assumed Specific Gravity=** 2.74 **Remarks:** TORVANE = 0.750 TSF

Figure AS	TM D2850
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1	Spe	ecimen No.	1	2	3	
		Water Content, %	71.8	74.8	70.8	
		Dry Density, pcf	55.5	53.9	55.4	
	Initial	Saturation, %	94.6	94.3	92.9	
	<u>=</u>	Void Ratio	2.0809	2.1746	2.0890	
3		Diameter, in.	1.403	1.400	1.399	
2		Height, in.	2.754	2.757	2.761	
		Water Content, %	75.9	79.4	76.2	
	72	Dry Density, pcf	55.5	53.9	55.4	
	At Test	Saturation, %	100.0	100.0	100.0	
	<u>'</u> =	Void Ratio	2.0809	2.1746	2.0890	
	_	Diameter, in.	1.403	1.400	1.399	
		Height, in.	2.754	2.757	2.761	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.920	20.730	41.450	
	Fai	I. Stress, psf	1391.6	946.2	1140.9	
	5	Strain, %	14.8	13.6	8.8	
	Ult.	Stress, psf	1391.6	946.2	1041.8	
	5	Strain, %	14.8	13.6	14.3	
7	$\sigma_1$	Failure, psf	2388.1	3931.3	7109.7	
	$\sigma_3$	Failure, psf	996.5	2985.1	5968.8	

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

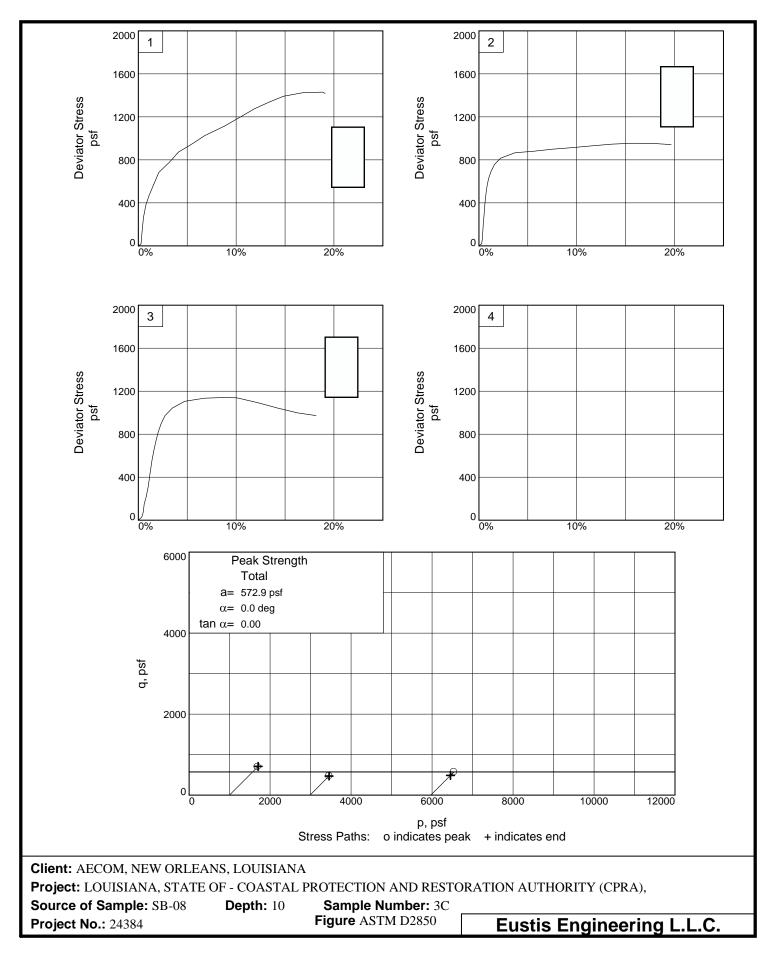
RESTORATION AUTHORITY (CPRA),

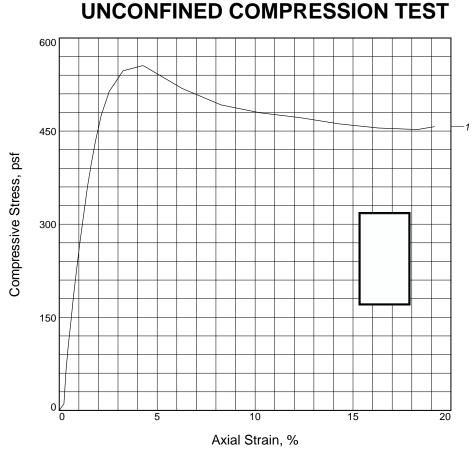
Source of Sample: SB-08 Depth: 10

Sample Number: 3C

**Proj. No.:** 24384 **Date Sampled:** 9/21/20







Specimen No.	1	
Unconfined strength, psf	555.5	
Undrained shear strength, psf	277.7	
Failure strain, %	4.3	
Strain rate, %/min.	1.00	
Water content, %	58.2	
Wet density, pcf	102.9	
Dry density, pcf	65.0	
Saturation, %	98.3	
Void ratio	1.6111	
Specimen diameter, in.	1.397	
Specimen height, in.	2.761	
Height/diameter ratio	1.98	

Description: SO GR CH4 W/ ARS ML, SL

**LL =** 82 **PL** = 19 **PI** = 63 **Assumed GS=** 2.72 **Type:** UNDISTURBED

Project No.: 24384 **Date Sampled:** 9/21/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

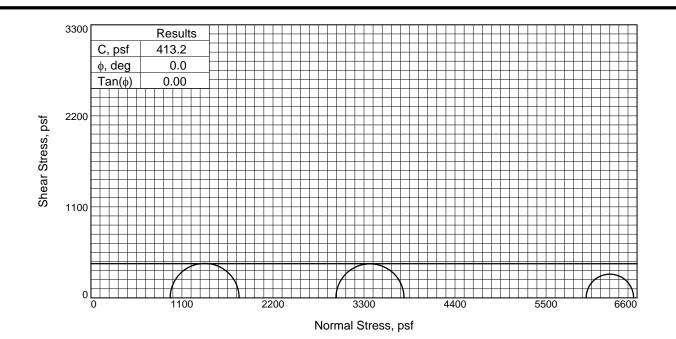
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

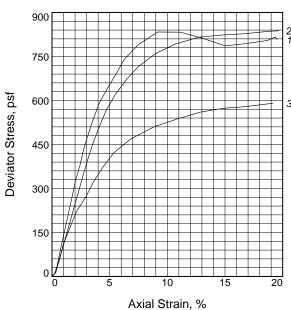
RESTORATION AUTHORITY (CPRA), **Source of Sample: SB-08** 

Sample Number: 4C

Figure ASTM D2166

Tested By: CC





Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: SO GR CL6 W/ ARS SP

**LL=** 42 **PL=** 19 **PI=** 23

**Assumed Specific Gravity=** 2.70 **Remarks:** TORVANE = 0.150 TSF

Figure ASTM D2850	į
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2	Specimen No.		1	2	3	
2		Water Content, % Dry Density, pcf	39.6 81.3	40.0 80.8	40.2 80.8	
	Initial	Saturation, %	99.6	99.2	99.9	
	П	Void Ratio	1.0727	1.0872	1.0855	
3		Diameter, in.	1.412	1.386	1.395	
		Height, in.	2.779	2.763	2.763	
		Water Content, %	39.7	40.3	40.2	
	;;	Dry Density, pcf	81.3	80.8	80.8	
	At Test	Saturation, %	100.0	100.0	100.0	
		Void Ratio	1.0727	1.0872	1.0855	
	1	Diameter, in.	1.412	1.386	1.395	
		Height, in.	2.779	2.763	2.763	
	Strain rate, %/min.		1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.650	20.560	41.560	
	Fail. Stress, psf Strain, %		833.5	823.2	573.5	
			9.2	14.8	14.9	
	Ult.	Ult. Stress, psf		823.2	573.5	
	Strain, %		15.0	14.8	14.9	
	σ <sub>1</sub> Failure, psf		1791.1	3783.8	6558.1	
	$\sigma_3$	Failure, psf	957.6	2960.6	5984.6	

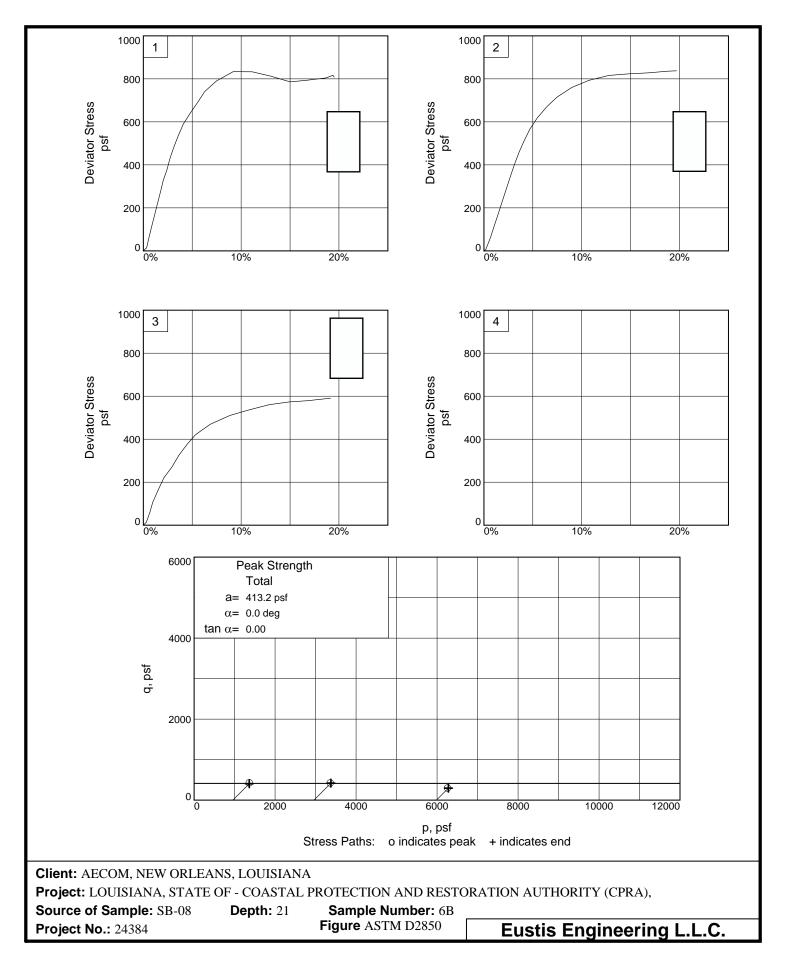
Client: AECOM, NEW ORLEANS, LOUISIANA

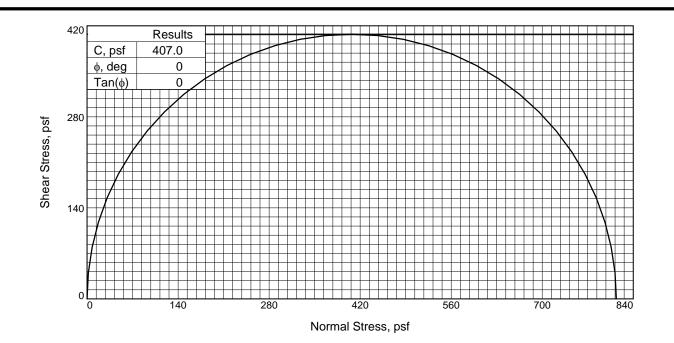
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

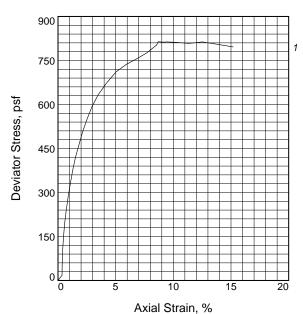
Sample Number: 6B

**Proj. No.:** 24384 **Date Sampled:** 10/8/20









Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: SO GR & T CL6 W/ CC

**LL=** 48 **PL=** 16 **PI=** 32

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.750 TSF

Figure	ASTM	D2850
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Spe	ecimen No.	1	
	Water Content, %	26.5	
	Dry Density, pcf	97.6	
nitial	Saturation, %	97.5	
<u>=</u>	Void Ratio	0.7389	
	Diameter, in.	2.854	
	Height, in.	5.770	
	Water Content, %	27.2	
#	Dry Density, pcf	97.6	
At Test	Saturation, %	100.0	
	Void Ratio	0.7389	
`	Diameter, in.	2.854	
	Height, in.	5.770	
Str	ain rate, %/min.	0.50	
Bad	ck Pressure, psi	0.000	
Cel	Il Pressure, psi	0.000	
Fai	I. Stress, psf	814.0	
5	Strain, %	8.7	
Ult.	Stress, psf	814.0	
5	Strain, %	8.7	
σ₁	Failure, psf	814.0	
$\sigma_3$	Failure, psf	0.0	

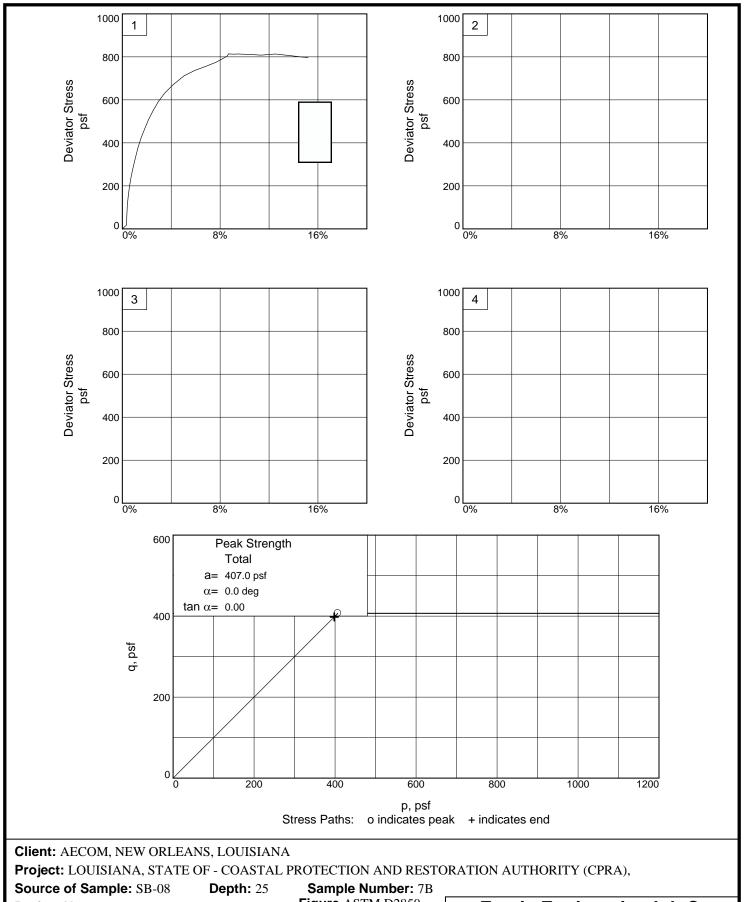
Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

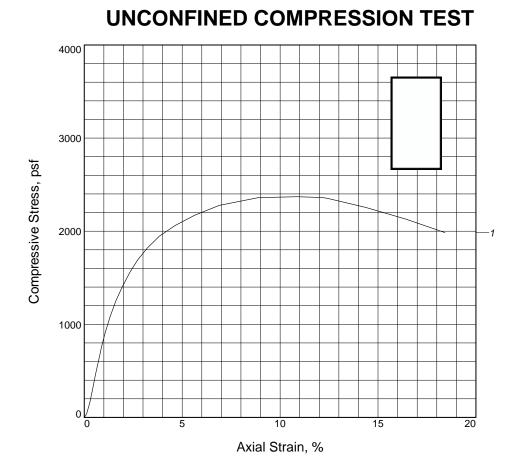
Sample Number: 7B

**Proj. No.:** 24384 **Date Sampled:** 10/7/20





Project No.: 24384 Figure ASTM D2850 Eustis Engineering L.L.C.



Specimen No.	1	
Unconfined strength, psf	2370.2	
Undrained shear strength, psf	1185.1	
Failure strain, %	10.9	
Strain rate, %/min.	1.00	
Water content, %	23.2	
Wet density, pcf	124.7	
Dry density, pcf	101.2	
Saturation, %	93.1	
Void ratio	0.6774	
Specimen diameter, in.	1.383	
Specimen height, in.	2.766	
Height/diameter ratio	2.00	

**Description:** ST LGR & T CH2 W/ ARS ML

LL = 57 PL = 20 PI = 37 Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/21/20

Remarks:

TORVANE = 0.875 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

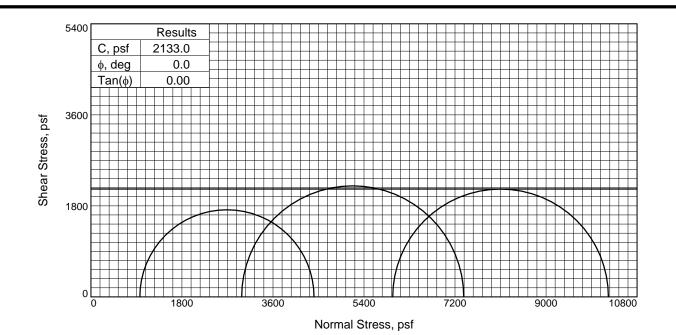
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-08 **Depth:** 26

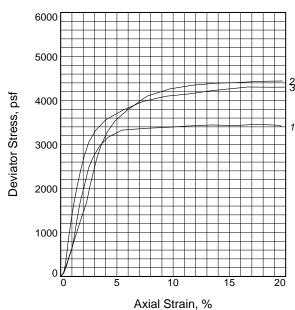
Sample Number: 7C

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Figure ASTM D2166

Tested By: CC





Unconsolidated Undrained Sample Type: UNDISTURBED

**Description:** VST LTG & T CH3 W/ ARS ML, CC,

SL

**LL=** 62 **PL=** 19 **PI=** 43

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 1.000 TSF

	Spe	ecimen No.	1	2	3	
		Water Content, % Dry Density, pcf	24.6 99.7	25.3 100.7	25.4 100.2	
	Initial	Saturation, %	95.2	100.7	99.2	
3	<u>=</u>	Void Ratio	0.7028	0.6864	0.6955	
		Diameter, in.	1.395	1.389	1.390	
		Height, in.	2.736	2.765	2.758	
1		Water Content, %	25.8	25.2	25.6	
	#	Dry Density, pcf	99.7	100.7	100.2	
	<u></u>	Saturation, %	100.0	100.0	100.0	
	At Test	Void Ratio	0.7028	0.6864	0.6955	
		Diameter, in.	1.395	1.389	1.390	
		Height, in.	2.736	2.765	2.758	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.720	20.700	41.450	
	Fai	I. Stress, psf	3444.1	4391.0	4264.0	
	5	Strain, %	13.5	13.7	15.0	
	Ult. Stress, psf		3444.1	4391.0	4264.0	
	5	Strain, %	13.5	13.7	15.0	
	$\sigma_1$	Failure, psf	4411.8	7371.8	10232.8	
	$\sigma_3$	Failure, psf	967.7	2980.8	5968.8	

Client: AECOM, NEW ORLEANS, LOUISIANA

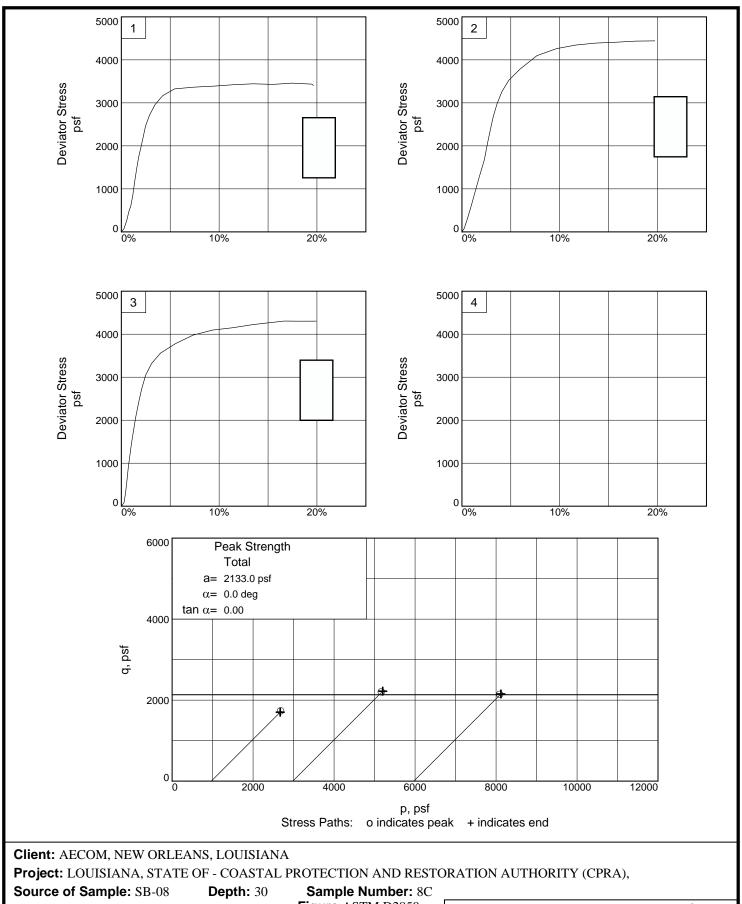
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

Sample Number: 8C

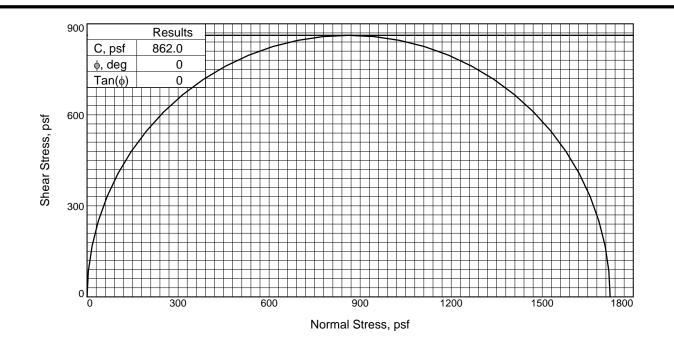
**Proj. No.:** 24384 **Date Sampled:** 9/21/20

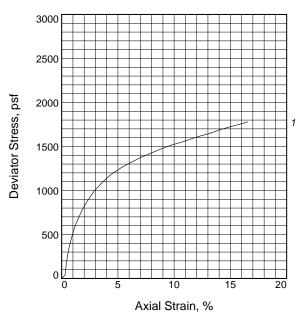


Figure ASTM D2850



Project No.: 24384 Figure ASTM D2850 Eustis Engineering L.L.C.





Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: M T & GR CH4 W/ ARS & LNS ML,

CC

**LL=** 78 **PL=** 22 **PI=** 56

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.750 TSF

Spe	ecimen No.	1	
	Water Content, % Dry Density, pcf	29.0 92.1	
nitial	Saturation, %	93.7	
=	Void Ratio Diameter, in.	0.8431 2.853	
	Height, in.	5.737	
	Water Content, %	31.0	
est	Dry Density, pcf Saturation, %	92.1 100.0	
At Test	Void Ratio	0.8431	
	Diameter, in. Height, in.	2.853 5.737	
Str	ain rate, %/min.	0.50	
Bad	ck Pressure, psi	0.000	
Cel	II Pressure, psi	0.000	
Fai	I. Stress, psf	1723.9	
5	Strain, %	15.0	
Ult.	. Stress, psf	1723.9	
8	Strain, %	15.0	
σ <sub>1</sub>	Failure, psf	1723.9	
$\sigma_3$	Failure, psf	0.0	

Client: AECOM, NEW ORLEANS, LOUISIANA

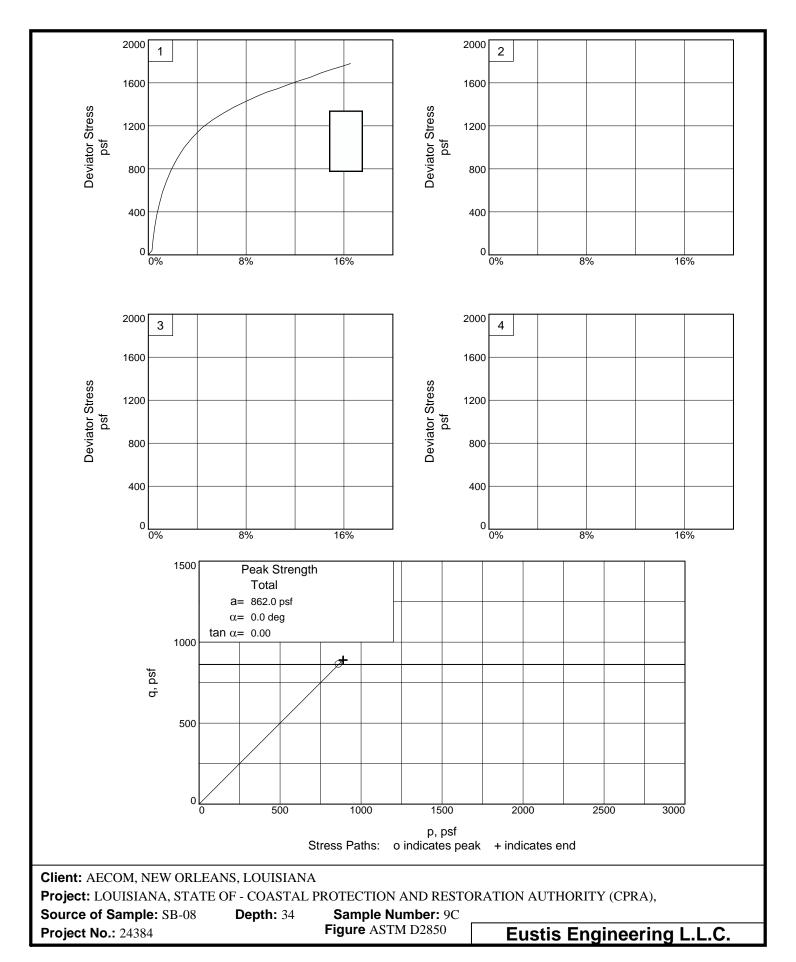
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

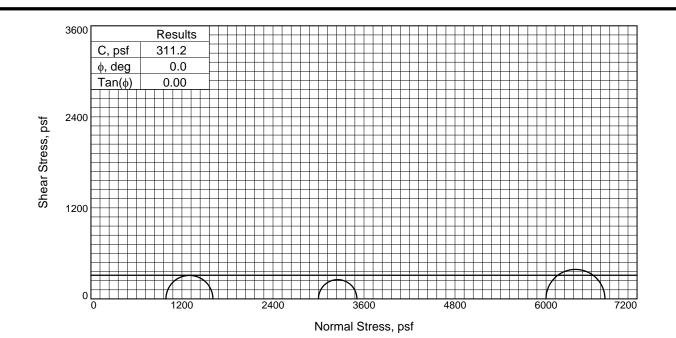
Sample Number: 9C

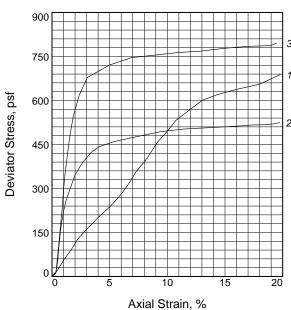
**Proj. No.:** 24384 **Date Sampled:** 10/7/20



Figure ASTM D2850







Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: SO GR & T CH4 W/ ARS & LNS ML,

CC

**LL=** 98 **PL=** 30 **PI=** 68

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.625 TSF

	Spe	ecimen No.	1	2	3	
3		Water Content, %	51.5	53.5	53.0	
	_	Dry Density, pcf	70.3	68.8	68.1	
1	Initial	Saturation, %	98.9	99.2	96.4	
	<u>-</u>	Void Ratio	1.4169	1.4675	1.4952	
		Diameter, in.	1.403	1.384	1.385	
2		Height, in.	2.757	2.773	2.753	
2		Water Content, %	52.1	54.0	55.0	
	#	Dry Density, pcf	70.3	68.8	68.1	
	At Test	Saturation, %	100.0	100.0	100.0	
	=	Void Ratio	1.4169	1.4675	1.4952	
	1	Diameter, in.	1.403	1.384	1.385	
		Height, in.	2.757	2.773	2.753	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bac	ck Pressure, psi	0.000	0.000	0.000	
	Cell Pressure, psi Fail. Stress, psf Strain, %		6.870	20.830	41.660	
			619.2	507.0	777.7	
			14.3	13.5	14.9	
	Ult.	Stress, psf	619.2	507.0	777.7	
	Strain, %		14.3	13.5	14.9	
	σ₁ Failure, psf		1608.5	3506.5	6776.7	
	$\sigma_3$	Failure, psf	989.3	2999.5	5999.0	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

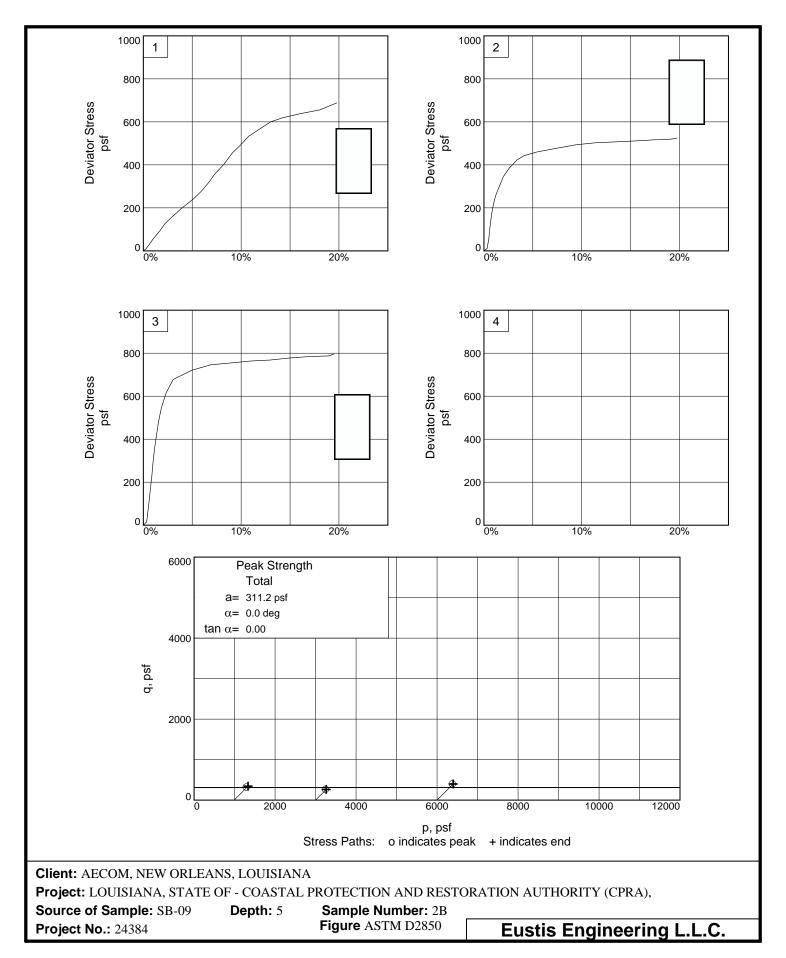
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 5

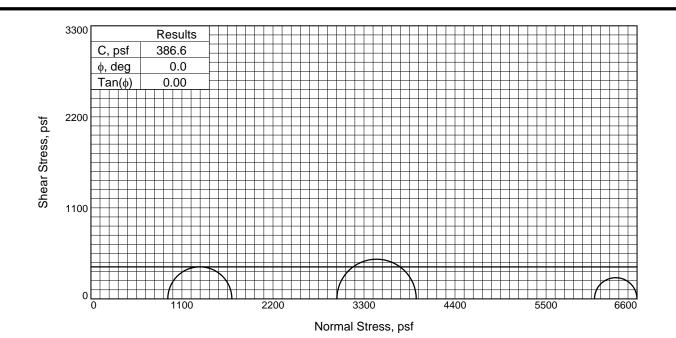
Sample Number: 2B

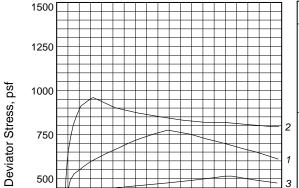
**Proj. No.:** 24384 **Date Sampled:** 9/21/20



Figure ASTM D2850







Axial Strain, %

Type of Test:

250

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: SO GR CHOA

**LL=** 130 **PL=** 32 **PI=** 98

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.375 TSF

Figure ASTM D2850
-------------------

	Spe	ecimen No.	1	2	3	
		Water Content, %	91.2	93.9	89.9	
		Dry Density, pcf	46.9	46.5	47.9	
	nitial	Saturation, %	94.8	96.4	96.2	
	Ξ	Void Ratio	2.6179	2.6494	2.5442	
		Diameter, in.	1.437	1.412	1.424	
		Height, in.	2.754	2.755	2.755	
,		Water Content, %	96.2	97.4	93.5	
-	st	Dry Density, pcf	46.9	46.5	47.9	
	Tes	Saturation, %	100.0	100.0	100.0	
1	At 1	Void Ratio	2.6179	2.6494	2.5442	
3	1	Diameter, in.	1.437	1.412	1.424	
		Height, in.	2.754	2.755	2.755	
	Strain rate, %/min.		1.00	1.00	1.00	
	Back Pressure, psi		0.000	0.000	0.000	
	Cell Pressure, psi		6.450	20.640	42.260	
	Fail. Stress, psf		774.7	959.4	513.0	
	Strain, %		9.9	3.3	14.9	
	Ult. Stress, psf		695.1	816.7	513.0	
	Strain, %		15.2	15.2	14.9	
	$\sigma_1$	Failure, psf	1703.5	3931.6	6598.4	
	$\sigma_3$	Failure, psf	928.8	2972.2	6085.4	

Client: AECOM, NEW ORLEANS, LOUISIANA

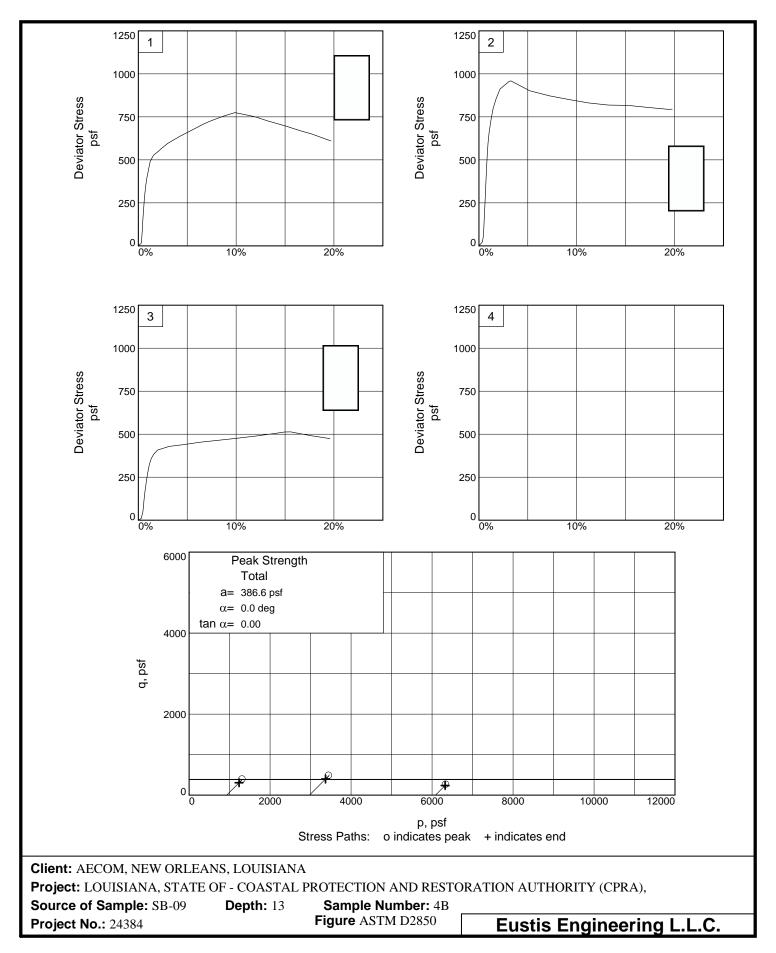
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

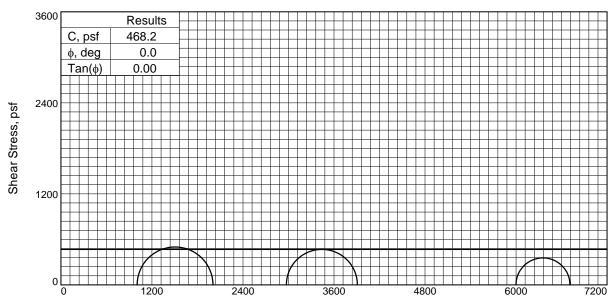
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 13

Sample Number: 4B

**Proj. No.:** 24384 **Date Sampled:** 10/8/20

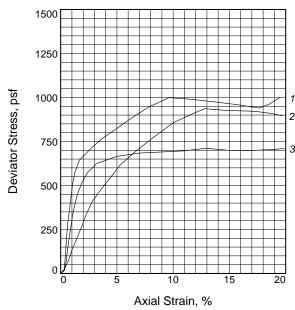






Normal Stress, psf

Specimen No.



		Water Content, %	37.6	36.2	38.6	
		Dry Density, pcf	80.8	84.1	78.4	
	ial	Saturation, %	93.0	96.4	90.1	
	Initial	Void Ratio	1.1011	1.0203	1.1659	
1		Diameter, in.	1.424	1.408	1.435	
2		Height, in.	2.734	2.746	2.766	
_		Water Content, %	40.5	37.5	42.9	
3	+=	Dry Density, pcf	80.8	84.1	78.4	
	At Test	Saturation, %	100.0	100.0	100.0	
		Void Ratio	1.1011	1.0203	1.1659	
	1	Diameter, in.	1.424	1.408	1.435	
		Height, in.	2.734	2.746	2.766	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.980	20.640	41.660	
	Fail. Stress, psf		999.5	936.4	711.6	
	Strain, %		9.6	12.8	13.0	
	Ult. Stress, psf		974.9	926.4	699.2	
	5	Strain, %	13.6	14.7	15.0	
	σ <sub>1</sub>	Failure, psf	2004.6	3908.5	6710.7	
	$\sigma_3$	Failure, psf	1005.1	2972.2	5999.0	

1

2

3

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: SO GR CH3 W/ ARS ML

**LL=** 64 **PL=** 23 **PI=** 41

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.150 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

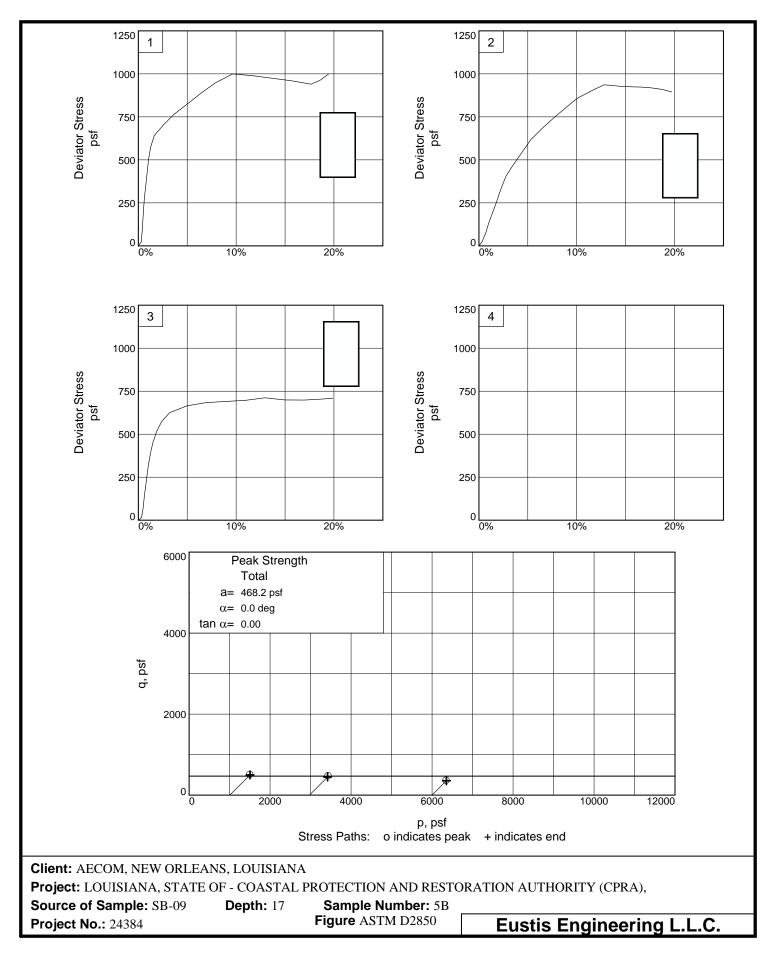
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 17

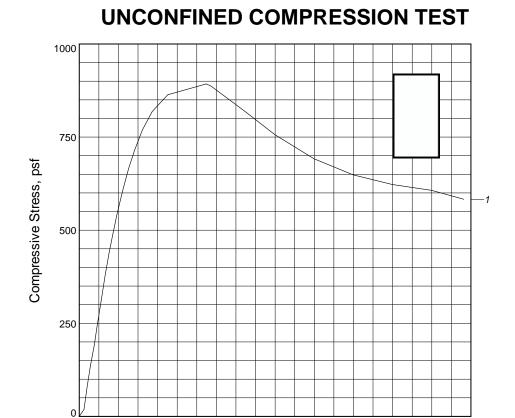
Sample Number: 5B

**Proj. No.:** 24384 **Date Sampled:** 9/21/20



Figure ASTM D2850





Avial	Strain.	0/
Axiai	oualli.	70

Specimen No.	1	
Unconfined strength, psf	892.7	
Undrained shear strength, psf	446.4	
Failure strain, %	6.5	
Strain rate, %/min.	1.00	
Water content, %	32.6	
Wet density, pcf	114.5	
Dry density, pcf	86.4	
Saturation, %	91.8	
Void ratio	0.9661	
Specimen diameter, in.	1.414	
Specimen height, in.	2.747	
Height/diameter ratio	1.94	

**Description:** SO GR CL4

LL = 41 PL = 17 PI = 24 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 9/21/20

Remarks:

TORVANE = 0.500 TSF

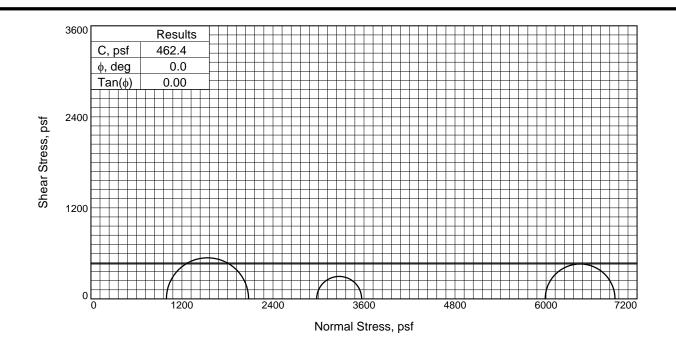
Client: AECOM, NEW ORLEANS, LOUISIANA

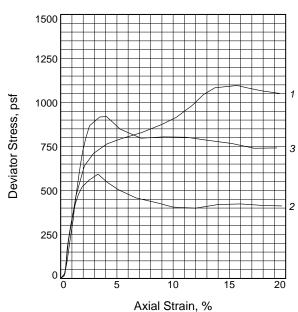
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

Sample Number: 6B

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Figure ASTM D2166





Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR CH3 W/ ARS ML, SIF

**LL=** 67 **PL=** 16 **PI=** 51

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.500 TSF

marks: TORVANE = 0.500 TSF	Sa
	Pr

Spe	ecimen No.	1	2	3	
	Water Content, %	53.5 66.8	51.1 69.5	53.6 67.8	
<u>_</u>	Dry Density, pcf Saturation, %	94.4	96.1	97.0	
Initia	Void Ratio	1.5436	1.4446	1.5035	
_	Diameter, in.	1.411	1.406	1.385	
	Height, in.	2.754	2.756	2.784	
	Water Content, %	56.8	53.1	55.3	
#	Dry Density, pcf	66.8	69.5	67.8	
At Test	Saturation, %	100.0	100.0	100.0	
=	Void Ratio	1.5436	1.4446	1.5035	
~	Diameter, in.	1.411	1.406	1.385	
	Height, in.	2.754	2.756	2.784	
Stra	ain rate, %/min.	1.00	1.00	1.00	
Bad	ck Pressure, psi	0.000	0.000	0.000	
Cel	l Pressure, psi	6.910	20.670	41.590	
Fai	I. Stress, psf	1083.5	592.6	920.2	
8	Strain, %	13.7	3.3	4.1	
Ult.	Stress, psf	1083.5	399.3	767.3	
5	Strain, %	13.7	12.0	15.2	
σ₁	Failure, psf	2078.5	3569.1	6909.1	
$\sigma_3$	Failure, psf	995.0	2976.5	5989.0	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

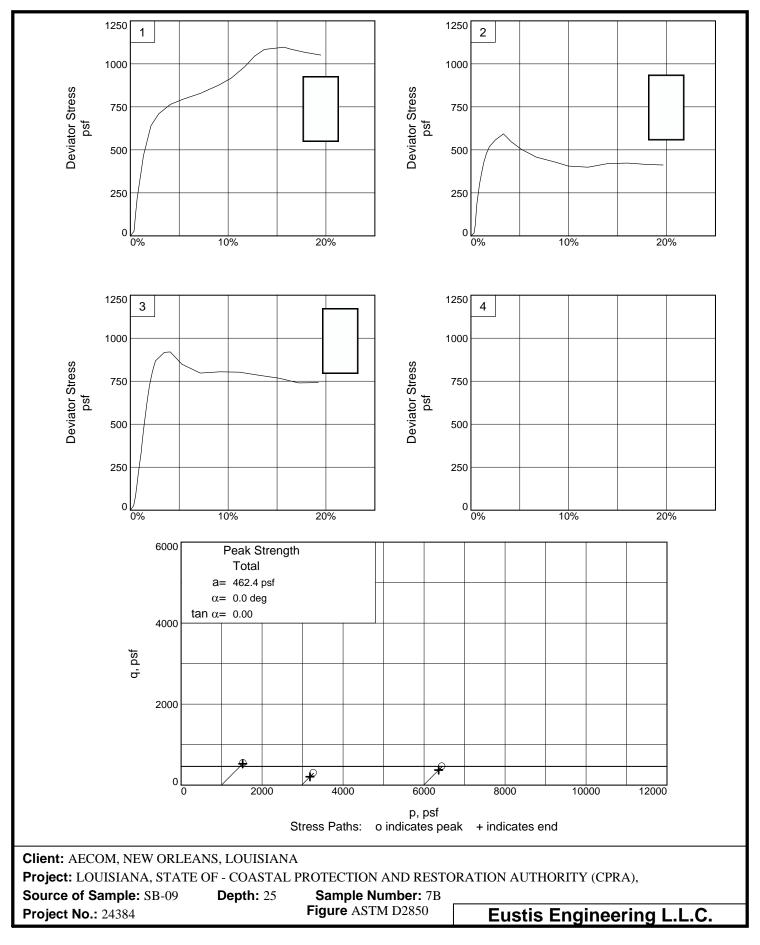
Source of Sample: SB-09 Depth: 25

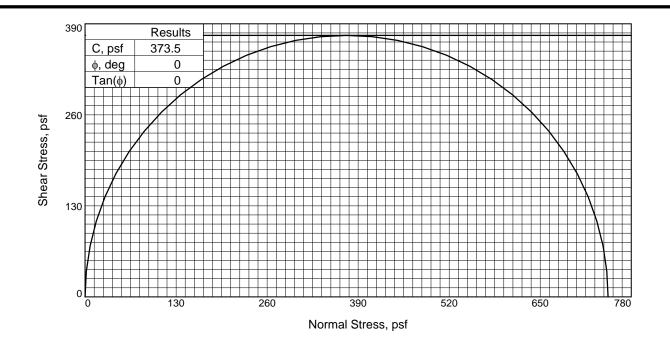
Sample Number: 7B

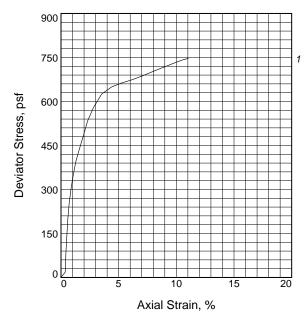
Proj. No.: 24384 Date Sampled: 9/21/20



Figure ASTM D2850







Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: SO GR CH2 W/ ARS & LNS ML

**LL=** 54 **PL=** 22 **PI=** 32

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.200 TSF

Specimen No.				
Dry Density, pcf   71.3   Saturation, %   100.0   Void Ratio   1.3822   Diameter, in.   2.850   Height, in.   5.784	Specimen No.		1	
Saturation, %   100.0     Void Ratio   1.3822     Diameter, in.   2.850     Height, in.   5.784     Water Content, %   50.8     Dry Density, pcf   71.3     Saturation, %   100.0     Void Ratio   1.3822     Diameter, in.   2.850     Height, in.   5.784     Strain rate, %/min.   0.50     Back Pressure, psi   0.000     Cell Pressure, psi   0.000     Fail. Stress, psf   746.9     Strain, %   11.0     Ult. Stress, psf   746.9     Strain, %   11.0     G1   Failure, psf   746.9		Water Content, %	50.8	
Diameter, in.   2.850     Height, in.   5.784     Water Content, %   50.8     Dry Density, pcf   71.3     Saturation, %   100.0     Void Ratio   1.3822     Diameter, in.   2.850     Height, in.   5.784     Strain rate, %/min.   0.50     Back Pressure, psi   0.000     Cell Pressure, psi   0.000     Fail. Stress, psf   746.9     Strain, %   11.0     Ult. Stress, psf   746.9     Strain, %   11.0     σ₁ Failure, psf   746.9		Dry Density, pcf	71.3	
Diameter, in.   2.850     Height, in.   5.784     Water Content, %   50.8     Dry Density, pcf   71.3     Saturation, %   100.0     Void Ratio   1.3822     Diameter, in.   2.850     Height, in.   5.784     Strain rate, %/min.   0.50     Back Pressure, psi   0.000     Cell Pressure, psi   0.000     Fail. Stress, psf   746.9     Strain, %   11.0     Ult. Stress, psf   746.9     Strain, %   11.0     σ₁ Failure, psf   746.9	lä.	Saturation, %	100.0	
Height, in.   5.784     Water Content, %   50.8     Dry Density, pcf   71.3     Saturation, %   100.0     Void Ratio   1.3822     Diameter, in.   2.850     Height, in.   5.784     Strain rate, %/min.   0.50     Back Pressure, psi   0.000     Cell Pressure, psi   0.000     Fail. Stress, psf   746.9     Strain, %   11.0     Ult. Stress, psf   746.9     Strain, %   11.0     σ₁ Failure, psf   746.9	'=	Void Ratio	1.3822	
Water Content, %   50.8     Dry Density, pcf   71.3     Saturation, %   100.0     Void Ratio   1.3822     Diameter, in.   2.850     Height, in.   5.784     Strain rate, %/min.   0.50     Back Pressure, psi   0.000     Cell Pressure, psi   0.000     Fail. Stress, psf   746.9     Strain, %   11.0     Ult. Stress, psf   746.9     Strain, %   11.0     Tailure, psf   746.9		Diameter, in.		
Dry Density, pcf   71.3   Saturation, %   100.0   Void Ratio   1.3822   Diameter, in.   2.850   Height, in.   5.784      Strain rate, %/min.   0.50   Back Pressure, psi   0.000   Cell Pressure, psi   0.000   Fail. Stress, psf   746.9   Strain, %   11.0   Ult. Stress, psf   746.9   Strain, %   11.0   Tailure, psf   746.9   Tailure, psf   Tailure, psf		Height, in.	5.784	
Saturation, %   100.0		Water Content, %	50.8	
Diameter, in.   2.850     Height, in.   5.784     Strain rate, %/min.   0.50     Back Pressure, psi   0.000     Cell Pressure, psi   0.000     Fail. Stress, psf   746.9     Strain, %   11.0     Ult. Stress, psf   746.9     Strain, %   11.0     Tailure, psf   746.9	بر ا	Dry Density, pcf	71.3	
Diameter, in.   2.850     Height, in.   5.784     Strain rate, %/min.   0.50     Back Pressure, psi   0.000     Cell Pressure, psi   0.000     Fail. Stress, psf   746.9     Strain, %   11.0     Ult. Stress, psf   746.9     Strain, %   11.0     Tailure, psf   746.9	ĕ	Saturation, %	100.0	
Diameter, in.   2.850     Height, in.   5.784     Strain rate, %/min.   0.50     Back Pressure, psi   0.000     Cell Pressure, psi   0.000     Fail. Stress, psf   746.9     Strain, %   11.0     Ult. Stress, psf   746.9     Strain, %   11.0     Tailure, psf   746.9	'≠	Void Ratio		
Strain rate, %/min.       0.50         Back Pressure, psi       0.000         Cell Pressure, psi       0.000         Fail. Stress, psf       746.9         Strain, %       11.0         Ult. Stress, psf       746.9         Strain, %       11.0         σ <sub>1</sub> Failure, psf       746.9	`	'		
Back Pressure, psi       0.000         Cell Pressure, psi       0.000         Fail. Stress, psf       746.9         Strain, %       11.0         Ult. Stress, psf       746.9         Strain, %       11.0         σ <sub>1</sub> Failure, psf       746.9		Height, in.	5.784	
Cell Pressure, psi       0.000         Fail. Stress, psf       746.9         Strain, %       11.0         Ult. Stress, psf       746.9         Strain, %       11.0         σ₁ Failure, psf       746.9	Str	ain rate, %/min.	0.50	
Fail. Stress, psf       746.9         Strain, %       11.0         Ult. Stress, psf       746.9         Strain, %       11.0         σ₁ Failure, psf       746.9	Ba	ck Pressure, psi	0.000	
Strain, %       11.0         Ult. Stress, psf       746.9         Strain, %       11.0         σ₁ Failure, psf       746.9	Ce	ll Pressure, psi	0.000	
Ult. Stress, psf $746.9$ Strain, % $11.0$ $\sigma_1$ Failure, psf $746.9$	Fail. Stress, psf		746.9	
Strain, % 11.0 σ <sub>1</sub> Failure, psf 746.9	5	Strain, %	11.0	
σ <sub>1</sub> Failure, psf 746.9	Ult	. Stress, psf	746.9	
[ ]	5	Strain, %	11.0	
$\sigma_3$ Failure, psf $0.0$	σ <sub>1</sub>	Failure, psf	746.9	
	$\sigma_3$	Failure, psf	0.0	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

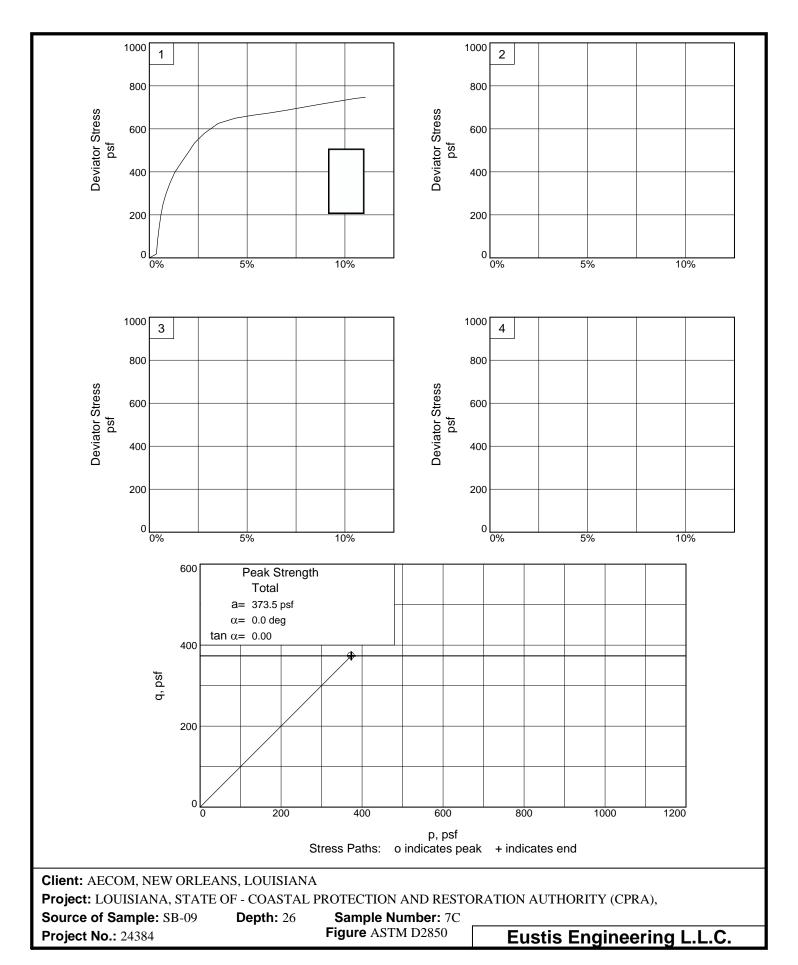
Source of Sample: SB-09 Depth: 26

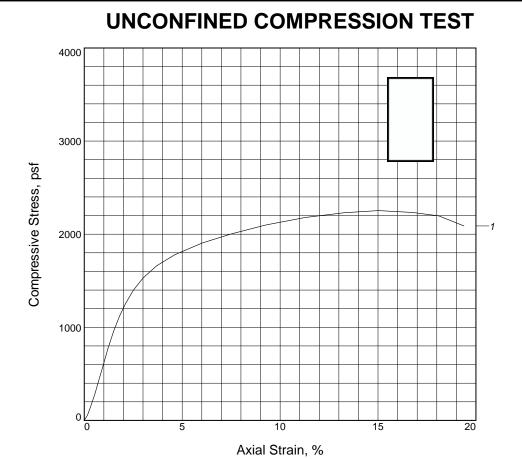
Sample Number: 7C

**Proj. No.:** 24384 **Date Sampled:** 10/7/20



Figure ASTM D2850





Specimen No.	1	
Unconfined strength, psf	2253.2	
Undrained shear strength, psf	1126.6	
Failure strain, %	15.0	
Strain rate, %/min.	1.00	
Water content, %	25.9	
Wet density, pcf	121.6	
Dry density, pcf	96.6	
Saturation, %	93.8	
Void ratio	0.7447	
Specimen diameter, in.	1.396	
Specimen height, in.	2.756	
Height/diameter ratio	1.97	
<b>l -</b>		

Description: ST LGR CH2 W/ ARS & LNS ML, RT

LL = 52 PL = 19 PI = 33 Assumed GS= 2.70 Type: UNDISTURBED

Project No.: 24384

**Date Sampled:** 9/21/20

Remarks:

TORVANE = 0.750 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

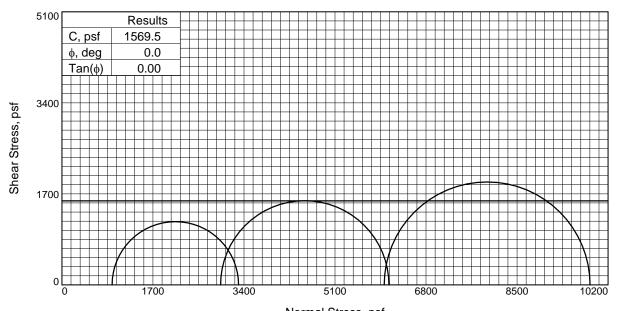
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

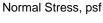
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 29

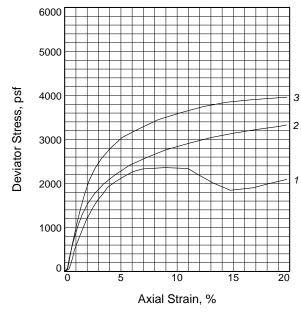
Sample Number: 8B

EUSTIS ENGINEERING SINCE 1946

Figure ASTM D2166







Type	of T	est:
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Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: ST GR & T CL4 W/ CC

**LL=** 41 **PL=** 17 **PI=** 24

**Assumed Specific Gravity=** 2.70 **Remarks:** TORVANE = 0.875 TSF

	Spe	ecimen No.	1	2	3	
		Water Content, %	24.6	23.8	23.9	
		Dry Density, pcf	100.4	99.5	101.0	
	Initial	Saturation, %	97.9	92.5	96.4	
	Ė	Void Ratio	0.6786	0.6940	0.6684	
3		Diameter, in.	1.398	1.397	1.396	
		Height, in.	2.763	2.759	2.764	
2		Water Content, %	25.1	25.7	24.8	
	+,	Dry Density, pcf	100.4	99.5	101.0	
	At Test	Saturation, %	100.0	100.0	100.0	
		Void Ratio	0.6786	0.6940	0.6684	
1	`	Diameter, in.	1.398	1.397	1.396	
		Height, in.	2.763	2.759	2.764	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.510	20.580	41.800	
	Fai	I. Stress, psf	2361.2	3143.9	3843.8	
	8	Strain, %	8.9	15.1	14.3	
	Ult.	Stress, psf	1845.9	3143.9	3843.8	
	8	Strain, %	14.7	15.1	14.3	
	σ <sub>1</sub>	Failure, psf	3298.6	6107.4	9863.0	
	$\sigma_3$	Failure, psf	937.4	2963.5	6019.2	

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

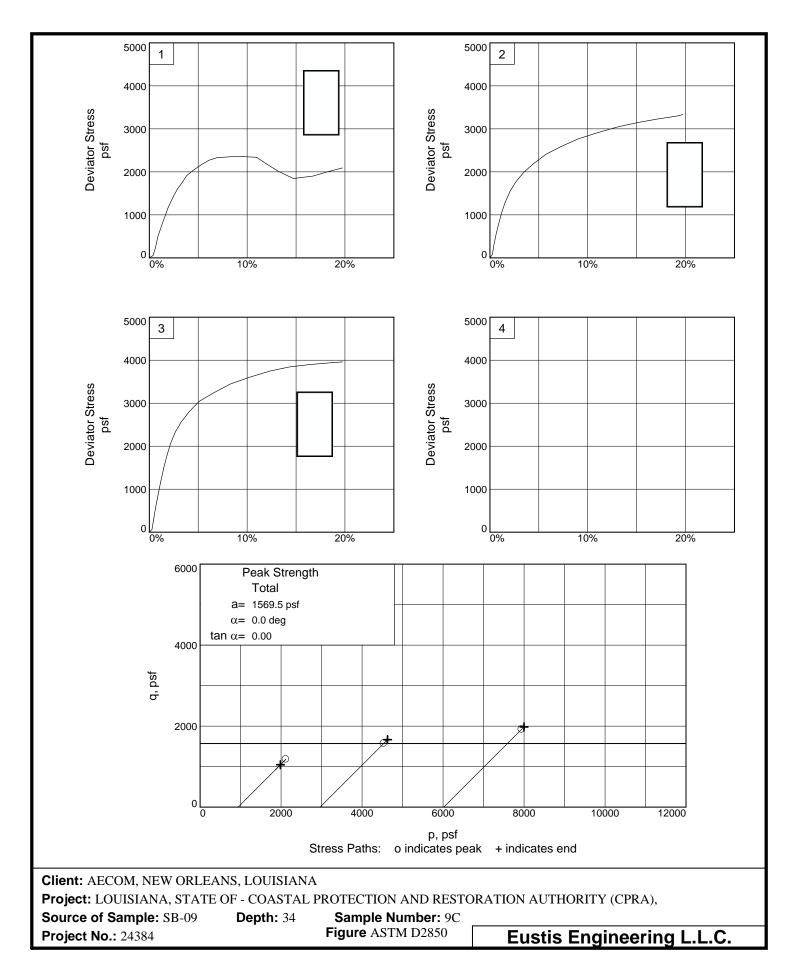
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 34

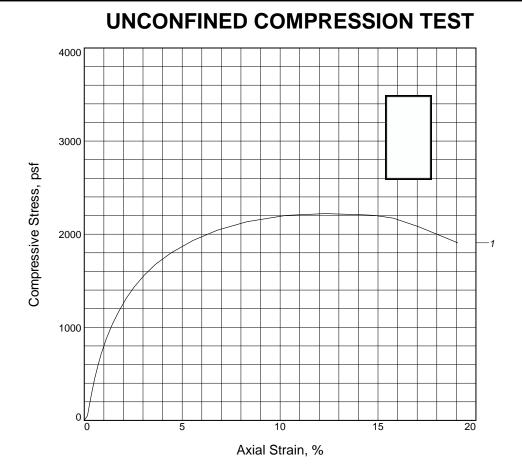
Sample Number: 9C

**Proj. No.:** 24384 **Date Sampled:** 9/21/20



Figure ASTM D2850





1		
Specimen No.	1	
Unconfined strength, psf	2221.3	
Undrained shear strength, psf	1110.6	
Failure strain, %	12.3	
Strain rate, %/min.	1.00	
Water content, %	26.0	
Wet density, pcf	122.8	
Dry density, pcf	97.5	
Saturation, %	96.3	
Void ratio	0.7295	
Specimen diameter, in.	1.398	
Specimen height, in.	2.761	
Height/diameter ratio	1.97	

**Description:** ST GR CL4 W/ CC

LL = 40 PL = 18 PI = 22 Assumed GS= 2.70 Type: UNDISTURBED

Project No.: 24384

**Date Sampled:** 9/21/20

Remarks:

TORVANE = 0.875 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

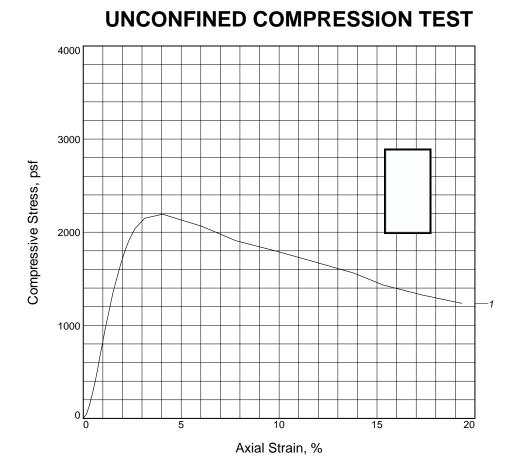
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 38

Sample Number: 10C

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Figure ASTM D2166



Specimen No.	1	
Unconfined strength, psf	2193.0	
Undrained shear strength, psf	1096.5	
Failure strain, %	4.0	
Strain rate, %/min.	1.00	
Water content, %	29.7	
Wet density, pcf	119.4	
Dry density, pcf	92.1	
Saturation, %	95.6	
Void ratio	0.8442	
Specimen diameter, in.	1.393	
Specimen height, in.	2.786	
Height/diameter ratio	2.00	

**Description:** ST GR CL6 W/ SIF

**PL** = 16 **LL =** 43 **PI =** 27 **Assumed GS=** 2.72 **Type:** UNDISTURBED

Project No.: 24384 **Date Sampled:** 10/6/20

Remarks:

TORVANE = 0.500 TSF

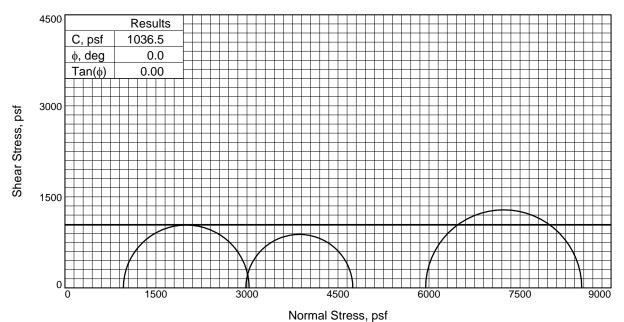
Client: AECOM, NEW ORLEANS, LOUISIANA

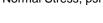
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

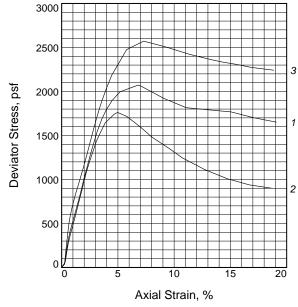
RESTORATION AUTHORITY (CPRA), **Source of Sample: SB-09 Depth:** 63.5

Sample Number: 19B

Figure ASTM D2166







Type of Test:	
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Unconsolidated Undrained Sample Type: UNDISTURBED **Description: ST GR CL6** 

**LL=** 48 **PL=** 18 **PI=** 30

Assumed Specific Gravity= 2.72	
Remarks: TORVANE = 0.500 TSF	

	Spe	ecimen No.	1	2	3	
		Water Content, %	32.2	34.8	32.0	
		Dry Density, pcf	90.3	87.2	90.7	
	<u>.a</u>	Saturation, %	99.4	99.9	99.9	
3	Initial	Void Ratio	0.8808	0.9467	0.8713	
		Diameter, in.	1.412	1.412	1.399	
		Height, in.	2.756	2.782	2.769	
1		Water Content, %	32.4	34.8	32.0	
	٠	Dry Density, pcf	90.3	87.2	90.7	
	es.	Saturation, %	100.0	100.0	100.0	
	At Test	Void Ratio	0.8808	0.9467	0.8713	
	4	Diameter, in.	1.412	1.412	1.399	
2		Height, in.	2.756	2.782	2.769	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.690	20.710	41.280	
	Fai	I. Stress, psf	2072.1	1762.3	2572.4	
	5	Strain, %	6.8	5.0	7.3	
	Ult.	Stress, psf	1767.4	1008.2	2333.5	
	5	Strain, %	15.1	14.7	14.3	
	$\sigma_1$	Failure, psf	3035.4	4744.5	8516.7	
	$\sigma_3$	Failure, psf	963.4	2982.2	5944.3	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

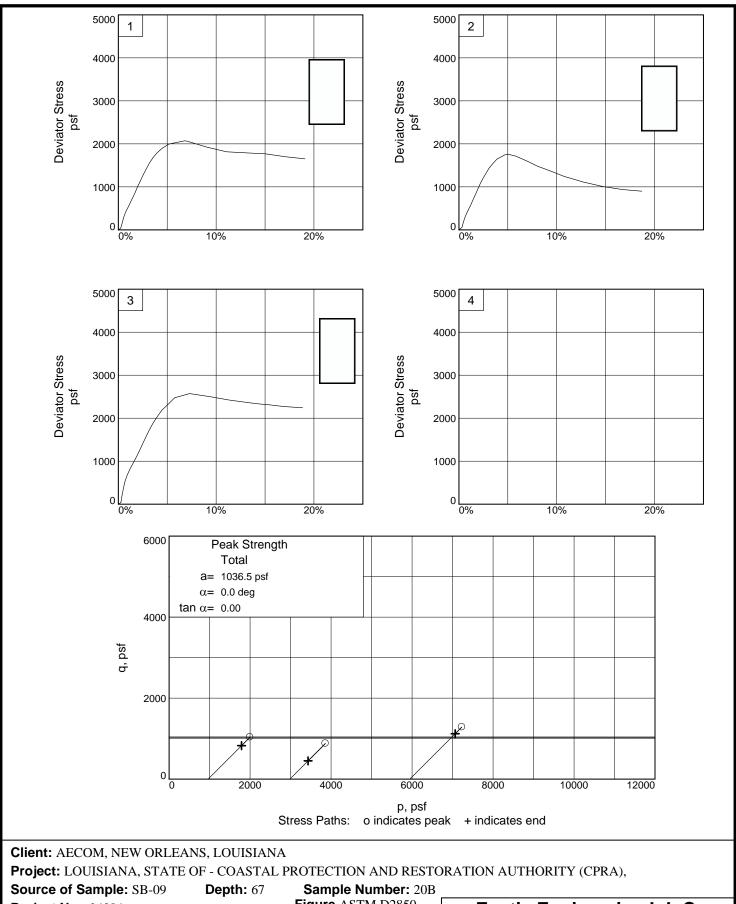
RESTORATION AUTHORITY (CPRA), **Source of Sample: SB-09** Depth: 67

Sample Number: 20B

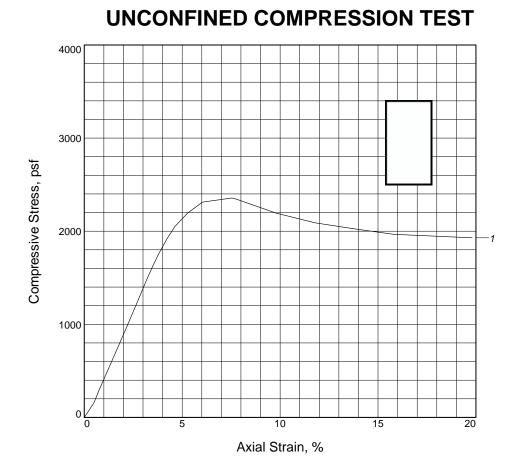
Proj. No.: 24384 **Date Sampled:** 10/6/20



Figure ASTM D2850



Project No.: 24384 Figure ASTM D2850 Eustis Engineering L.L.C.



Specimen No.	1	
Unconfined strength, psf	2356.8	
Undrained shear strength, psf	1178.4	
Failure strain, %	7.5	
Strain rate, %/min.	1.00	
Water content, %	32.5	
Wet density, pcf	117.7	
Dry density, pcf	88.8	
Saturation, %	97.0	
Void ratio	0.9119	
Specimen diameter, in.	1.413	
Specimen height, in.	2.751	
Height/diameter ratio	1.95	
Description: ST GR CH2 W/ ARS & LNS ML		

**LL** = 50 **PL** = 15

Assumed GS= 2.72

**Type:** UNDISTURBED

Project No.: 24384 Date Sampled: 10/6/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 71

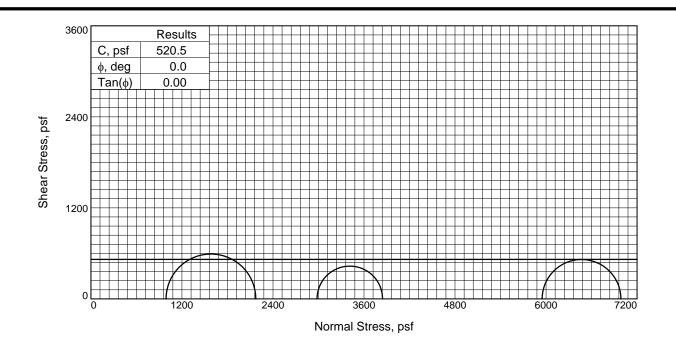
Sample Number: 21B

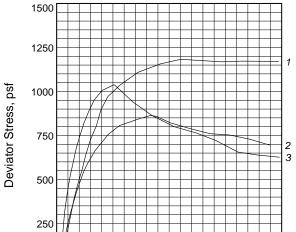
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Figure ASTM D2166

Tested By: CC Checked By: RR

**PI** = 35





Axial Strain, %

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: M GR CH4 W/ ARS ML, SIF

**LL=** 91 **PL=** 18 **PI=** 73

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.625 TSF

<b>Proj. No.:</b> 24384	
	7

Figure	<b>ASTM D2850</b>

Spe	ecimen No.	1	2	3	
	Water Content, %	45.3	43.5	46.6	
	Dry Density, pcf	74.2	76.6	74.4	
Initial	Saturation, %	95.7	97.1	99.0	
	Void Ratio	1.2893	1.2181	1.2812	
	Diameter, in.	1.399	1.381	1.401	
	Height, in.	2.805	2.878	2.889	
	Water Content, %	47.4	44.8	47.1	
#	Dry Density, pcf	74.2	76.6	74.4	
At Test	Saturation, %	100.0	100.0	100.0	
'#	Void Ratio	1.2893	1.2181	1.2812	
`	Diameter, in.	1.399	1.381	1.401	
	Height, in.	2.805	2.878	2.889	
Stra	ain rate, %/min.	1.00	1.00	1.00	
Bad	ck Pressure, psi	0.000	0.000	0.000	
Cel	l Pressure, psi	6.880	20.700	41.310	
Fai	l. Stress, psf	1183.0	864.1	1038.3	
8	Strain, %	11.0	8.4	5.1	
Ult.	Stress, psf	1168.7	752.6	721.3	
5	Strain, %	14.7	15.4	14.2	
σ <sub>1</sub>	Failure, psf	2173.7	3844.9	6986.9	
$\sigma_3$	Failure, psf	990.7	2980.8	5948.6	

Client: AECOM, NEW ORLEANS, LOUISIANA

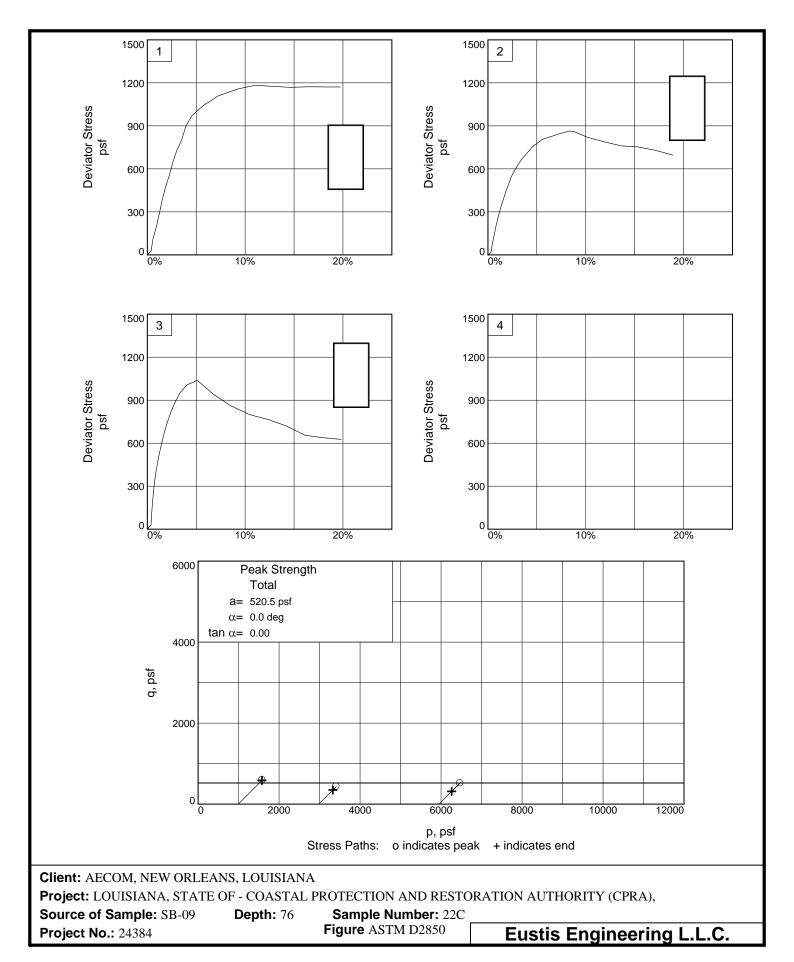
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

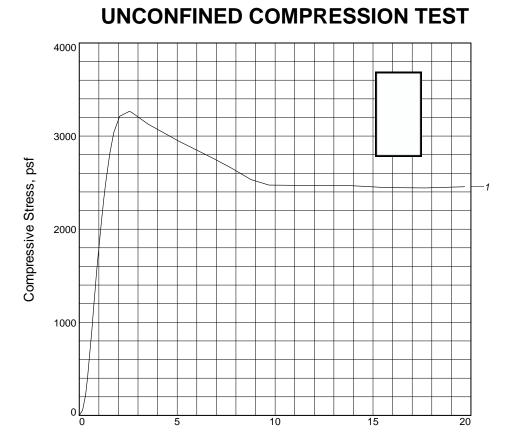
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 76

Sample Number: 22C

Proj. No.: 24384 Date Sampled: 10/6/20







Avial	Strain	0/
Axiai	Suani	. /0

Specimen No.	1	
Unconfined strength, psf	3266.1	
Undrained shear strength, psf	1633.1	
Failure strain, %	2.6	
Strain rate, %/min.	1.00	
Water content, %	41.8	
Wet density, pcf	112.6	
Dry density, pcf	79.4	
Saturation, %	99.8	
Void ratio	1.1385	
Specimen diameter, in.	1.391	
Specimen height, in.	2.756	
Height/diameter ratio	1.98	

**Description:** ST GR CH4 W/ ARS ML

**PL** = 19 **LL =** 75 **PI =** 56 Assumed GS= 2.72 **Type:** UNDISTURBED

Project No.: 24384 **Date Sampled:** 10/6/20

Remarks:

TORVANE = 0.625 TSF

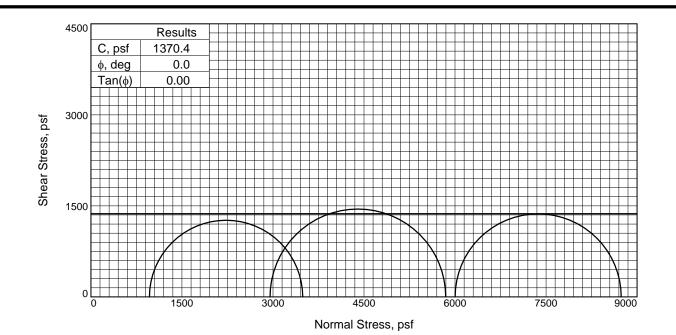
Client: AECOM, NEW ORLEANS, LOUISIANA

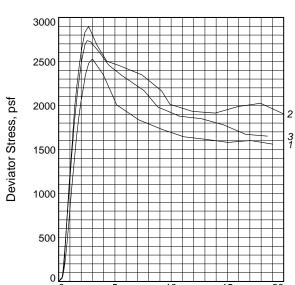
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), **Source of Sample: SB-09** 

Sample Number: 23C

Figure ASTM D2166





Axial Strain, %

Type of Test:

Unconsolidated Undrained **Sample Type:** UNDISTURBED

Description: ST GR CH4 W/ ARS ML, SIF

**LL=** 74 **PL=** 20 **PI=** 54

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.625 TSF

	Spe	ecimen No.	1	2	3	
		Water Content, %	43.3	43.7	45.0	
		Dry Density, pcf	78.0	77.4	76.3	
	Initial	Saturation, %	100.0	99.7	100.0	
	Ė	Void Ratio	1.1780	1.1927	1.2246	
		Diameter, in.	1.392	1.401	1.391	
•		Height, in.	2.795	2.755	2.830	
3		Water Content, %	43.3	43.8	45.0	
	;;	Dry Density, pcf	78.0	77.4	76.3	
	At Test	Saturation, %	100.0	100.0	100.0	
	🗏	Void Ratio	1.1780	1.1927	1.2246	
	~	Diameter, in.	1.392	1.401	1.391	
		Height, in.	2.795	2.755	2.830	
	Stra	ain rate, %/min.	1.00	1.00	1.00	
	Bad	ck Pressure, psi	0.000	0.000	0.000	
	Cel	l Pressure, psi	6.700	20.500	41.680	
	Fai	l. Stress, psf	2525.2	2894.7	2736.4	
	8	Strain, %	3.0	2.7	2.5	
	Ult.	Stress, psf	1580.4	1914.0	1779.8	
	5	Strain, %	15.1	13.9	14.7	
	σ <sub>1</sub>	Failure, psf	3490.0	5846.7	8738.3	
	$\sigma_3$	Failure, psf	964.8	2952.0	6001.9	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

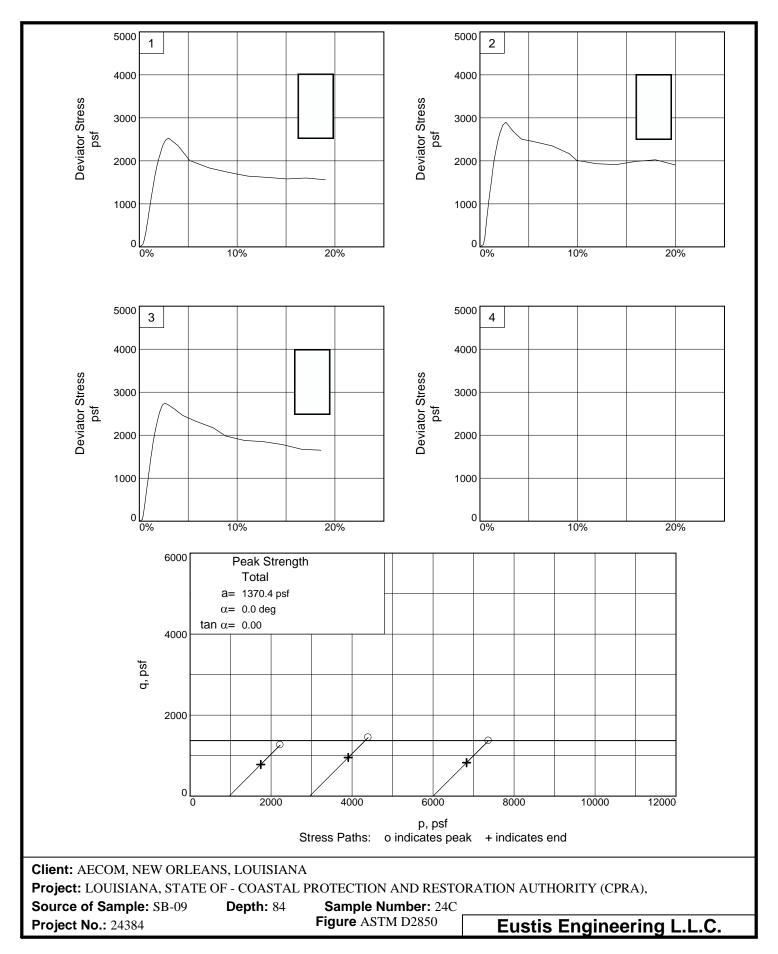
Source of Sample: SB-09 Depth: 84

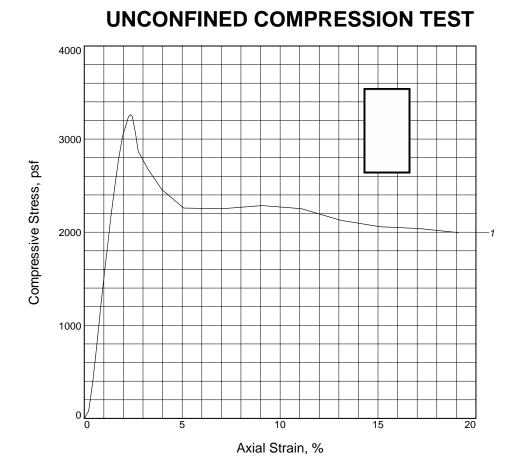
Sample Number: 24C

**Proj. No.:** 24384 **Date Sampled:** 10/6/20



Figure ASTM D2850





Specimen No.	1	
Unconfined strength, psf	3262.5	
Undrained shear strength, psf	1631.2	
Failure strain, %	2.4	
Strain rate, %/min.	1.00	
Water content, %	44.3	
Wet density, pcf	111.0	
Dry density, pcf	76.9	
Saturation, %	99.8	
Void ratio	1.2071	
Specimen diameter, in.	1.395	
Specimen height, in.	2.757	
Height/diameter ratio	1.98	
Description: ST GR CH3 W/ ARS & LNS ML		

Project No.: 24384

**PL** = 17

**PI** = 52

Remarks:

**LL =** 69

TORVANE = 0.500 TSF

**Date Sampled:** 10/6/20

Client: AECOM, NEW ORLEANS, LOUISIANA

Assumed GS= 2.72

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

**Type:** UNDISTURBED

RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 91

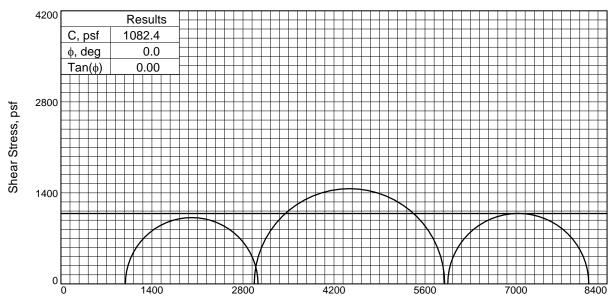
Sample Number: 26B

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Figure ASTM D2166

Tested By: CC

Checked By: RR

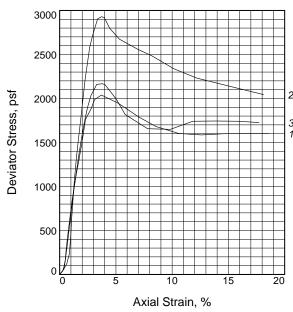


Normal Stress, psf

Water Content, %

Dry Density, pcf

Specimen No.



1		<u>a</u>	Saturation, %	94.9	96.9	97.4
┨		Initial	Void Ratio	1.0308	1.1104	1.0822
1	2	_	Diameter, in.	1.403	1.406	1.391
1			Height, in.	2.755	2.781	2.760
1	3		Water Content, %	37.9	40.8	39.8
1		st	Dry Density, pcf	83.6	80.5	81.6
1		<u>e</u>	Saturation, %	100.0	100.0	100.0
1		At Te	Void Ratio	1.0308	1.1104	1.0822
┨		4	Diameter, in.	1.403	1.406	1.391
1			Height, in.	2.755	2.781	2.760
1		Stra	ain rate, %/min.	1.00	1.00	1.00
1		Bad	ck Pressure, psi	0.000	0.000	0.000
1		Cel	l Pressure, psi	6.890	20.650	41.320
1		Fai	I. Stress, psf	2038.7	2928.7	2167.8
)		5	Strain, %	3.7	3.7	3.8
		Ult.	Stress, psf	1585.5	2169.3	1649.1
		5	Strain, %	12.6	14.2	9.8
		$\sigma_1$	Failure, psf	3030.9	5902.3	8117.9

1

36.0

83.6

2

39.6

80.5

3

38.7

81.6

5950.1

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: ST GR CH3 W/ ARS & LNS ML, O

**PL=** 18 **PI=** 45 **LL=** 63

**Assumed Specific Gravity=** 2.72 **Remarks:** TORVANE = 0.500 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

992.2

2973.6

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-09 Depth: 95

Sample Number: 27B

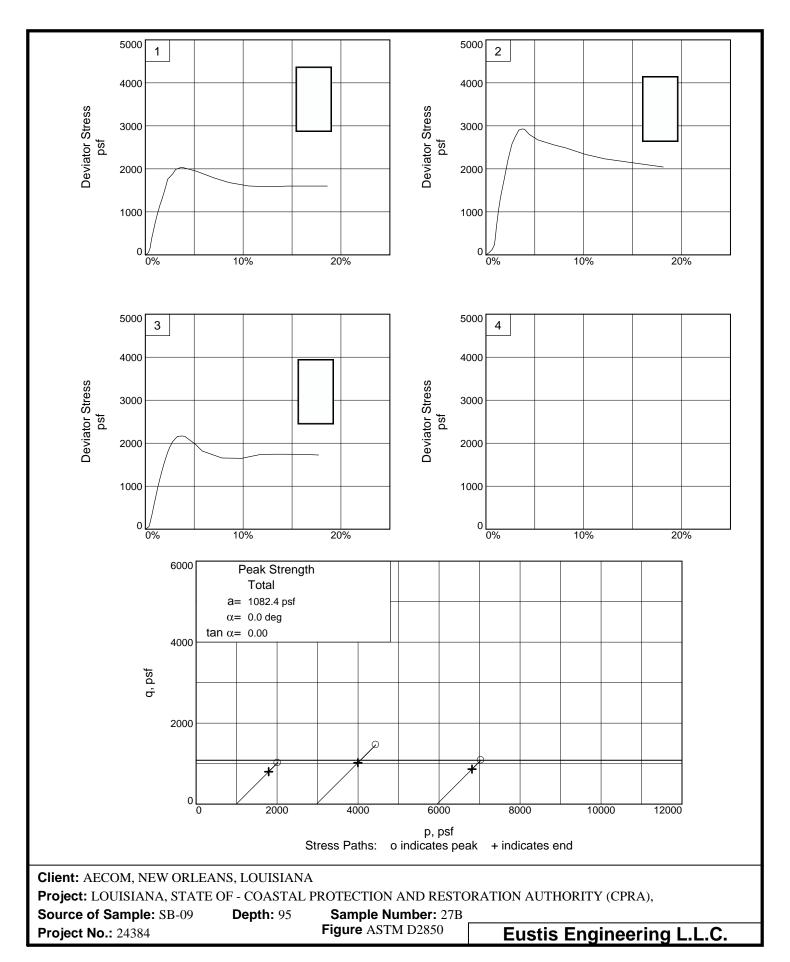
 $\sigma_3$  Failure, psf

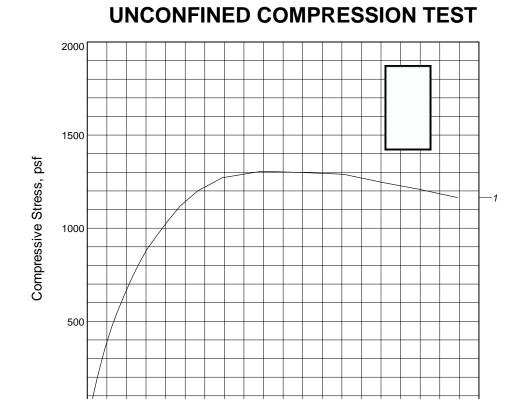
**Date Sampled:** 10/6/20 Proj. No.: 24384



Figure ASTM D2850

Checked By: RR Tested By: CC





Avial	Strain.	0/
Axiai	oualli.	7/0

Specimen No.	1	
Unconfined strength, psf	1305.9	
Undrained shear strength, psf	652.9	
Failure strain, %	8.9	
Strain rate, %/min.	1.00	
Water content, %	28.7	
Wet density, pcf	119.1	
Dry density, pcf	92.5	
Saturation, %	93.6	
Void ratio	0.8358	
Specimen diameter, in.	1.391	
Specimen height, in.	2.749	
Height/diameter ratio	1.98	

**Description:** M GR & T CL6 W/ SIF, WD

LL = 45 PL = 17 PI = 28 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 10/6/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

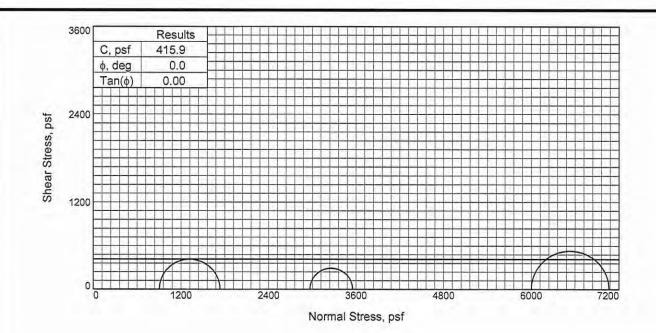
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

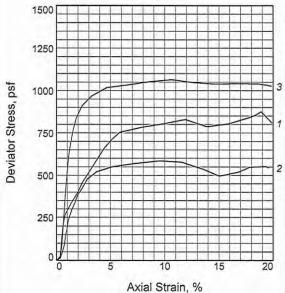
RESTORATION AUTHORITY (CPRA), **Source of Sample:** SB-09 **Depth:** 99

Sample Number: 28B

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Figure ASTM D2166





	Sp	ecimen No.	1	2	3	
3	Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	55.6 66.0 96.2 1.5728 1.374 2.757	58.2 65.7 100.0 1.5844 1.364 2.745	55.7 65.9 96.2 1.5752 1.387 2.752	
1	AtTest	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	57.8 66.0 100.0 1.5728 1.374 2.757	58.2 65.7 100.0 1.5844 1.364 2.745		
	Ba Ce Fai	ain rate, %/min. ck Pressure, psi Il Pressure, psi il. Stress, psf Strain, % . Stress, psf Strain, %	1.00 0.000 6.280 829.8 11.9 786.8 13.9	1.00 0.000 20.610 585.1 9.6 496.1 15.0	1.00 0.000 41.670 1064.2 10.6 1038.8 14.7	
	$\sigma_1$ $\sigma_3$		1734.1 904.3	3552.9 2967.8	7064.7 6000.5	

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED

Description: SO GR & T CH4 W/ ARS ML, RT

PI= 73

LL= 100 PL= 27

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.375 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

Source of Sample: SB-12 Depth: 1

Sample Number: 1B

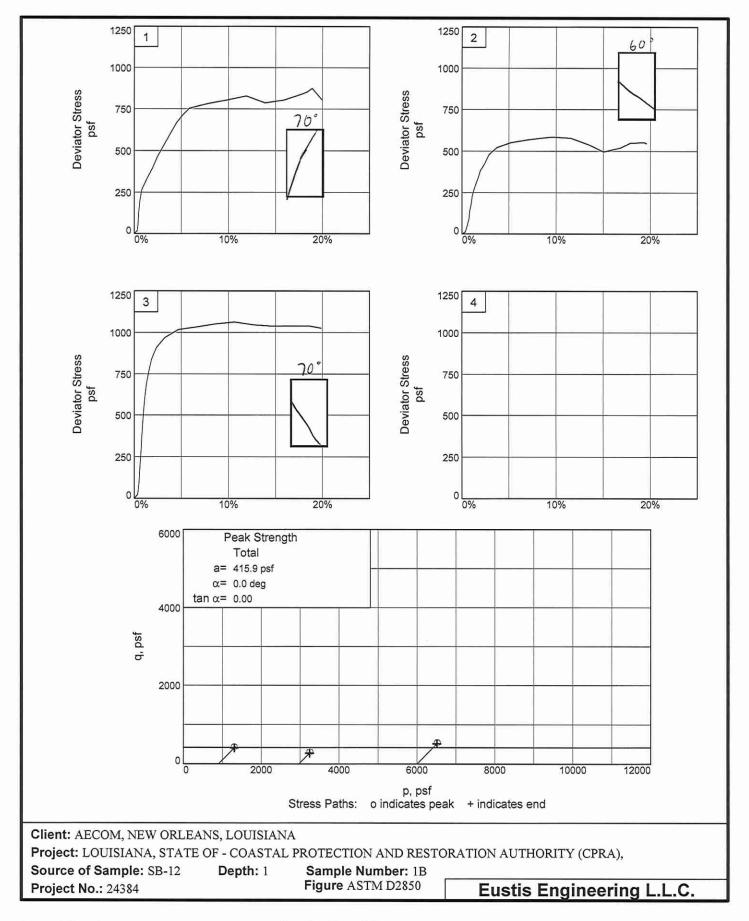
Proj. No.: 24384 Date Sampled: 11/11/20



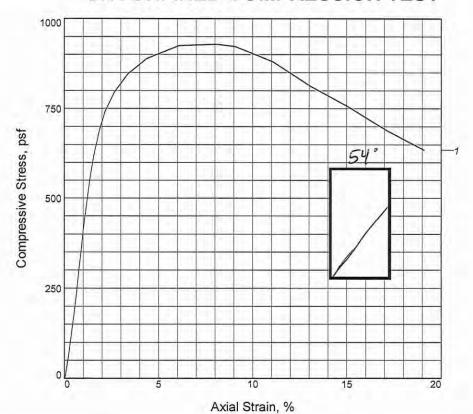
Figure ASTM D2850

Tested By: DE

Checked By: RR



#### **UNCONFINED COMPRESSION TEST**



Specimen No.	1	
Unconfined strength, psf	928.8	
Undrained shear strength, psf	464.4	
Failure strain, %	8.1	
Strain rate, %/min.	1.00	
Water content, %	60.5	
Wet density, pcf	103.1	
Dry density, pcf	64.2	
Saturation, %	100.1	
Void ratio	1.6438	
Specimen diameter, in.	1.365	
Specimen height, in.	2.764	
Height/diameter ratio	2.02	

Description: SO GR & T CH4 W/ ARS ML, RT

LL = 97 PL = 26 PI = 71 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 11/11/20

Remarks:

TORVANE = 0.375 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-12 Depth: 5

Sample Number: 3B

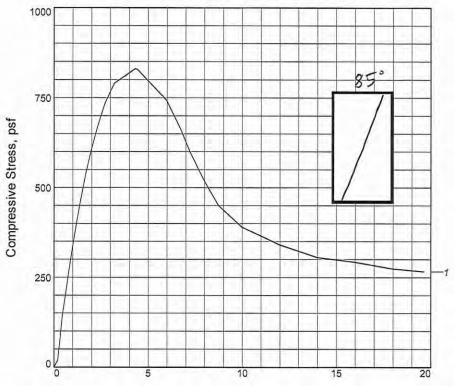
EUSTIS ENGINEERING

Figure ASTM D2166

Tested By: CC

Checked By: RR

## **UNCONFINED COMPRESSION TEST**



Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	830.6	
Undrained shear strength, psf	415.3	
Failure strain, %	4.3	
Strain rate, %/min.	1.00	
Water content, %	156.6	
Wet density, pcf	81.3	
Dry density, pcf	31.7	
Saturation, %	98.3	
Void ratio	4.2231	
Specimen diameter, in.	1.389	
Specimen height, in.	2.742	
Height/diameter ratio	1.97	

Description: SO DGR & BR CHOB

LL = PL = PI= Assumed GS= 2.65 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 11/16/20

Remarks:

TORVANE = 0.375 TSF

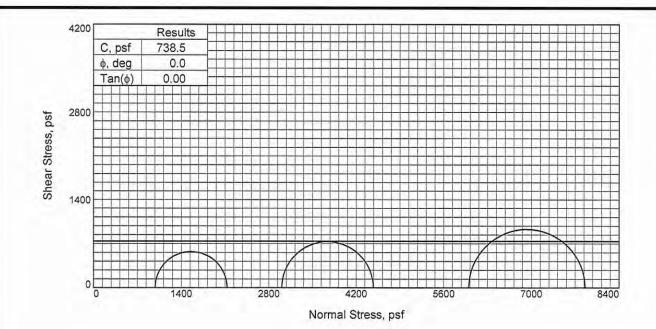
Client: AECOM, NEW ORLEANS, LOUISIANA

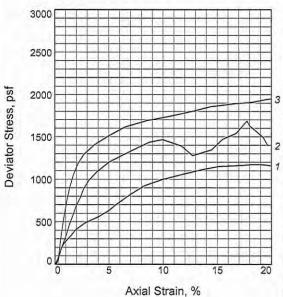
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-12 Depth: 13

Sample Number: 5B

Figure ASTM D2166





Sp	ecimen No.	1	2	3	
Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	25.7 99.3 98.4 0.7101 1.421 2.859	26.2 98.5 98.5 0.7246 1.412 2.883	97.6 0.7303	
AtTest	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	26.1 99.3 100.0 0.7101 1.421 2.859	26.6 98.5 100.0 0.7246 1.412 2.883	1.393	
Str	ain rate, %/min.	1.00	1.00	1.00	
Ва	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	6.830	20.890	41.670	
Fa	il. Stress, psf	1149.2	1465.6	1857.2	
103	Strain, %	15.1	10.0	14.4	
Ult	. Stress, psf	1149.2	1276.1	1857.2	
1	Strain, %	15.1	12.7	14.4	
σ,	Failure, psf	2132.8	4473.8	7857.7	
$\sigma_3$	Failure, psf	983.5	3008.2	6000.5	

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: M GR & DGR CL6 W/ O

LL= 42 PL= 16 PI= 26

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.350 TSF Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-12 Depth: 14

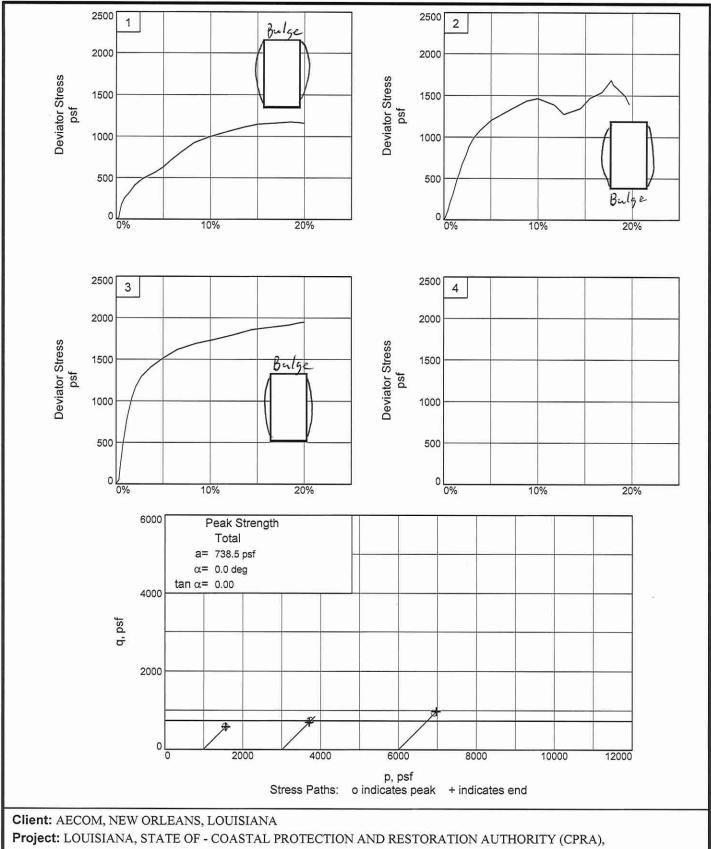
Client: AECOM, NEW ORLEANS, LOUISIANA

Sample Number: 5C

Proj. No.: 24384 Date Sampled: 11/11/20



Figure ASTM D2850



Source of Sample: SB-12

Project No.: 24384

Depth: 14

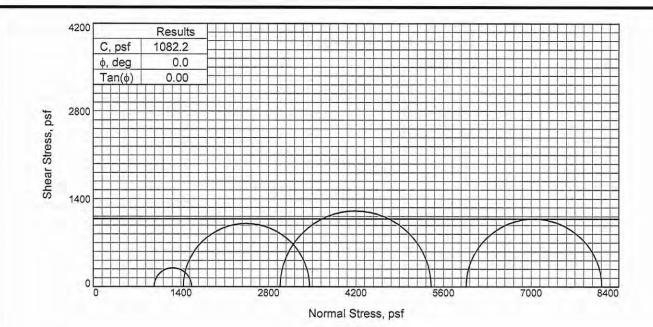
Sample Number: 5C

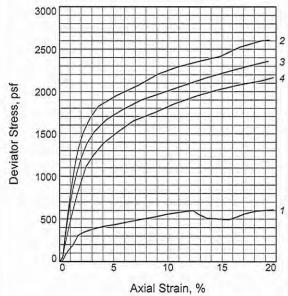
Figure ASTM D2850

**Eustis Engineering L.L.C.** 

Tested By: EM

Checked By: RR





Specimen No. 2 3 1 4 Water Content, % 25.9 24.3 24.3 24.4 Dry Density, pcf 96.1 98.9 98.4 98.5 Initial 91.7 Saturation, % 92.2 91.1 91.5 Void Ratio 0.7666 0.7170 0.7256 0.7245 Diameter, in. 1.414 1.404 1.406 1.385 Height, in. 2.748 2.754 2.755 2.752 Water Content. % 28.2 26.4 26.7 26.6 Dry Density, pcf 96.1 98.9 98.4 98.5 Saturation, % 100.0 100.0 100.0 100.0 Void Ratio 0.7666 0.7170 0.7256 0.7245 Diameter, in. 1.404 1.414 1.406 1.385 Height, in. 2.754 2.755 2.752 2.748 Strain rate, %/min. 1.00 1.00 1.00 1.00 0.000 Back Pressure, psi 0.000 0.000 0.000 Cell Pressure, psi 6.750 20.740 41.400 9.990 Fail. Stress, psf 598.7 2416.1 2166.0 2017.8 Strain, % 12.3 14.9 13.6 14.6 Ult. Stress, psf 510.6 2416.1 2166.0 2017.8 Strain, % 13.6 14.9 13.6 14.6 σ<sub>1</sub> Failure, psf 1570.7 5402.6 8127.6 3456.4 σ<sub>3</sub> Failure, psf 972.0 2986.6 5961.6 1438.6

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED

Description: ST GNG & T CL6 W/ WD

LL= 43

PL= 19

PI= 24

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.625 TSF Source of Sample: SB-12 Sample Number: 6B

Client: AECOM, NEW ORLEANS, LOUISIANA

RESTORATION AUTHORITY (CPRA),

Proj. No.: 24384

Date Sampled: 11/11/20



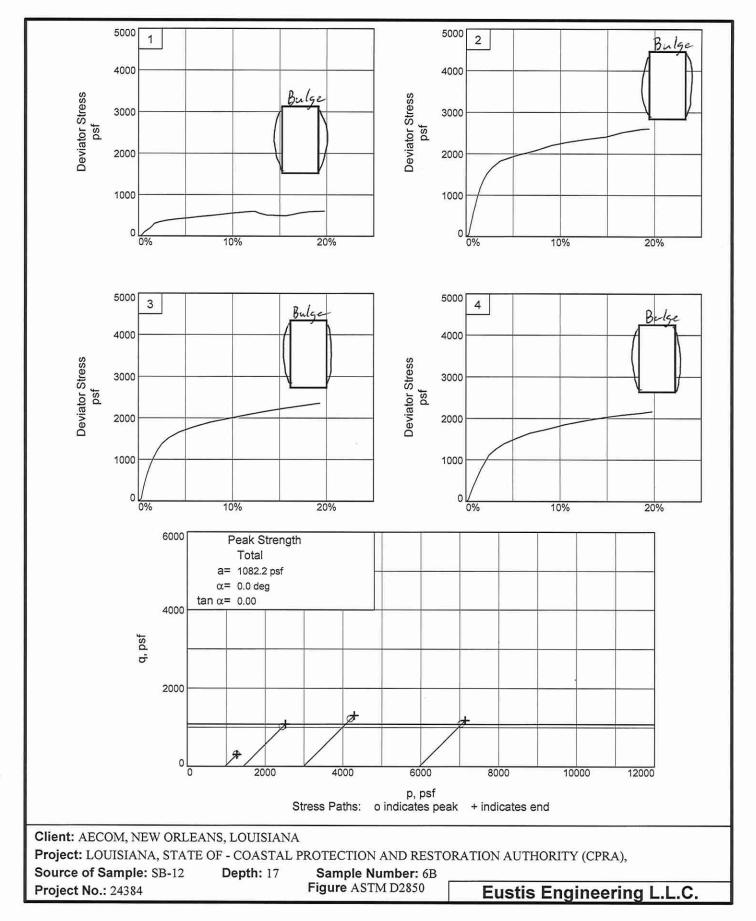
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Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

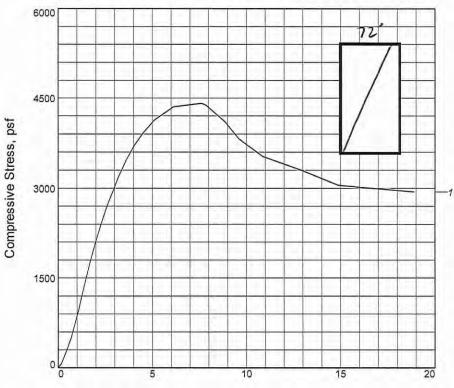
Figure ASTM D2850

Tested By: DE

Checked By: RR



## **UNCONFINED COMPRESSION TEST**



Axial Otlain, /	Axial	Strain,	%
-----------------	-------	---------	---

Specimen No.	1	
Unconfined strength, psf	4413.0	
Undrained shear strength, psf	2206.5	
Failure strain, %	7.6	
Strain rate, %/min.	1.00	
Water content, %	21.9	
Wet density, pcf	124.6	
Dry density, pcf	102.2	
Saturation, %	90.1	
Void ratio	0.6612	
Specimen diameter, in.	1.395	
Specimen height, in.	2.765	
Height/diameter ratio	1.98	

Description: VST GR & T CH2 W/ ARS & LNS ML

LL = 55 PL = 21 PI = 34 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 11/11/20

Date Sampled: 11/11/2

Remarks:

TORVANE = 0.750 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

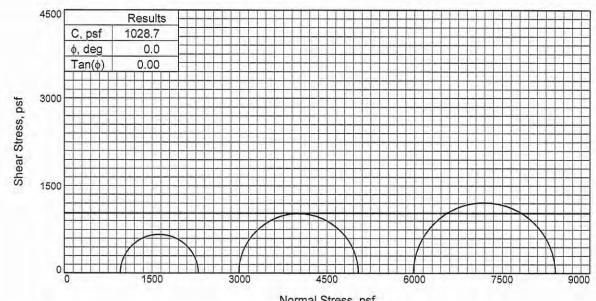
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-12 Depth: 22

Sample Number: 7C

EUSTIS

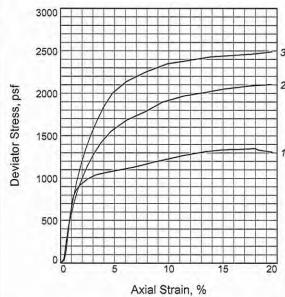
NGINEER

Figure ASTM D2166



Normal Stress, psf

Specimen No.



Ob.	Connent IVO.		~	0	
Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	28.4 92.6 93.3 0.8204 1.393 2.755	27.6 94.4 94.7 0.7863 1.395 2.757	28.6 92.1 93.0 0.8306 1.388 2.751	
AtTest	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	30.4 92.6 100.0 0.8204 1.393 2.755	29.1 94.4 100.0 0.7863 1.395 2.757	30.8 92.1 100.0 0.8306 1.388 2.751	
	ain rate, %/min. ck Pressure, psi	1.00 0.000	1.00 0.000	1.00 0.000	
	ll Pressure, psi il. Stress, psf	6.650 1335.2	20.740 2051.1	41.530 2429.0	
Ult.	Strain, % . Stress, psf	15.0 1335.2	15.1 2051.1	13.9 2429.0	
σ1	Strain, % Failure, psf Failure, psf	15.0 2292.8 957.6	15.1 5037.7 2986.6	13.9 8409.4 5980.3	

2

3

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED Description: ST T & GR CL6 W/CC

LL= 43 PL= 19 PI= 24

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.375 TSF Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-12 Depth: 26

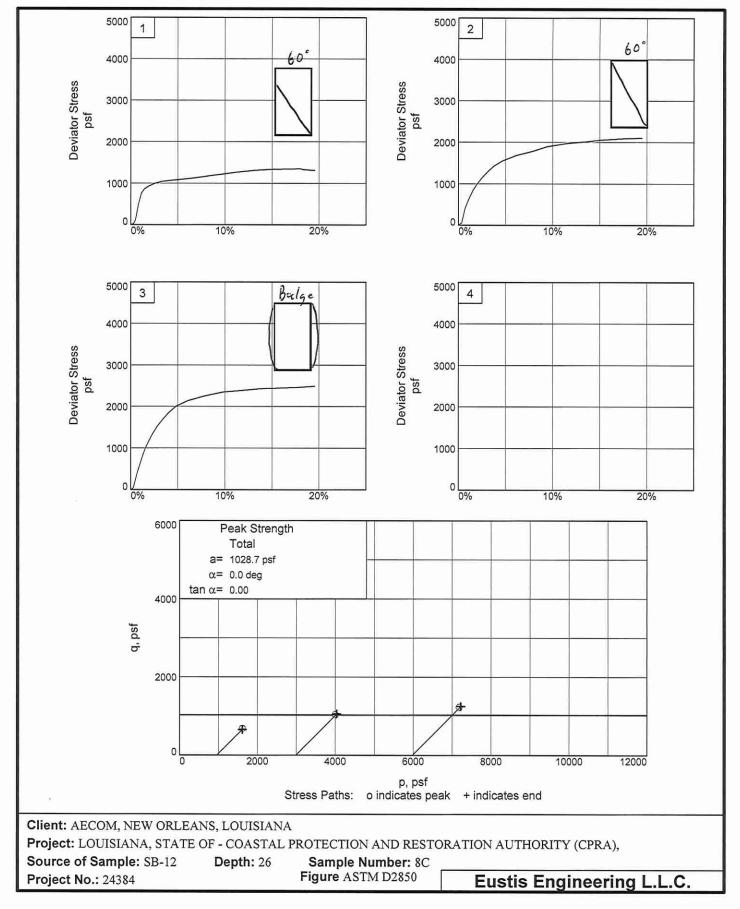
Sample Number: 8C

Proj. No.: 24384 Date Sampled: 11/11/20

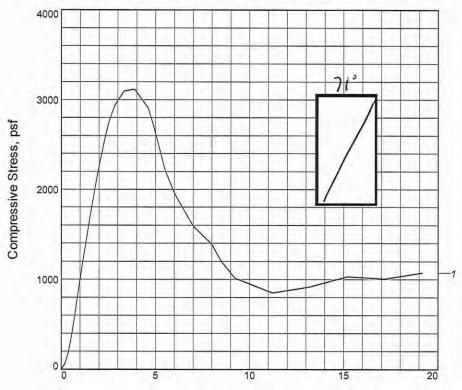


Figure ASTM D2850

Checked By: RR Tested By: DE



#### **UNCONFINED COMPRESSION TEST**



Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	3117.3	
Undrained shear strength, psf	1558.7	
Failure strain, %	3.8	
Strain rate, %/min.	1.00	
Water content, %	26.4	
Wet density, pcf	121.2	
Dry density, pcf	95.9	
Saturation, %	93.3	
Void ratio	0.7707	
Specimen diameter, in.	1.404	
Specimen height, in.	2.765	
Height/diameter ratio	1.97	

Description: ST T & GR CL6

LL = 40 PL = 19 PI = 21 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 11/11/20

Remarks:

TORVANE = 0.500 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

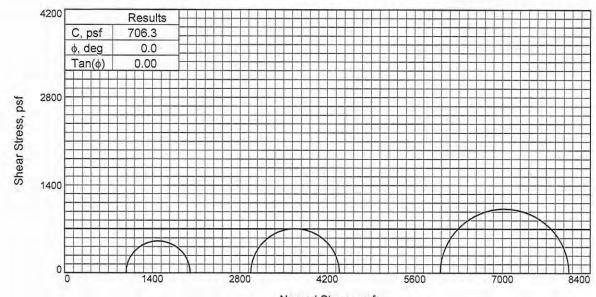
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-12 Depth: 30

Sample Number: 9C

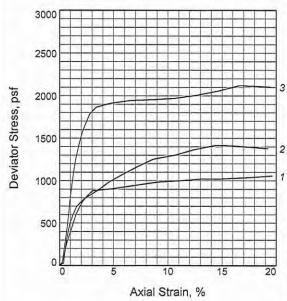
EUSTIS ENGINEERING

Figure ASTM D2166



Normal Stress, psf

Specimen No.



		Water Content, % Dry Density, pcf	32.4 86.2	30.0 89.7	31.8 87.8	
	a	Saturation, %	91.4	92.3		
	Initia	Void Ratio	0.9559	0.8787		
3	18	Diameter, in.	1.392		Control of the Contro	
		Height, in.	2.754	2.783	2.794	
		Water Content, %	35.4	32.5	34.0	
_	+2	Dry Density, pcf	86.2	89.7	87.8	
2	At Test	Saturation, %	100.0	100.0	100.0	
	=	Void Ratio	0.9559	0.8787	0.9192	
1	1	Diameter, in.	1.392	1.385	1.395	
		Height, in.	2.754	2.783	2.794	
	Str	ain rate, %/min.	1.00	1.00	1.00	
	Ba	ck Pressure, psi	0.000	0.000	0.000	
	Ce	l Pressure, psi	6.820	20.670	41.710	
	Fai	I. Stress, psf	1022.5	1417.5	2054.8	
	5	Strain, %	15.0	14.4	14.6	
Ult. Stress, psf		Stress, psf	1022.5	1417.5	2054.8	
	5	Strain, %	15.0	14.4	14.6	
H	$\sigma_1$	Failure, psf	2004.6	4394.0	8061.1	
	$\sigma_3$	Failure, psf	982.1	2976.5	6006.2	

Type of Test:

Unconsolidated Undrained

Sample Type: UNDISTURBED

Description: M GR & T CL6 W/ CC

LL= 48

PL= 18

PI= 30

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.375 TSF

Sample Number: 10C

Source of Sample: SB-12

Client: AECOM, NEW ORLEANS, LOUISIANA

RESTORATION AUTHORITY (CPRA),

Proj. No.: 24384

Date Sampled: 11/11/20

2

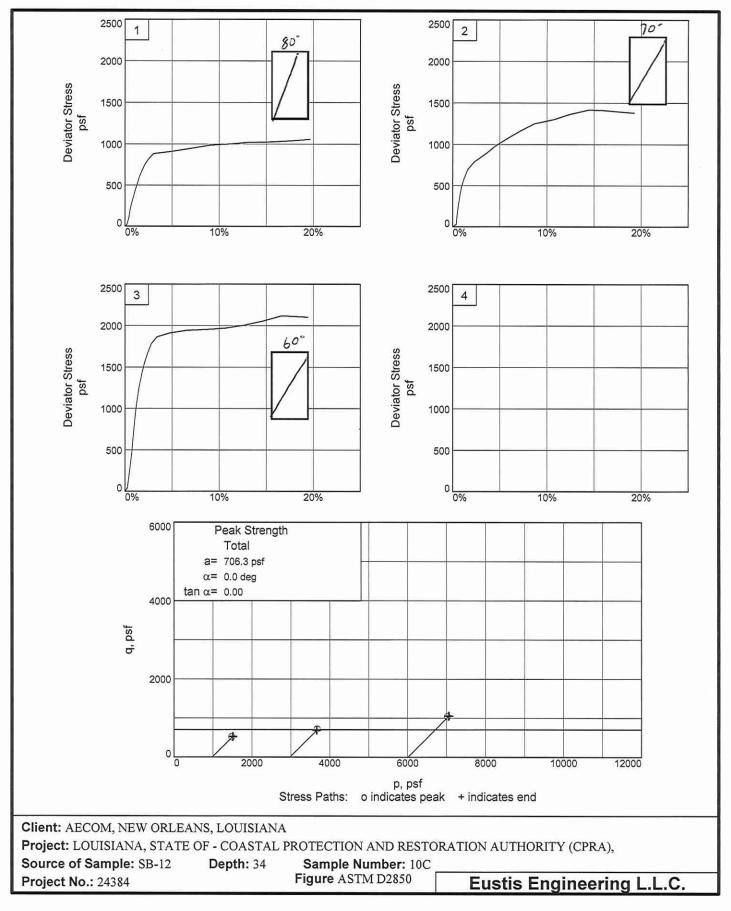
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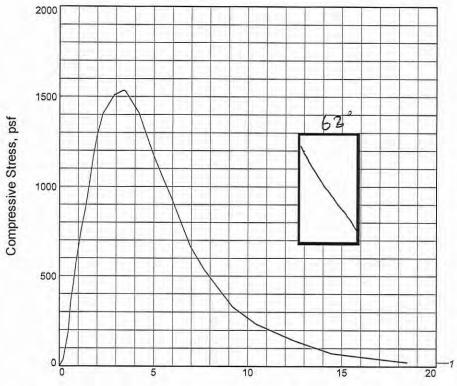
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

Depth: 34

Figure ASTM D2850



#### **UNCONFINED COMPRESSION TEST**



Axial Strain, %

Specimen No.	1	
Unconfined strength, psf	1535.4	
Undrained shear strength, psf	767.7	
Failure strain, %	3.4	
Strain rate, %/min.	1.00	
Water content, %	31.8	
Wet density, pcf	117.9	
Dry density, pcf	89.5	
Saturation, %	96.4	
Void ratio	0.8978	
Specimen diameter, in.	1.384	
Specimen height, in.	2.745	
Height/diameter ratio	1.98	

Description: ST GR & T CH3 W/ ARS ML, SL

LL = 64 PL = 18 PI = 46 Assumed GS= 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 11/11/20

Remarks:

TORVANE = 0.625 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),

Source of Sample: SB-12

Sample Number: 15B

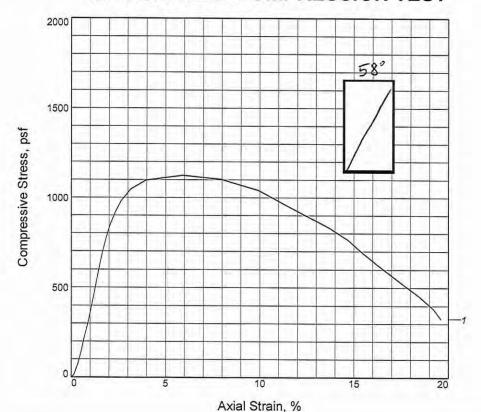
Depth: 47

Figure ASTM D2166

Tested By: CC

Checked By: RR

# **UNCONFINED COMPRESSION TEST**



Specimen No.	1	
Unconfined strength, psf	1124.4	
Undrained shear strength, psf	562.2	
Failure strain, %	5.9	
Strain rate, %/min.	1.00	
Water content, %	47.0	
Wet density, pcf	106.8	
Dry density, pcf	72.7	
Saturation, %	95.7	
Void ratio	1.3359	
Specimen diameter, in.	1.408	
Specimen height, in.	2.755	
Height/diameter ratio	1.96	

Description: M GR & T CH4 W/ ARS ML, CC, SL

LL = 101 PL = 31 PI = 70 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 11/11/20

Remarks:

TORVANE = 0.375 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

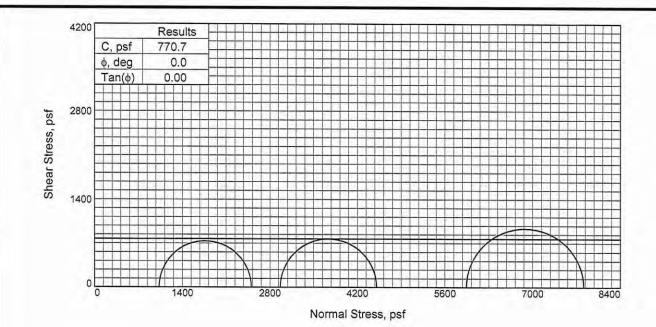
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

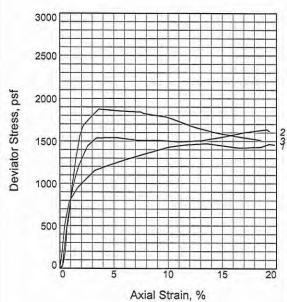
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-13 Depth: 1

Sample Number: 1B

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Figure ASTM D2166





Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: M GR & T CH4 W/ ARS ML, WD, SL

LL= 104 PL= 30 PI= 74

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.375 TSF

Sp	ecimen No.	1	2	3	
<u></u>	Water Content, % Dry Density, pcf Saturation, %	64.6 57.9	59.1 63.3	63.8 58.0	
Initial	Void Ratio	91.1 1.9304	95.6 1.6828	90.0 1.9285	
	Diameter, in. Height, in.	in. 1.392 1.425 2.765 2.765 ntent, % 71.0 61.9	1.425	333 W E.S.	
te de	Water Content, % Dry Density, pcf		61.9 63.3	23, 24550	
At Test	Saturation, % Void Ratio	100.0 1.9304	100.0 1.6828	. 77 - 17 - 17	
4	Diameter, in. Height, in.	1.392 2.765		1.371 2.700	
Str	ain rate, %/min.	1.00	1.00	1.00	
Ва	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	7.150	20.600	41.260	
Fai	il. Stress, psf	1472.1	1540.8	1874.0	
5	Strain, %	13.6	5.2	3.6	
Ult	. Stress, psf	1472.1	1493.5	1595.4	
	Strain, %	13.6	11.1	14.4	
σ <sub>1</sub>	Failure, psf	2501.7	4507.2	7815.5	
$\sigma_3$	Failure, psf	1029.6	2966.4	5941.4	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

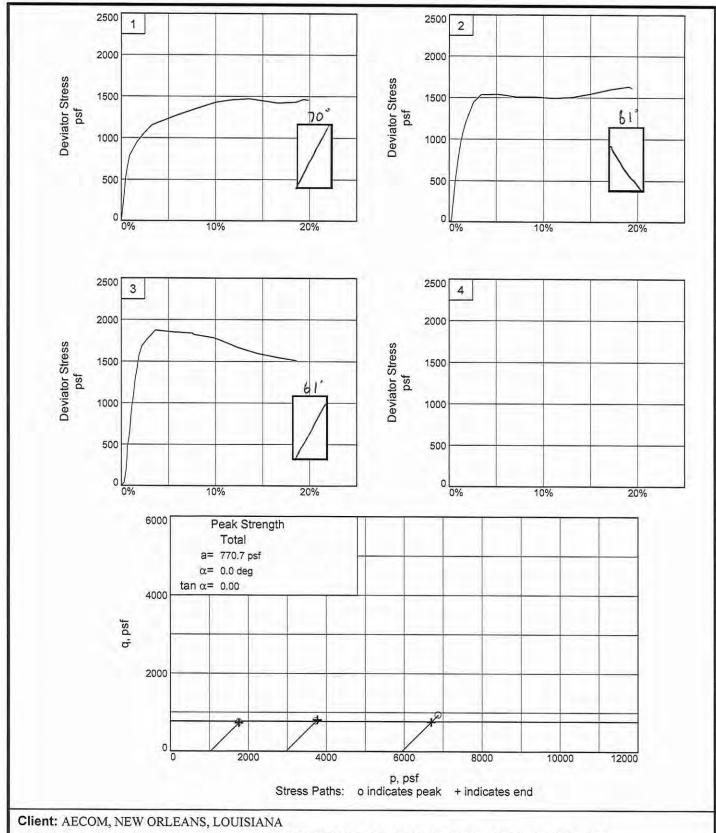
RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-13 Depth: 5

Sample Number: 3B

Proj. No.: 24384 Date Sampled: 11/16/20



Figure ASTM D2850



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA),

Source of Sample: SB-13

Depth: 5

Sample Number: 3B

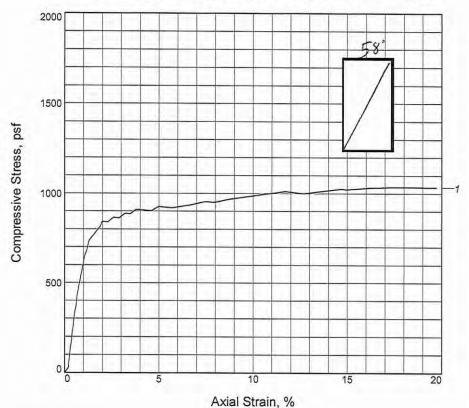
Project No.: 24384 Figure ASTM D 2850

**Eustis Engineering L.L.C.** 

Tested By: EM

Checked By: RR

## **UNCONFINED COMPRESSION TEST**



Specimen No.	1	
Unconfined strength, psf	1022.7	
Undrained shear strength, psf	511.4	
Failure strain, %	15.0	
Strain rate, %/min.	1.00	
Water content, %	44.4	
Wet density, pcf	110.4	
Dry density, pcf	76.5	
Saturation, %	98.8	
Void ratio	1.2211	
Specimen diameter, in.	1.386	
Specimen height, in.	2.753	
Height/diameter ratio	1.99	

Description: M DGR CH3 W/ WD, O

LL = 69 PL = 24 PI = 45 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384 Date Sampled: 11/11/20

Remarks:

TORVANE = 0.375 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-13 Depth: 9

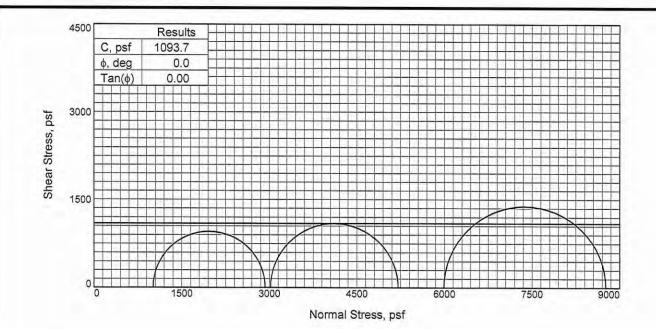
Sample Number: 4B

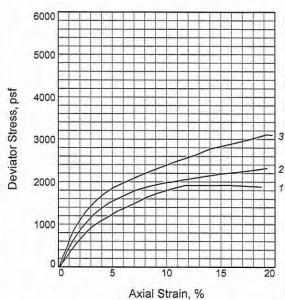
EUSTIS

Figure ASTM D2166

Tested By: CC

Checked By: RR





Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: ST GR CL6 W/O

LL= 40 PL= 20 Pl= 20

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.375 TSF

The state of the s
Proj. No.: 2438

Sp	pecimen No.	1	2	3	
Initial	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	26.0 96.9 94.2 0.7515 1.382 2.744	24.9 98.7 94.2 0.7199 1.401 2.742	25.5 97.1 92.4 0.7490 1.415 2.760	
At Test	Water Content, % Dry Density, pcf Saturation, % Void Ratio Diameter, in. Height, in.	27.6 96.9 100.0 0.7515 1.382 2.744	26.5 98.7 100.0 0.7199 1.401 2.742	100.0	
Ba Ce Fa	rain rate, %/min. ack Pressure, psi ell Pressure, psi ail. Stress, psf Strain, % t. Stress, psf Strain, %	1.00 0.000 7.020 1916.8 11.7 1914.3	1.00 0.000 20.970 2186.5 15.0 2186.5 15.0	1.00 0.000 41.600 2770.7 14.0 2770.7 14.0	
σ <sub>1</sub>	Failure, psf Failure, psf	2927.7 1010.9	5206.1 3019.7		

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

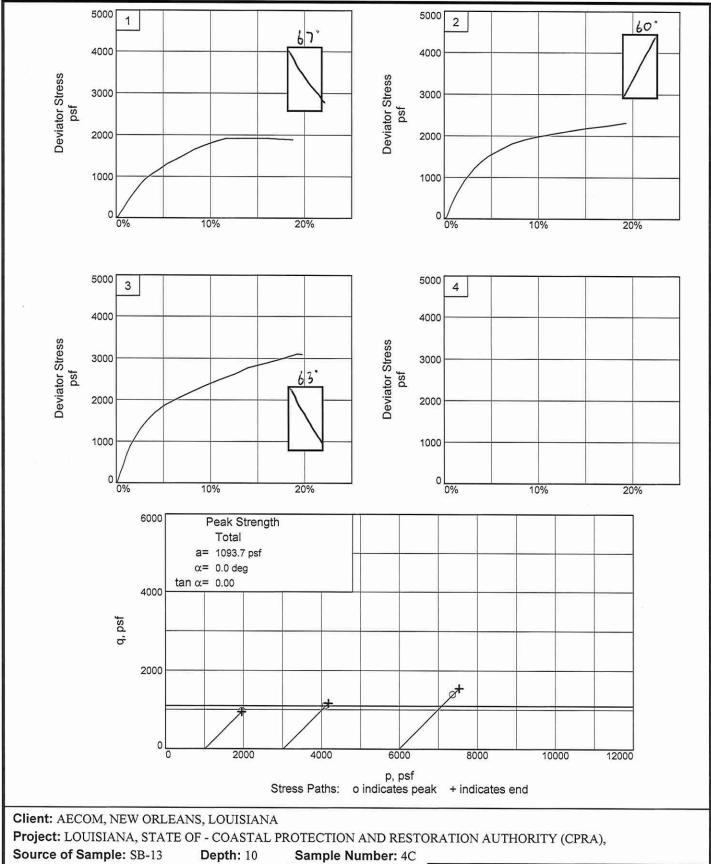
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-13 Depth: 10

Sample Number: 4C

Proj. No.: 24384 Date Sampled: 11/11/20



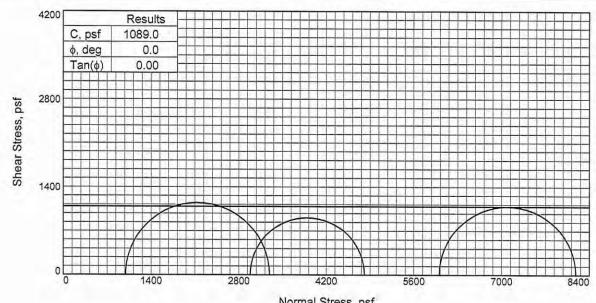
Figure ASTM D2850



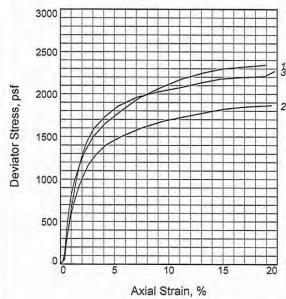
Project No.: 24384

Sample Number: 4C Figure ASTM D2850

**Eustis Engineering L.L.C.** 



Normal Stress, psf



Sp	ecimen No.	1	2	3	
	Water Content, % Dry Density, pcf	23.2 99.6	25.3 97.1	24.3 97.9	
Initial	Saturation, %	90.5	92.9	91.0	
Ξ	Void Ratio	0.6928	0.7361	0.7211	
	Diameter, in. Height, in.	1.374 2.713	7.7	1.394 2.755	
	Water Content, %	25.7	27.3	26.7	-
+	Dry Density, pcf	99.6	97.1	97.9	
At Test	Saturation, %	100.0	100.0	100.0	
At .	Void Ratio	0.6928	0.7361	0.7211	
	Diameter, in.	1.374	1.396	1.394	
	Height, in.	2.713	2.732	2.755	
Str	ain rate, %/min.	1.00	1.00	1.00	
Ba	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	6.900	20.720	41.700	
Fai	I. Stress, psf	2297.7	1824.6	2179.2	
5	Strain, %	15.0	15.1	14.8	
Ult.	Stress, psf	2297.7	1824.6	2179.2	
	Strain, %	15.0	15.1	14.8	
σ1	Failure, psf	3291.3	4808.3	8184.0	
σ3	Failure, psf	993.6	2983.7	6004.8	

Type of Test:

Unconsolidated Undrained Sample Type: UNDISTURBED Description: ST GR CL6 W/RT

LL= 49 PL= 20 PI= 29

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.375 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

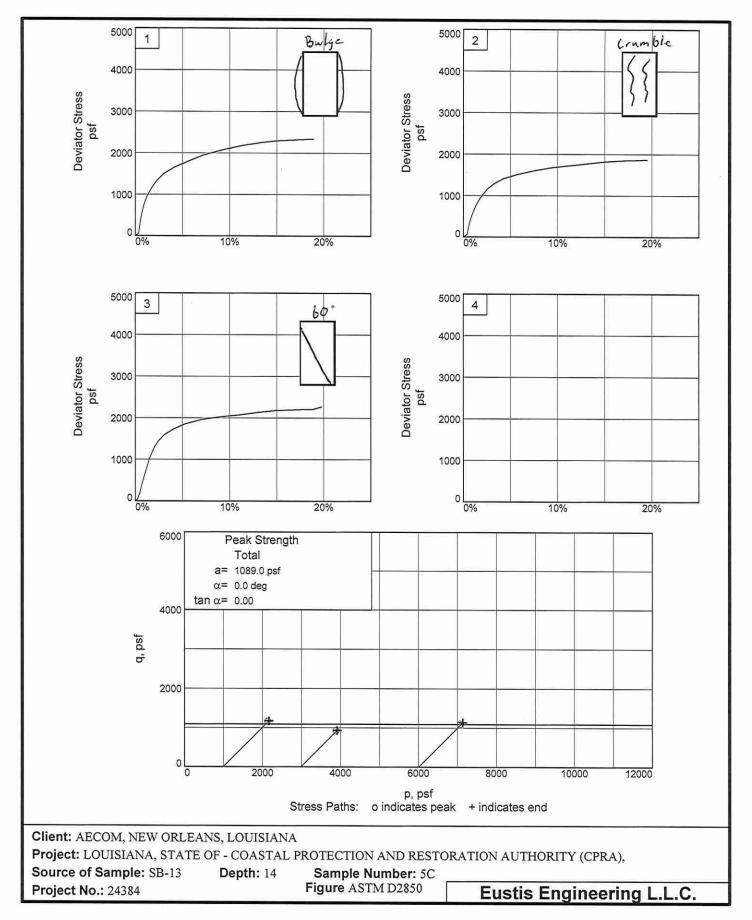
RESTORATION AUTHORITY (CPRA), Source of Sample: SB-13 Depth: 14

Sample Number: 5C

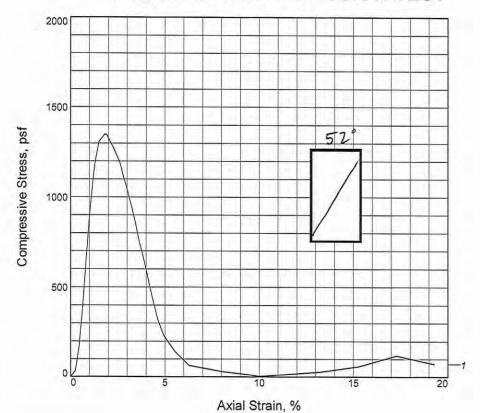
Proj. No.: 24384 Date Sampled: 11/11/20



Figure ASTM D2850



## **UNCONFINED COMPRESSION TEST**



Specimen No.	1	
Unconfined strength, psf	1349.7	
Undrained shear strength, psf	674.9	
Failure strain, %	1.8	
Strain rate, %/min.	1.00	
Water content, %	41.3	
Wet density, pcf	108.2	
Dry density, pcf	76.6	
Saturation, %	92.3	
Void ratio	1.2160	
Specimen diameter, in.	1.415	
Specimen height, in.	2.742	
Height/diameter ratio	1.94	

Description: M T & GR CH4 W/ ARS ML, SL

LL = 79 PL = 26 PI = 53 Assumed GS = 2.72 Type: UNDISTURBED

Project No.: 24384

Date Sampled: 11/11/20

Remarks:

TORVANE = 0.375 TSF

Client: AECOM, NEW ORLEANS, LOUISIANA

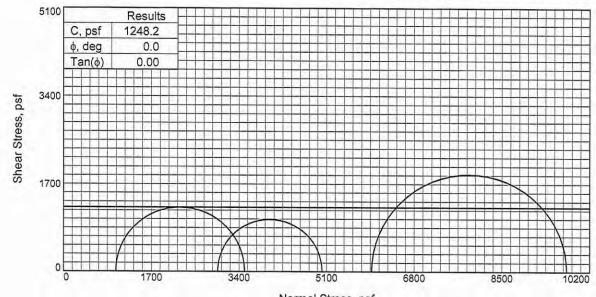
**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA), Source of Sample: SB-13 Depth: 22

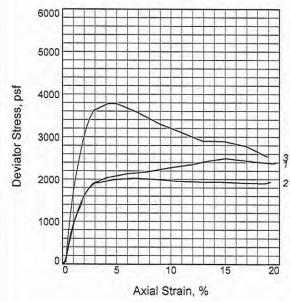
Sample Number: 7C

EUSTIS

Figure ASTM D2166



Normal Stress, psf



	_	
Type	of	Test:

Unconsolidated Undrained Sample Type: UNDISTURBED

Description: ST T & GR CH3 W/ ARS ML

LL= 60

PL= 23

PI= 37

Assumed Specific Gravity= 2.72 Remarks: TORVANE = 0.500 TSF

Sp	ecimen No.	1	2	3	
	Water Content, % Dry Density, pcf	35.9 85.3	34.3 85.4	34.4 87.0	
<u>a</u>	Saturation, %	98.8	94.4	98.4	
Initial	Void Ratio	0.9897	0.9886		
	Diameter, in.	1.405	1.395	1.385	
	Height, in.	2.805	2.763	2.752	
	Water Content, %	36.4	36.3	35.0	
**	Dry Density, pcf	85.3	85.4	87.0	
At Test	Saturation, %	100.0	100.0	100.0	
#	Void Ratio	0.9897	0.9886	0.9509	
1	Diameter, in.	1.405	1.395	1.385	
	Height, in.	2.805	2.763	2.752	
Str	ain rate, %/min.	1.00	1.00	1.00	
Ва	ck Pressure, psi	0.000	0.000	0.000	
Ce	Il Pressure, psi	7.030	20.740	41.490	
Fai	I. Stress, psf	2495.6	2025.7	3779.3	
5	Strain, %	15.0	6.6	4.2	
Ult	Stress, psf	2495.6	1935.7	2893.9	
5	Strain, %	15.0	14.7	14.9	
σ1	Failure, psf	3507.9	5012.3	9753.8	
$\sigma_3$	Failure, psf	1012.3	2986.6	5974.6	

Client: AECOM, NEW ORLEANS, LOUISIANA

Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

RESTORATION AUTHORITY (CPRA),
Source of Sample: SB-13 Depth: 26

Sample Number: 8C

Proj. No.: 24384

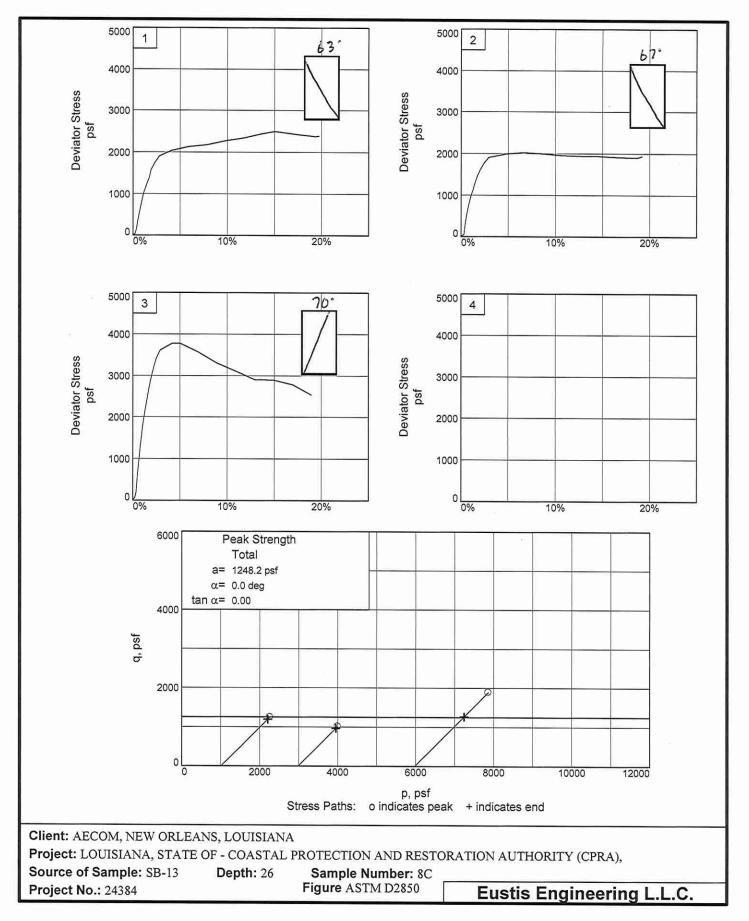
Date Sampled: 11/11/20

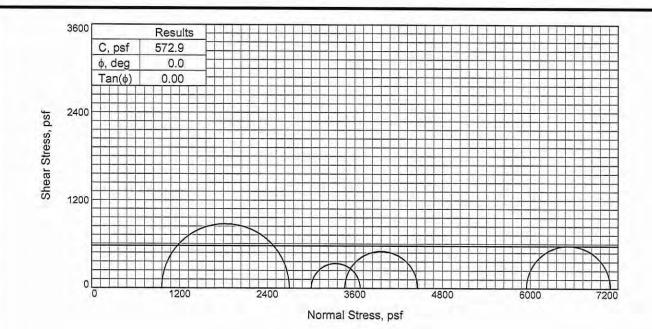


Figure ASTM D2850

Tested By: DE

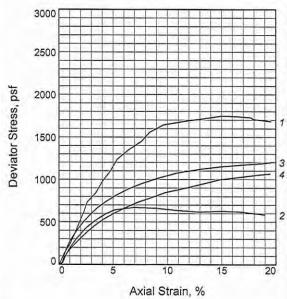
Checked By: RR





Specimen No.

Water Content, %



Dry Density, pcf 84.5 83.5 84.0 Saturation, % 97.1 97.2 97.7 Void Ratio 0.9939 1.0193 1.0059 Diameter, in. 1.435 1.435 1.415 Height, in. 2.695 2.662 2.705 Water Content, % 36.8 37.8 37.3 Dry Density, pcf 84.5 83.5 84.0 At Test Saturation, % 100.0 100.0 100.0 Void Ratio 0.9939 1.0193 1.0059 Diameter, in. 1.435 1.435 1.415 Height, in. 2.662 2.695 2.705 1.00 Strain rate, %/min. 1.00 1.00 Back Pressure, psi 0.000 0.000 0.000 Cell Pressure, psi 6.640 20.850 41.300 Fail. Stress, psf 1746.8 671.1 1150.4 Strain, % 15.0 6.9 14.9 Ult. Stress, psf 1746.8 617.1 1150.4 Strain, % 15.0 12.9 14.9 σ<sub>1</sub> Failure, psf 2702.9 3673.5 7097.6 σ<sub>3</sub> Failure, psf 956.2 3002.4 5947.2

Client: AECOM, NEW ORLEANS, LOUISIANA

RESTORATION AUTHORITY (CPRA),

Type of Test:

Unconsolidated Undrained
Sample Type: UNDISTURBED
Description: M GR CL6 W/ ARS SP

LL= 48

PL= 21

PI= 27

Assumed Specific Gravity= 2.70 Remarks: TORVANE = 0.150 TSF

Sample Number: 10B

Source of Sample: SB-13

Proj. No.: 24384

Date Sampled: 11/11/20

2

36.7

3

36.4

4

35.6

85.3

98.4

0.9761

1.424

2.879

36.2

85.3

100.0

1.424

2.879

1.00

0.000

999.4

14.9

999.4

14.9

4461.2

3461.8

24.040

0.9761

1

35.7



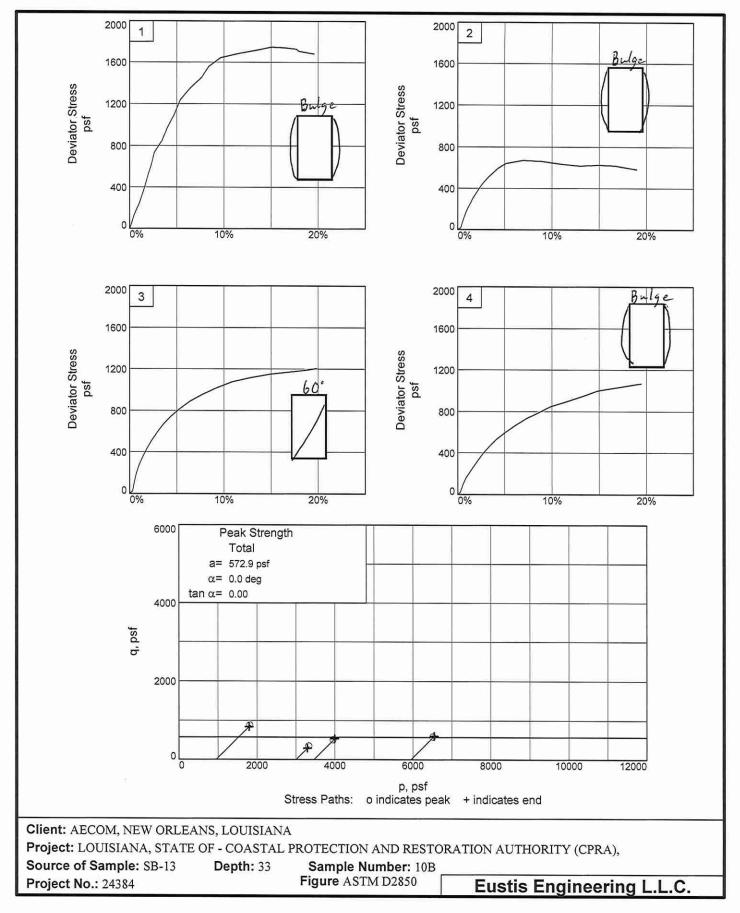
Depth: 33

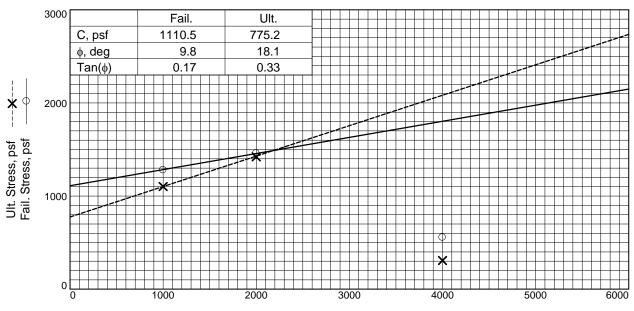
Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND

Figure ASTM D2850

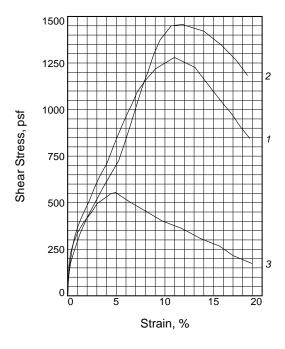
Tested By: DE

Checked By: RR





Normal Stress, psf



Spe	ecimen No.	1	2	3	
	Water Content, %	21.3	21.2	21.3	
	Dry Density, pcf	90.6	90.5	90.5	
Initial	Saturation, %	66.9	66.5	66.7	
Ē	Void Ratio	0.8608	0.8620	0.8622	
	Diameter, in.	2.612	2.610	2.610	
	Height, in.	0.772	0.774	0.774	
	Water Content, %	21.3	21.2	21.3	
	Dry Density, pcf	90.6	90.5	90.5	
At Test	Saturation, %	66.9	66.5	66.7	
¥	Void Ratio	0.8608	0.8620	0.8622	
	Diameter, in.	2.612	2.610	2.610	
	Height, in.	0.772	0.774	0.774	
Noi	rmal Stress, psf	1000.0	2000.0	4000.0	
Fai	I. Stress, psf	1279.4	1458.0	555.1	
St	train, %	11.0	11.9	4.9	
Ult.	Stress, psf	1102.0	1420.9	307.4	
St	train, %	14.9	14.0	13.7	
Stra	ain rate, %/min.	0.42	0.42	0.42	

Sample Type: UNDISTURBED

Description: SO GR CL4 W/ ARS SP, SIF

**Assumed Specific Gravity=** 2.7

Remarks:

Client: AECOM, NEW ORLEANS, LOUISIANA

**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND

Sample Number: 8B

Proj. No.: 24384

**Date Sampled:** 

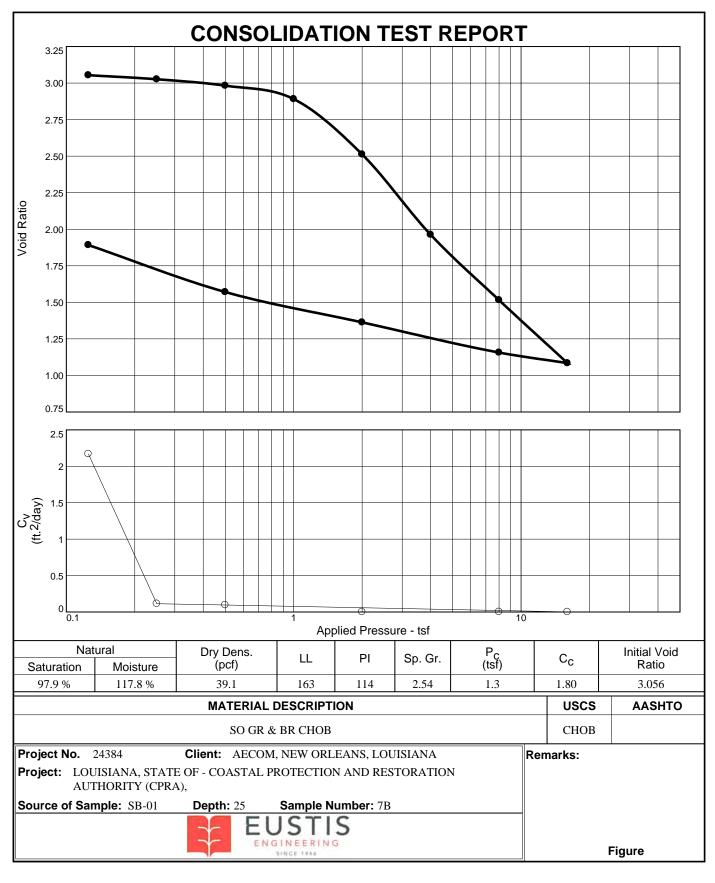


Figure \_\_\_\_

Tested By: ASR Checked By: ASR

## APPENDIX IX

COMPILATION OF CONSOLIDATION TEST REPORTS (SDR-A, SDR-B, SDR-C)



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** Location: SB-01

**Depth: 25** Sample Number: 7B

Material Description: SO GR & BR CHOB

**Liquid Limit:** 163 Plasticity Index: 114

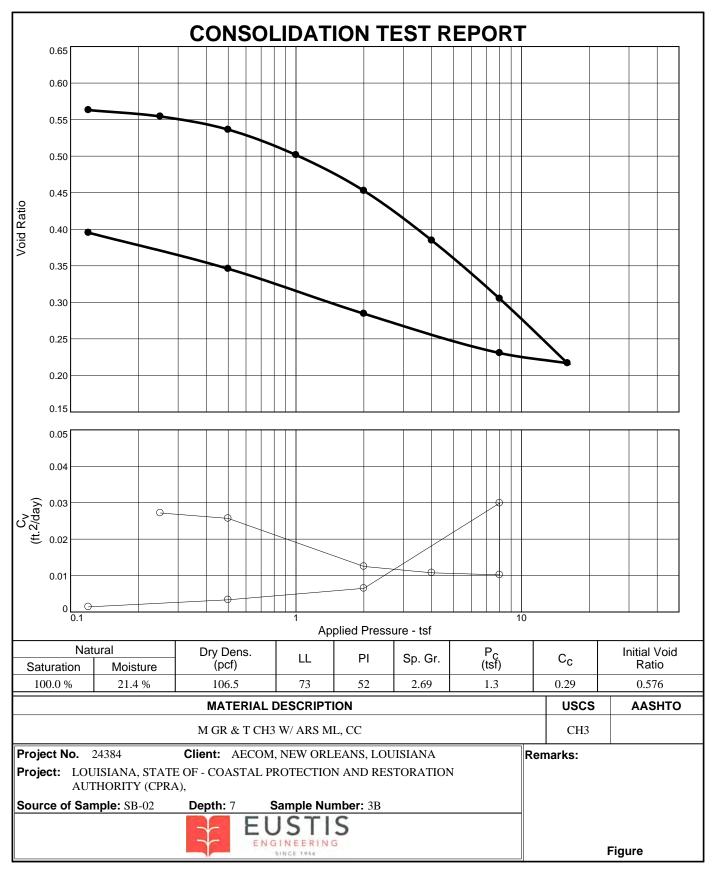
**USCS**: CHOB

Tested by: RH Checked by: RR

restea by: BH	Спескеа	DY: KK
	Test Specimen Data	a
NATURAL MOISTURE	VOID RATIO	AFTER TEST
Wet w+t = $279.98$ g.	<b>Spec. Gr.</b> = $2.54$	Wet w+t = $364.00$ g.
<b>Dry w+t</b> = $128.57$ g.	<b>Est. Ht. Solids =</b> $0.246 \text{ in.}$	<b>Dry w+t</b> = $260.84 \text{ g}$ .
<b>Tare Wt.</b> = $0.00  \text{g}$ .	Init. V.R. $= 3.056$	<b>Tare Wt.</b> = $132.27$ g.
<b>Moisture =</b> 117.8 %	Init. Sat. = 97.9 %	<b>Moisture =</b> 80.2 %
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $128.57^*$ g.
<b>Height</b> = 0.998 in.	Height = $0.998$ in.	
<b>Diameter =</b> 3.998 in.	Diameter = $3.998$ in.	
Weight = 279.98 g.		
<b>Dry Dens. =</b> 39.1 pcf	* Final dry weight used as m	nineral solids weight

End-Of-Load Summary						
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	$c_{lpha}$	Void Ratio	% Strain
start	1.98515	0.00000			3.056	
0.13	1.98571	0.00056	2.170	0.000	3.054	0.1 Comprs.
0.25	1.99256	0.00741	0.113	0.002	3.026	0.7 Comprs.
0.50	2.00335	0.01820	0.096	0.005	2.982	1.8 Comprs.
1.00	2.02581	0.04066		0.019	2.891	4.1 Comprs.
2.00	2.11900	0.13385		0.188	2.512	13.4 Comprs.
4.00	2.25410	0.26895		0.273	1.963	26.9 Comprs.
8.00	2.36426	0.37911		0.174	1.515	38.0 Comprs.
16.00	2.47029	0.48514	0.001	0.105	1.084	48.6 Comprs.
8.00	2.45249	0.46734	0.003		1.157	46.8 Comprs.
2.00	2.40171	0.41656	0.001		1.363	41.7 Comprs.
0.50	2.35060	0.36545			1.571	36.6 Comprs.
0.13	2.27158	0.28643			1.892	28.7 Comprs.

Compression index (C<sub>c</sub>), tsf = 1.80 Preconsolidation pressure (P<sub>p</sub>), tsf = 1.3 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 2.780



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-02

Depth: 7 **Sample Number:** 3B

Material Description: M GR & T CH3 W/ ARS ML, CC

**Liquid Limit:** 73 Plasticity Index: 52

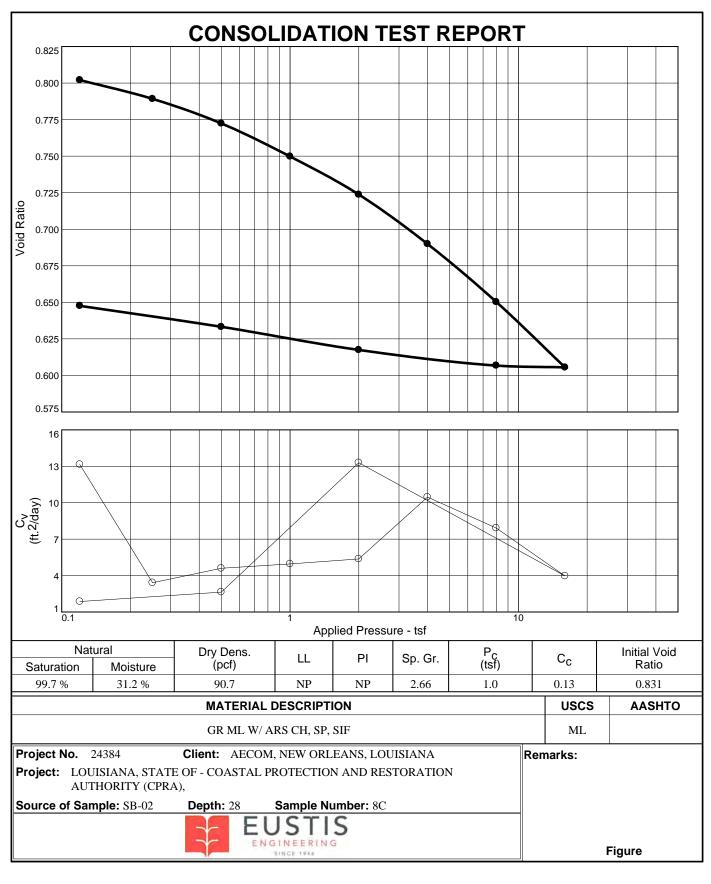
**USCS: CH3** 

Tested by: BH Checked by: RR

rested by. Dir	Officered	by. KK
	Test Specimen Dat	a
NATURAL MOISTURE	VOID RATIO	AFTER TEST
Wet w+t = 299.80 g.	<b>Spec. Gr.</b> = $2.69$	Wet w+t = $450.98$ g.
<b>Dry w+t</b> = 246.90 g.	<b>Est. Ht. Solids =</b> $0.446 \text{ in.}$	<b>Dry w+t</b> = $402.12$ g.
Tare Wt. = $0.00  \text{g}$ .	Init. V.R. = $0.576$	<b>Tare Wt.</b> = $155.22$ g.
<b>Moisture =</b> 21.4 %	Init. Sat. = 100.0 %	<b>Moisture =</b> 19.8 %
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $246.90^*$ g.
Height = $0.703$ in.	Height = $0.703$ in.	-
<b>Diameter =</b> 3.999 in.	<b>Diameter</b> = 3.999 in.	
Weight = 299.80 g.		
Dry Dens = 106.5 ncf	* Final dry weight used as n	nineral solids weight

			End	-Ot-Load Su	ımmary			
Pressure (tsf)			C <sub>V</sub> (ft. <sup>2</sup> /day)	${f c}_{lpha}$	Void Ratio	% Strain		
start	0.00590		0.00000			0.576		
0.12	0.01280	0.00090	0.00600		0.000	0.563	0.9 Comprs.	
0.25	0.01910	0.00330	0.00990	0.027	0.001	0.554	1.4 Comprs.	
0.50	0.02950	0.00570	0.01790	0.026	0.003	0.536	2.5 Comprs.	
1.00	0.04690	0.00760	0.03340		0.006	0.502	4.8 Comprs.	
2.00	0.07050	0.00940	0.05520	0.013	0.007	0.453	7.9 Comprs.	
4.00	0.10250	0.01110	0.08550	0.011	0.008	0.385	12.2 Comprs.	
8.00	0.13990	0.01300	0.12100	0.010	0.011	0.305	17.2 Comprs.	
16.00	0.18150	0.01520	0.16040		0.014	0.217	22.8 Comprs.	
8.00	0.17240	0.01230	0.15420	0.030		0.231	21.9 Comprs.	
2.00	0.14620	0.01010	0.13020	0.006	0.000	0.284	18.5 Comprs.	
0.50	0.11750	0.00880	0.10280	0.003		0.346	14.6 Comprs.	
0.12	0.09260	0.00590	0.08080	0.001		0.395	11.5 Comprs.	

Compression index (C<sub>C</sub>), tsf = 0.29 Preconsolidation pressure (P<sub>p</sub>), tsf = 1.3 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 0.483



**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-02

Depth: 28 Sample Number: 8C

Material Description: GR ML W/ ARS CH, SP, SIF

Liquid Limit: NP Plasticity Index: NP

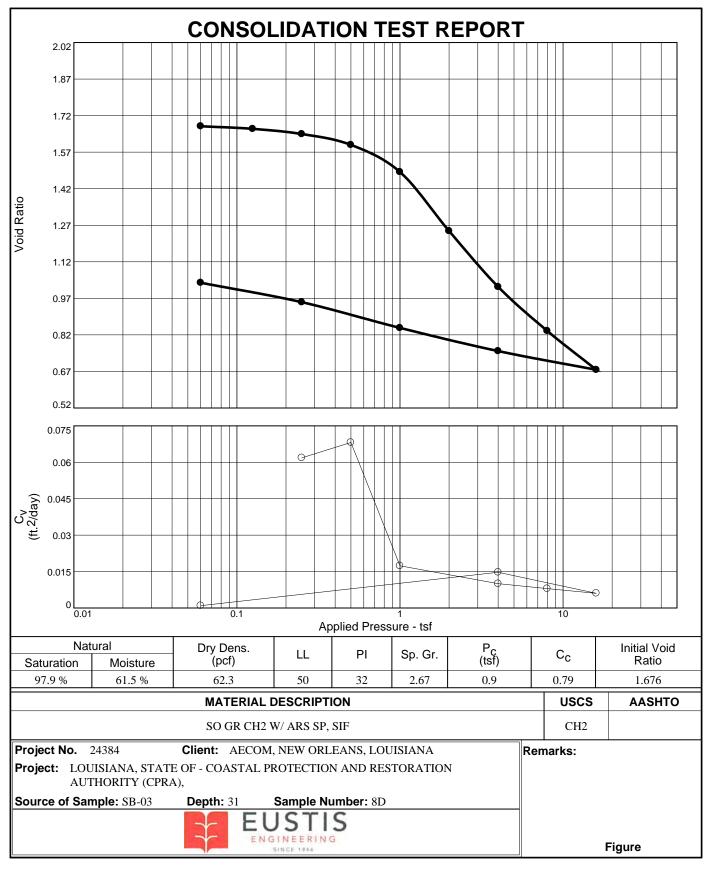
USCS: ML

Tested by: BH Checked by: RR

resieu by. Dir	Checked by. KK					
	Test Specimen Data					
NATURAL MOISTURE	VOID RATIO	AFTER TEST				
Wet w+t = 294.96 g.	<b>Spec. Gr.</b> = $2.66$	Wet w+t = $476.64$ g.				
<b>Dry w+t</b> = $224.87$ g.	<b>Est. Ht. Solids =</b> $0.411$ in.	<b>Dry w+t</b> = $410.17$ g.				
<b>Tare Wt.</b> = $0.00  \text{g}$ .	Init. V.R. $= 0.831$	<b>Tare Wt.</b> = $185.30 \text{ g}$ .				
<b>Moisture =</b> 31.2 %	Init. Sat. = 99.7 %	<b>Moisture =</b> 29.6 %				
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $224.87^*$ g.				
Height = $0.753$ in.	Height = $0.753$ in.					
<b>Diameter =</b> 3.997 in.	Diameter = $3.997$ in.					
<b>Weight</b> = 294.96 g.						
<b>Dry Dens. =</b> 90.7 pcf	* Final dry weight used as mineral solids weight					

		End	-Of-Load S	ummary		
Final Dial (in.)	Machine Defl. (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	$c_{lpha}$	Void Ratio	% Strain
0.00030		0.00000			0.831	
0.01330	0.00090	0.01210	13.150		0.802	1.6 Comprs.
0.02100	0.00330	0.01740	3.407		0.789	2.3 Comprs.
0.03030	0.00570	0.02430	4.600		0.772	3.2 Comprs.
0.04150	0.00760	0.03360	4.959		0.750	4.5 Comprs.
0.05400	0.00940	0.04430	5.370		0.724	5.9 Comprs.
0.06960	0.01110	0.05820	10.464		0.690	7.7 Comprs.
0.08780	0.01300	0.07450	7.914		0.650	9.9 Comprs.
0.10840	0.01520	0.09290	3.963		0.606	12.3 Comprs.
0.10500	0.01230	0.09240			0.607	12.3 Comprs.
0.09840	0.01010	0.08800	13.293		0.617	11.7 Comprs.
0.09060	0.00880	0.08150	2.636		0.633	10.8 Comprs.
0.08180	0.00590	0.07560	1.859		0.648	10.0 Comprs.
	Dial (in.) 0.00030 0.01330 0.02100 0.03030 0.04150 0.05400 0.06960 0.08780 0.10840 0.10500 0.09840 0.09060	Dial (in.)         Defl. (in.)           0.00030         0.00090           0.02100         0.00330           0.03030         0.00570           0.04150         0.00760           0.05400         0.00940           0.06960         0.01110           0.08780         0.01300           0.10840         0.01520           0.10500         0.01230           0.09840         0.01010           0.09060         0.00880	Final Dial (in.)         Machine Defl. (in.)         Deformation (in.)           0.00030         0.000000         0.01210           0.02100         0.00330         0.01740           0.03030         0.00570         0.02430           0.04150         0.00760         0.03360           0.05400         0.00940         0.04430           0.06960         0.01110         0.05820           0.10840         0.01520         0.09290           0.10500         0.01230         0.09240           0.09840         0.01010         0.08800           0.09060         0.00880         0.08150	Final Dial (in.)         Machine Defl. (in.)         Deformation (in.)         C <sub>V</sub> (ft.2/day)           0.00030         0.00000         0.00000           0.01330         0.00090         0.01210         13.150           0.02100         0.00330         0.01740         3.407           0.03030         0.00570         0.02430         4.600           0.04150         0.00760         0.03360         4.959           0.05400         0.00940         0.04430         5.370           0.06960         0.01110         0.05820         10.464           0.08780         0.01300         0.07450         7.914           0.10840         0.01520         0.09290         3.963           0.10500         0.01230         0.09240           0.09840         0.01010         0.08800         13.293           0.09060         0.00880         0.08150         2.636	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	Final Dial (in.)Machine Defl. (in.)Deformation (in.) $C_V$ (ft.2/day) $C_α$ Void Ratio0.000300.0000000.8310.013300.000900.0121013.1500.8020.021000.003300.017403.4070.7890.030300.005700.024304.6000.7720.041500.007600.033604.9590.7500.054000.009400.044305.3700.7240.069600.011100.0582010.4640.6900.087800.013000.074507.9140.6500.108400.015200.092903.9630.6060.105000.012300.092400.6070.098400.010100.0880013.2930.6170.090600.008800.081502.6360.633

Compression index (C<sub>C</sub>), tsf = 0.13 Preconsolidation pressure (P<sub>p</sub>), tsf = 1.0 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 0.749



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-03

**Depth:** 31 Sample Number: 8D

Material Description: SO GR CH2 W/ ARS SP, SIF

Liquid Limit: 50 Plasticity Index: 32

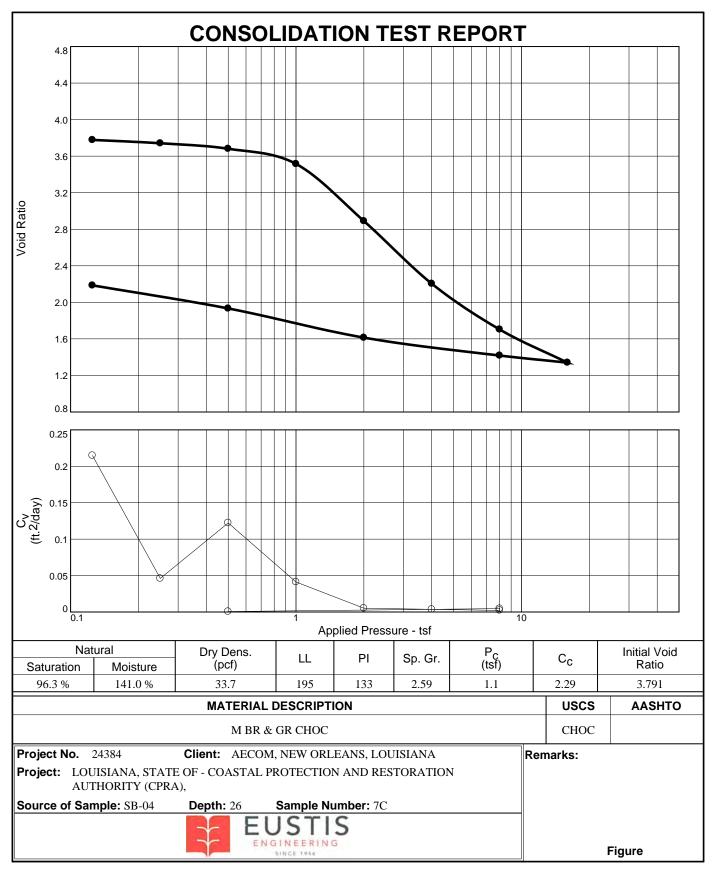
USCS: CH2

Tested by: BH Checked by: RR

rested by. Dir	Official by.	KK			
	Test Specimen Data				
NATURAL MOISTURE	VOID RATIO	AFTER TEST			
Wet w+t = $332.06$ g.	<b>Spec. Gr.</b> = $2.67$	Wet w+t = $603.99$ g.			
<b>Dry w+t</b> = $205.67$ g.	<b>Est. Ht. Solids =</b> $0.373 \text{ in.}$	<b>Dry w+t</b> = $519.15$ g.			
Tare Wt. = $0.00  \text{g}$ .	Init. V.R. = $1.676$	Tare Wt. = $313.48 \text{ g}$ .			
<b>Moisture =</b> 61.5 %	Init. Sat. = 97.9 %	<b>Moisture =</b> 41.3 %			
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $205.67^*$ g.			
<b>Height</b> = 0.997 in.	Height = $0.997$ in.				
Diameter = $4.008$ in.	Diameter = $4.008$ in.				
<b>Weight</b> = 332.06 g.					
<b>Dry Dens. =</b> 62.3 pcf	* Final dry weight used as mineral solids weight				

			EIIU	I-OI-LOau	Sullillar	y	
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	$c_{lpha}$	Void Ratio	% Strain	
start	1.99763	0.00000			1.676		
0.06	1.99749	-0.00014			1.676	0.0 Swell	
0.13	2.00151	0.00388		0.001	1.666	0.4 Comprs.	
0.25	2.00939	0.01176	0.062	0.002	1.644	1.2 Comprs.	
0.50	2.02599	0.02836	0.068	0.008	1.600	2.8 Comprs.	
1.00	2.06745	0.06982	0.017	0.026	1.489	7.0 Comprs.	
2.00	2.15768	0.16005		0.060	1.246	16.1 Comprs.	
4.00	2.24307	0.24544	0.010	0.038	1.017	24.6 Comprs.	
8.00	2.31041	0.31278	0.008	0.033	0.836	31.4 Comprs.	
16.00	2.36991	0.37228	0.006	0.020	0.677	37.3 Comprs.	
4.00	2.34127	0.34364	0.015		0.754	34.5 Comprs.	
1.00	2.30582	0.30819		0.003	0.849	30.9 Comprs.	
0.25	2.26652	0.26889			0.954	27.0 Comprs.	
0.06	2.23683	0.23920	0.001		1.034	24.0 Comprs.	

Compression index (C<sub>c</sub>), tsf = 0.79 Preconsolidation pressure (P<sub>p</sub>), tsf = 0.9 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 1.524



**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-04

Depth: 26 Sample Number: 7C

Material Description: M BR & GR CHOC

Liquid Limit: 195 Plasticity Index: 133

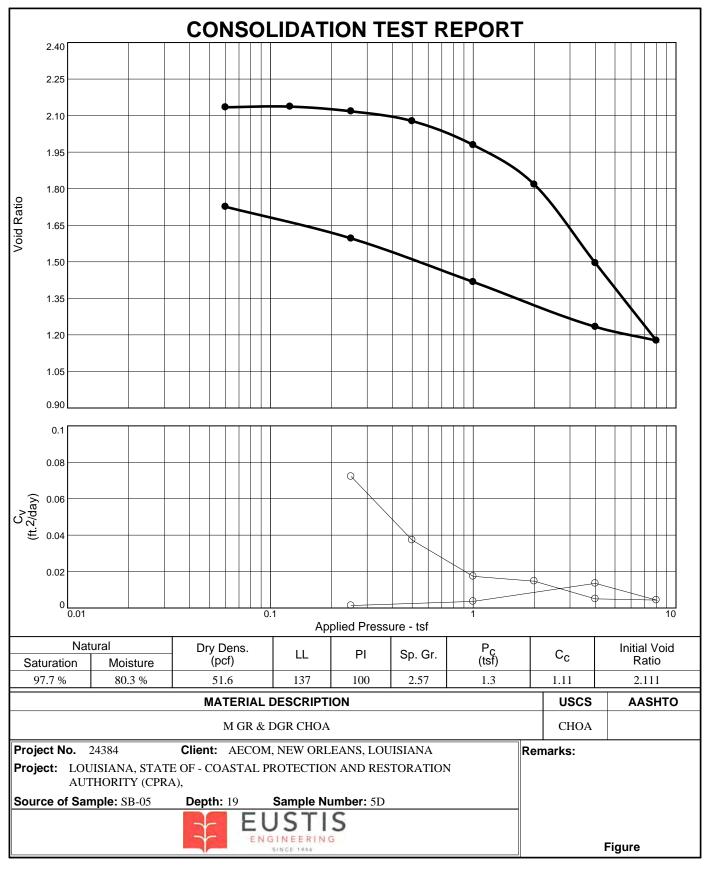
USCS: CHOC Tested by: BH

Checked by: RR

100100.09.211					
	Test Specimen Data				
NATURAL MOISTURE	VOID RATIO	AFTER TEST			
Wet w+t = $267.71$ g.	<b>Spec. Gr.</b> = $2.59$	Wet w+t = $396.91$ g.			
<b>Dry w+t</b> = $111.10$ g.	Est. Ht. Solids = $0.208$ in.	<b>Dry w+t</b> = $297.64$ g.			
<b>Tare Wt.</b> = $0.00  \text{g}$ .	Init. V.R. $= 3.791$	<b>Tare Wt.</b> = $186.54 \text{ g}$ .			
Moisture = 141.0 %	Init. Sat. = 96.3 %	Moisture = 89.4 %			
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $111.10^*$ g.			
<b>Height</b> = 0.998 in.	<b>Height</b> = 0.998 in.				
Diameter = $4.000$ in.	<b>Diameter</b> = $4.000$ in.				
<b>Weight</b> = 267.71 g.					
<b>Dry Dens. =</b> 33.7 pcf	* Final dry weight used as mineral solids weight				

End-Of-Load Summary						
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	${\sf c}_{lpha}$	Void Ratio	% Strain
start	1.81857	0.00000			3.791	
0.13	1.82129	0.00272	0.215	0.006	3.778	0.3 Comprs.
0.25	1.82899	0.01042	0.046	0.004	3.741	1.0 Comprs.
0.50	1.84151	0.02294	0.122	0.010	3.681	2.3 Comprs.
1.00	1.87600	0.05743	0.041	0.045	3.515	5.8 Comprs.
2.00	2.00626	0.18769	0.005	0.119	2.890	18.8 Comprs.
4.00	2.14909	0.33052	0.004	0.118	2.204	33.1 Comprs.
8.00	2.25342	0.43485	0.002	0.141	1.703	43.6 Comprs.
16.00	2.32911	0.51054		0.109	1.340	51.2 Comprs.
8.00	2.31293	0.49436	0.005		1.418	49.5 Comprs.
2.00	2.27205	0.45348			1.614	45.4 Comprs.
0.50	2.20551	0.38694	0.001		1.933	38.8 Comprs.
0.13	2.15290	0.33433			2.186	33.5 Comprs.

Compression index (C<sub>c</sub>), tsf = 2.29 Preconsolidation pressure (P<sub>p</sub>), tsf = 1.1 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 3.478



**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-05

Depth: 19 Sample Number: 5D

Material Description: M GR & DGR CHOA

Liquid Limit: 137 Plasticity Index: 100

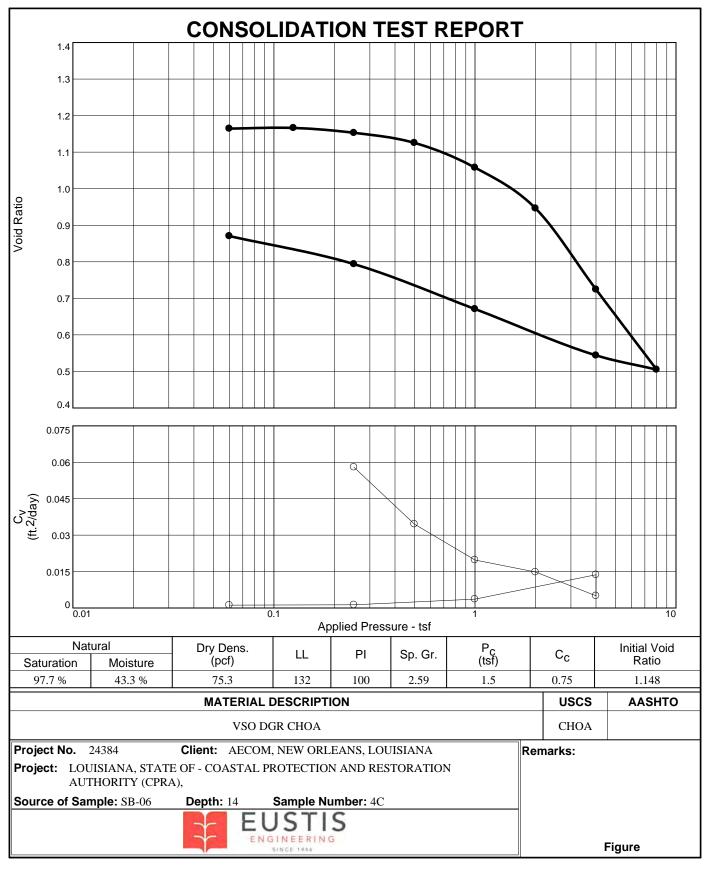
USCS: CHOA

Tested by: BH Checked by: RR

restea by: BH	Cnecked by	y: KK					
	Test Specimen Data						
NATURAL MOISTURE	VOID RATIO	AFTER TEST					
Wet w+t = $305.76$ g.	<b>Spec. Gr.</b> = $2.57$	Wet w+t = $540.71$ g.					
<b>Dry w+t</b> = 169.60 g.	<b>Est. Ht. Solids =</b> $0.321$ in.	<b>Dry w+t</b> = $429.83 \text{ g}$ .					
<b>Tare Wt.</b> = $0.00  \text{g}$ .	Init. V.R. = $2.111$	<b>Tare Wt.</b> = $260.23$ g.					
<b>Moisture =</b> 80.3 %	Init. Sat. = 97.7 %	<b>Moisture =</b> 65.4 %					
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $169.60^*$ g.					
<b>Height</b> = 0.998 in.	Height = $0.998$ in.						
<b>Diameter =</b> 3.998 in.	<b>Diameter</b> = 3.998 in.						
<b>Weight</b> = 305.76 g.							
<b>Dry Dens. =</b> 51.6 pcf	* Final dry weight used as min	* Final dry weight used as mineral solids weight					

End-Of-Load Summary							
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	$\mathbf{c}_{lpha}$	Void Ratio	% Strain	
start	1.70778	0.00000			2.111		
0.06	1.70033	-0.00745			2.134	0.7 Swell	
0.13	1.69950	-0.00828			2.137	0.8 Swell	
0.25	1.70571	-0.00207	0.072	0.004	2.118	0.2 Swell	
0.50	1.71855	0.01077	0.037	0.007	2.078	1.1 Comprs.	
1.00	1.75004	0.04226	0.017	0.016	1.979	4.2 Comprs.	
2.00	1.80197	0.09419	0.015	0.050	1.817	9.4 Comprs.	
4.00	1.90536	0.19758	0.005	0.060	1.495	19.8 Comprs.	
8.00	2.00748	0.29970	0.004	0.107	1.177	30.0 Comprs.	
4.00	1.98939	0.28161	0.014		1.233	28.2 Comprs.	
1.00	1.93032	0.22254	0.004		1.417	22.3 Comprs.	
0.25	1.87303	0.16525	0.001		1.596	16.6 Comprs.	
0.06	1.83135	0.12357			1.726	12.4 Comprs.	

Compression index (C<sub>c</sub>), tsf = 1.11 Preconsolidation pressure (P<sub>p</sub>), tsf = 1.3 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 1.927



**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-06

Depth: 14 Sample Number: 4C

Material Description: VSO DGR CHOA

Liquid Limit: 132 Plasticity Index: 100

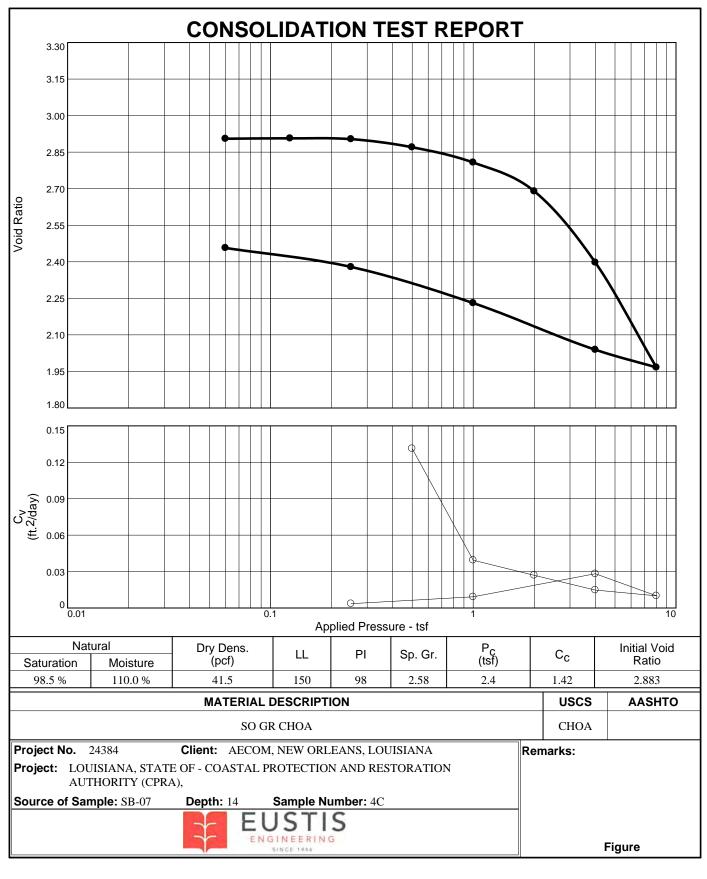
USCS: CHOA
Tested by: BH

ested by: BH Checked by: RR

resteu by. bn	Checked by. KK					
	Test Specimen Data					
NATURAL MOISTURE	VOID RATIO	AFTER TEST				
Wet w+t = $355.41$ g.	<b>Spec. Gr.</b> = 2.59	Wet w+t = $475.02$ g.				
<b>Dry w+t</b> = $248.00$ g.	<b>Est. Ht. Solids =</b> 0.466 in.	<b>Dry w+t</b> = $378.98 \text{ g}$ .				
<b>Tare Wt.</b> = $0.00  \text{g}$ .	<b>Init. V.R.</b> = 1.148	Tare Wt. = $130.98 \text{ g}$ .				
<b>Moisture =</b> 43.3 %	Init. Sat. = 97.7 %	<b>Moisture =</b> 38.7 %				
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $248.00^*$ g.				
<b>Height</b> = 1.001 in.	<b>Height</b> = 1.001 in.					
<b>Diameter =</b> 3.996 in.	<b>Diameter</b> = 3.996 in.					
<b>Weight</b> = 355.41 g.						
<b>Dry Dens. =</b> 75.3 pcf	* Final dry weight used as mineral solids	weight				

End-Of-Load Summary						
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	${\sf c}_{lpha}$	Void Ratio	% Strain
start	1.70778	0.00000			1.148	
0.06	1.70033	-0.00745			1.164	0.7 Swell
0.13	1.69950	-0.00828			1.166	0.8 Swell
0.25	1.70571	-0.00207	0.058	0.002	1.153	0.2 Swell
0.50	1.71855	0.01077	0.035	0.005	1.125	1.1 Comprs.
1.00	1.75004	0.04226	0.020	0.012	1.058	4.2 Comprs.
2.00	1.80197	0.09419	0.015	0.034	0.946	9.4 Comprs.
4.00	1.90536	0.19758	0.005	0.041	0.724	19.7 Comprs.
8.00	2.00748	0.29970		0.061	0.505	29.9 Comprs.
4.00	1.98939	0.28161	0.014		0.544	28.1 Comprs.
1.00	1.93032	0.22254	0.004		0.671	22.2 Comprs.
0.25	1.87303	0.16525	0.001		0.794	16.5 Comprs.
0.06	1.83729	0.12951	0.001		0.870	12.9 Comprs.

Compression index (C<sub>C</sub>), tsf = 0.75 Preconsolidation pressure (P<sub>p</sub>), tsf = 1.5 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 1.006



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-07

**Depth:** 14 Sample Number: 4C

**Material Description:** SO GR CHOA

Liquid Limit: 150 Plasticity Index: 98

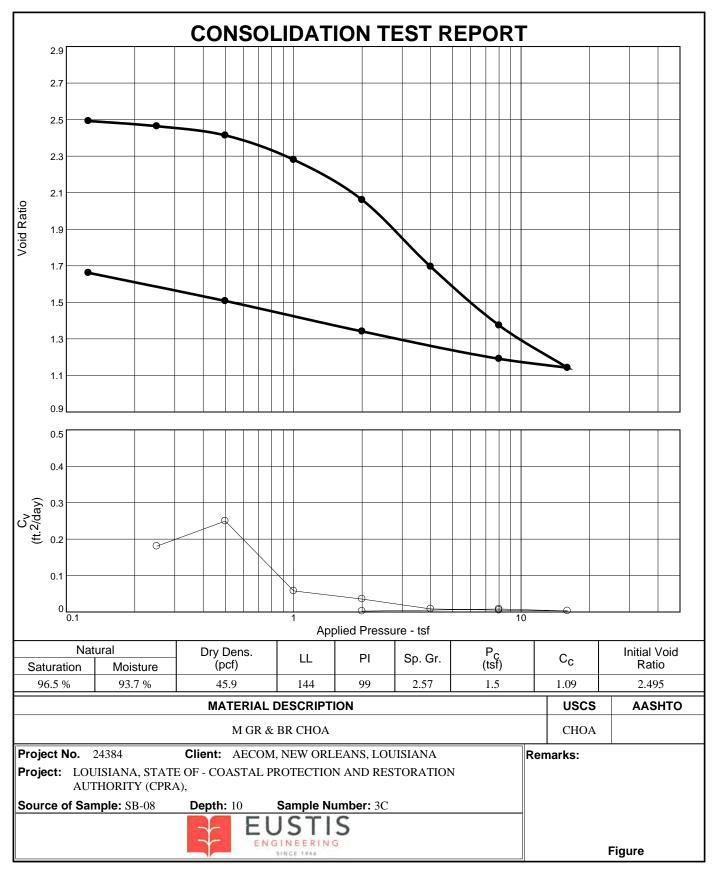
**USCS:** CHOA Tested by: BH

Checked by: RR

100tou by 1 Dil	onothou by the				
	Test Specimen Data				
NATURAL MOISTURE	VOID RATIO	AFTER TEST			
Wet w+t = $287.23$ g.	<b>Spec. Gr.</b> = $2.58$	Wet w+t = $402.81$ g.			
Dry w+t = $136.75$ g.	<b>Est. Ht. Solids =</b> $0.257 \text{ in.}$	<b>Dry w+t</b> = $265.07$ g.			
Tare Wt. = $0.00  \text{g}$ .	Init. V.R. = $2.883$	Tare Wt. = $128.32$ g.			
Moisture = 110.0 %	Init. Sat. = $98.5 \%$	<b>Moisture =</b> 100.7 %			
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $136.75^*$ g.			
<b>Height</b> = 0.999 in.	<b>Height</b> = 0.999 in.	-			
Diameter = $4.001$ in.	Diameter = $4.001$ in.				
Weight = 287.23 g.					
<b>Dry Dens. =</b> 41.5 pcf	* Final dry weight used as mi	ineral solids weight			

			End	I-OI-Load	Summar	у
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	${\sf c}_{lpha}$	Void Ratio	% Strain
start	1.12972	0.00000			2.883	
0.06	1.12405	-0.00567			2.905	0.6 Swell
0.13	1.12368	-0.00604			2.907	0.6 Swell
0.25	1.12450	-0.00522			2.903	0.5 Swell
0.50	1.13307	0.00335	0.131	0.004	2.870	0.3 Comprs.
1.00	1.14920	0.01948	0.039	0.008	2.807	1.9 Comprs.
2.00	1.17949	0.04977	0.027	0.028	2.690	5.0 Comprs.
4.00	1.25473	0.12501	0.015	0.052	2.397	12.5 Comprs.
8.00	1.36551	0.23579	0.010	0.082	1.967	23.6 Comprs.
4.00	1.34687	0.21715	0.028		2.039	21.7 Comprs.
1.00	1.29760	0.16788	0.009		2.231	16.8 Comprs.
0.25	1.25948	0.12976	0.004		2.379	13.0 Comprs.
0.06	1.23937	0.10965			2.457	11.0 Comprs.

Compression index (C<sub>C</sub>), tsf = 1.42 Preconsolidation pressure (P<sub>p</sub>), tsf = 2.4 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 2.638



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-08

**Depth:** 10 Sample Number: 3C

Material Description: M GR & BR CHOA

Liquid Limit: 144 Plasticity Index: 99

**USCS:** CHOA Tested by: BH

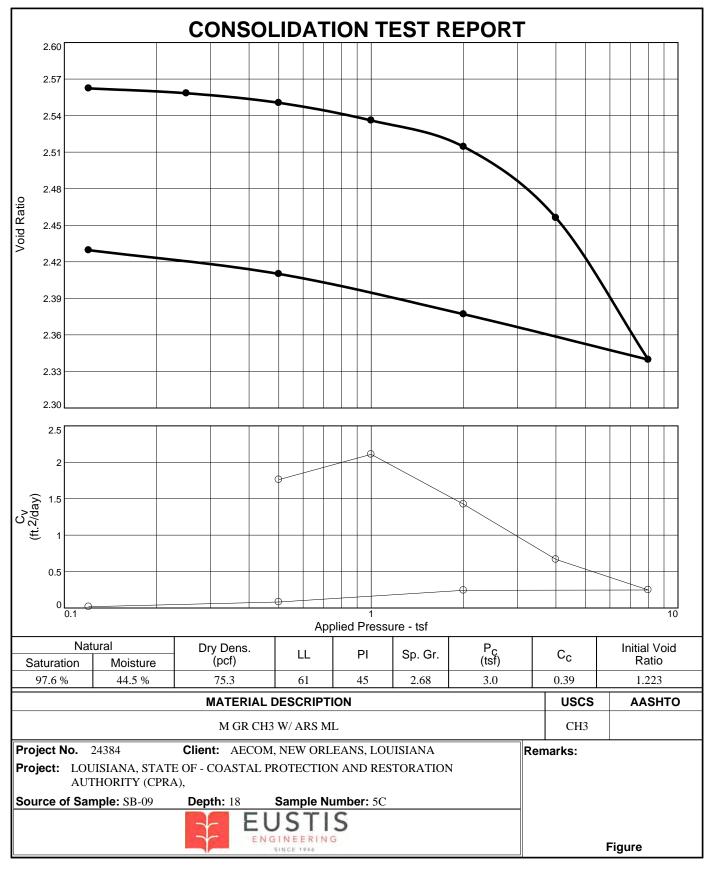
Checked by: RR

	Test Specimen D	<b>Data</b>			
NATURAL MOISTURE	VOID RATIO	AFTER TEST			
Wet w+t = $293.64 \text{ g}$ .	<b>Spec. Gr.</b> = $2.57$	Wet w+t = $379.74$ g.			
<b>Dry w+t</b> = $151.59$ g.	Est. Ht. Solids = $0.285$ in	Dry w+t = $281.23$ g.			
Tare Wt. = $0.00  \text{g}$ .	Init. V.R. = $2.495$	<b>Tare Wt.</b> = $129.64$ g.			
<b>Moisture =</b> 93.7 %	Init. Sat. = 96.5 %	<b>Moisture =</b> 65.0 %			
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $151.59^*$ g.			
<b>Height</b> = 0.997 in.	<b>Height</b> = 0.997 in	i.			
Diameter = $4.008$ in.	Diameter = $4.008$ in	1.			
Weight = 293.64 g.					
Dry Dens. = 45.9 pcf * Final dry weight used as mineral solids weight					

* Final dry weight used as mineral solids weight
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End-Of-Load Summary						
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	${\sf c}_{lpha}$	Void Ratio	% Strain
start	1.99354	0.00000			2.495	
0.13	1.99414	0.00060		0.001	2.493	0.1 Comprs.
0.25	2.00232	0.00878	0.181	0.003	2.464	0.9 Comprs.
0.50	2.01666	0.02312	0.250	0.007	2.414	2.3 Comprs.
1.00	2.05469	0.06115	0.058	0.017	2.280	6.1 Comprs.
2.00	2.11737	0.12383	0.036	0.060	2.061	12.4 Comprs.
4.00	2.22155	0.22801	0.008	0.076	1.695	22.9 Comprs.
8.00	2.31319	0.31965	0.005	0.043	1.374	32.1 Comprs.
16.00	2.37944	0.38590	0.003	0.063	1.142	38.7 Comprs.
8.00	2.36542	0.37188	0.008		1.191	37.3 Comprs.
2.00	2.32271	0.32917	0.002		1.341	33.0 Comprs.
0.50	2.27517	0.28163			1.507	28.2 Comprs.
0.13	2.23119	0.23765			1.662	23.8 Comprs.

Compression index (C<sub>c</sub>), tsf = 1.09 Preconsolidation pressure (P<sub>p</sub>), tsf = 1.5 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 2.178



**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-09

Depth: 18 Sample Number: 5C

Material Description: M GR CH3 W/ ARS ML

Liquid Limit: 61 Plasticity Index: 45

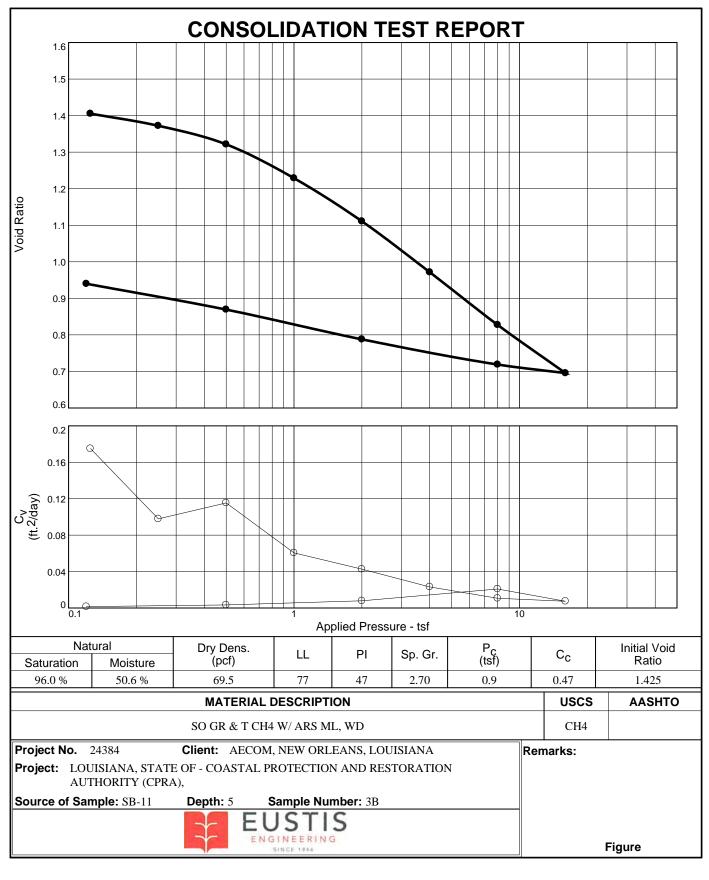
USCS: CH3

Tested by: BH Checked by: RR

rested by: BH	Cnecked by: KK					
Test Specimen Data						
NATURAL MOISTURE	VOID RATIO	AFTER TEST				
Wet w+t = 358.41 g.	<b>Spec. Gr.</b> = $2.68$	Wet w+t = $475.02$ g.				
<b>Dry w+t</b> = $248.00$ g.	<b>Est. Ht. Solids =</b> $0.450 \text{ in.}$	Dry w+t = $378.98 \text{ g}.$				
<b>Tare Wt.</b> = $0.00  \text{g}$ .	Init. V.R. = $1.223$	<b>Tare Wt.</b> = $130.98$ g.				
<b>Moisture =</b> 44.5 %	Init. Sat. = 97.6 %	<b>Moisture =</b> 38.7 %				
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $248.00^*$ g.				
Height = $1.001$ in.	Height = $1.001$ in.					
<b>Diameter =</b> 3.996 in.	<b>Diameter</b> = 3.996 in.					
<b>Weight</b> = 358.41 g.						
<b>Dry Dens. =</b> 75.3 pcf	* Final dry weight used as mineral solids	s weight				

			Ena	-Ot-Load	Summar	y	
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	$\mathbf{c}_{lpha}$	Void Ratio	% Strain	
start	1.74834	0.00000			1.223		
0.12	1.74828	-0.00006			2.562	0.0 Swell	
0.25	1.75053	0.00219		0.004	2.558	0.1 Comprs.	
0.50	1.75493	0.00659	1.762	0.002	2.550	0.3 Comprs.	
1.00	1.76305	0.01471	2.110	0.002	2.536	0.7 Comprs.	
2.00	1.77515	0.02681	1.425	0.003	2.514	1.3 Comprs.	
4.00	1.80785	0.05951	0.667	0.008	2.456	3.0 Comprs.	
8.00	1.87316	0.12482	0.247	0.018	2.340	6.2 Comprs.	
2.00	1.85226	0.10392	0.243		2.377	5.2 Comprs.	
0.50	1.83372	0.08538	0.082		2.410	4.3 Comprs.	
0.12	1.82276	0.07442	0.017		2.430	3.7 Comprs.	

Compression index (C<sub>c</sub>), tsf = 0.39 Preconsolidation pressure (P<sub>p</sub>), tsf = 3.0 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 2.488



**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-11

Depth: 5 Sample Number: 3B

Material Description: SO GR & T CH4 W/ ARS ML, WD

Liquid Limit: 77 Plasticity Index: 47

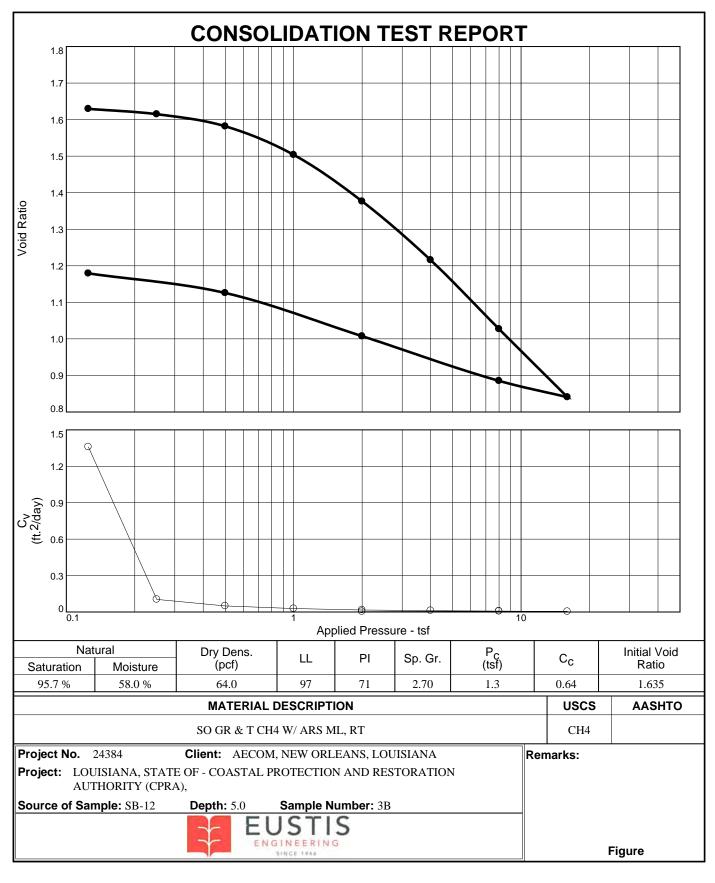
USCS: CH4

Tested by: BH Checked by: RR

rested by: BH	Cnecked by: RR				
	Test Specimen Data				
NATURAL MOISTURE	VOID RATIO	AFTER TEST			
Wet w+t = $344.23$ g.	<b>Spec. Gr.</b> = $2.70$	Wet w+t = $444.10$ g.			
<b>Dry w+t</b> = $228.51$ g.	<b>Est. Ht. Solids =</b> $0.411$ in.	<b>Dry w+t</b> = $358.41$ g.			
<b>Tare Wt.</b> = $0.00  \text{g}$ .	Init. V.R. = $1.425$	<b>Tare Wt.</b> = $129.90 \text{ g}$ .			
<b>Moisture =</b> 50.6 %	Init. Sat. = 96.0 %	<b>Moisture =</b> 37.5 %			
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $228.51^*$ g.			
Height = $0.997$ in.	Height = $0.997$ in.				
<b>Diameter =</b> 3.999 in.	<b>Diameter</b> = $3.999$ in.				
Weight = 344.23 g.					
Dry Dens. = 69.5 pcf * Final dry weight used as mineral solids weight					

End-Of-Load Summary							
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	${\sf c}_{lpha}$	Void Ratio	% Strain	
start	2.40405	0.00000			1.425		
0.13	2.41197	0.00792	0.175	0.005	1.405	0.8 Comprs.	
0.25	2.42561	0.02156	0.098	0.005	1.372	2.2 Comprs.	
0.50	2.44650	0.04245	0.115	0.009	1.321	4.3 Comprs.	
1.00	2.48460	0.08055	0.061	0.014	1.229	8.1 Comprs.	
2.00	2.53308	0.12903	0.043	0.021	1.111	12.9 Comprs.	
4.00	2.59038	0.18633	0.023	0.022	0.971	18.7 Comprs.	
8.00	2.64962	0.24557	0.011	0.020	0.827	24.6 Comprs.	
16.00	2.70387	0.29982	0.007	0.014	0.695	30.1 Comprs.	
8.00	2.69424	0.29019	0.021		0.719	29.1 Comprs.	
2.00	2.66589	0.26184	0.008		0.788	26.3 Comprs.	
0.50	2.63243	0.22838	0.003		0.869	22.9 Comprs.	
0.12	2.60337	0.19932	0.002		0.940	20.0 Comprs.	

Compression index (C<sub>C</sub>), tsf = 0.47 Preconsolidation pressure (P<sub>p</sub>), tsf = 0.9 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 1.242



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-12

**Depth:** 5.0 **Sample Number:** 3B

Material Description: SO GR & T CH4 W/ ARS ML, RT

**Liquid Limit:** 97 Plasticity Index: 71

USCS: CH4

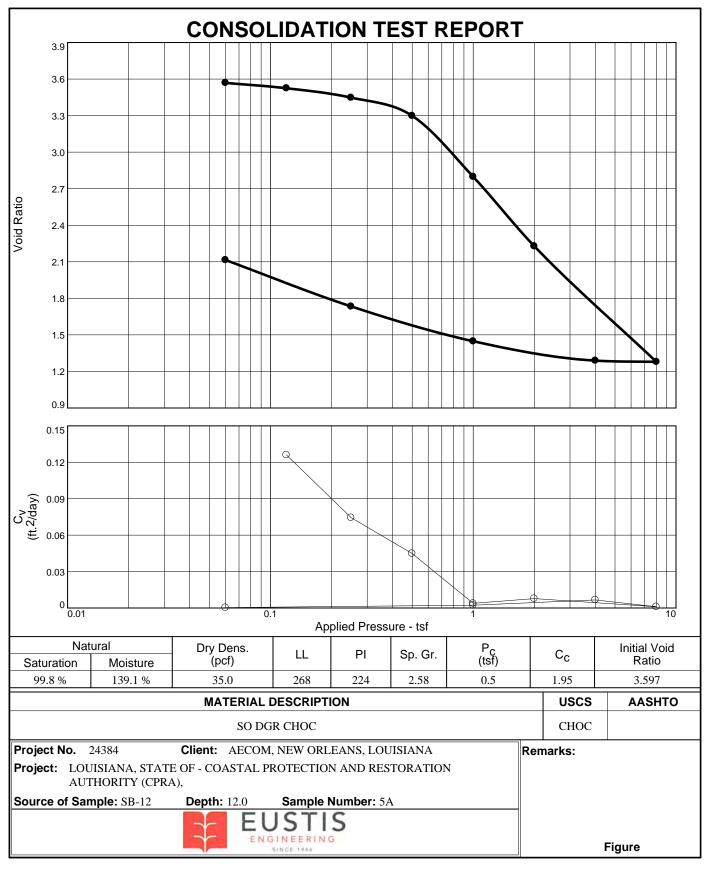
Tested by: RH Checked by: RR

Tested by: BH Checked by: RK				
	Test Specimen Data			
NATURAL MOISTURE	VOID RATIO	AFTER TEST		
Wet w+t = 333.67 g.	<b>Spec. Gr.</b> = $2.70$	Wet w+t = $602.56$ g.		
<b>Dry w+t</b> = $211.21$ g.	<b>Est. Ht. Solids =</b> $0.378 \text{ in.}$	<b>Dry w+t</b> = $505.40$ g.		
Tare Wt. = 0.00 g.	Init. V.R. = $1.635$	<b>Tare Wt.</b> = $294.19$ g.		
Moisture = 58.0 %	Init. Sat. = 95.7 %	<b>Moisture =</b> 46.0 %		
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $211.21^*$ g.		
<b>Height</b> = 0.997 in.	<b>Height</b> = 0.997 in.			
<b>Diameter =</b> 4.008 in.	<b>Diameter</b> = $4.008$ in.			
<b>Weight</b> = 333.67 g.				
Dry Dens. = 64.0 pcf * Final dry weight used as mineral solids weight				

*					
Final	dry weight	used as	mineral	solids	weight
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End-Of-Load Summary						
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	${f c}_{lpha}$	Void Ratio	% Strain
start	1.99817	0.00000			1.635	
0.13	2.00033	0.00216	1.360	0.001	1.629	0.2 Comprs.
0.25	2.00585	0.00768	0.103	0.002	1.615	0.8 Comprs.
0.50	2.01837	0.02020	0.050	0.003	1.582	2.0 Comprs.
1.00	2.04789	0.04972	0.028	0.010	1.504	5.0 Comprs.
2.00	2.09606	0.09789	0.016	0.020	1.376	9.8 Comprs.
4.00	2.15685	0.15868	0.011	0.043	1.216	15.9 Comprs.
8.00	2.22822	0.23005	0.006	0.029	1.027	23.1 Comprs.
16.00	2.29879	0.30062	0.004	0.032	0.841	30.2 Comprs.
8.00	2.28192	0.28375	0.007		0.885	28.5 Comprs.
2.00	2.23564	0.23747	0.003		1.007	23.8 Comprs.
0.50	2.19090	0.19273			1.126	19.3 Comprs.
0.13	2.17064	0.17247			1.179	17.3 Comprs.
					<b></b>	10 11 11 15 1 1 151

Compression index (C<sub>c</sub>), tsf = 0.64 Preconsolidation pressure (P<sub>p</sub>), tsf = 1.3 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 1.454



**Project:** LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-12

Depth: 12.0 Sample Number: 5A

Material Description: SO DGR CHOC

Liquid Limit: 268 Plasticity Index: 224

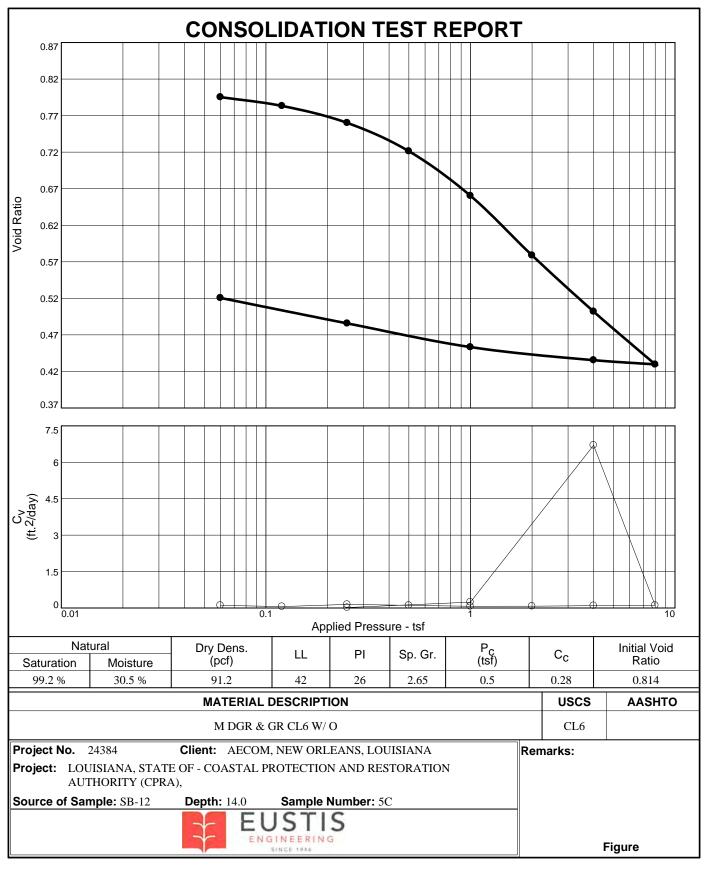
USCS: CHOC

ested by: BH Checked by: RR

rested by: BH	Checked by: KR				
	Test Specimen Data				
NATURAL MOISTURE	VOID RATIO	AFTER TEST			
Wet w+t = $203.83$ g.	<b>Spec. Gr.</b> = $2.58$	Wet w+t = $545.46$ g.			
Dry w+t = $85.25 \text{ g}$ .	<b>Est. Ht. Solids =</b> $0.164 \text{ in.}$	<b>Dry w+t</b> = $448.45$ g.			
<b>Tare Wt.</b> = $0.00  \text{g}$ .	Init. V.R. = 3.597	<b>Tare Wt.</b> = $363.20 \text{ g}$ .			
Moisture = 139.1 %	Init. Sat. = 99.8 %	<b>Moisture =</b> 113.8 %			
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $85.25^*$ g.			
Height = $0.753$ in.	Height = $0.753$ in.				
<b>Diameter =</b> 3.959 in.	<b>Diameter</b> = $3.959$ in.				
Weight = 203.83 g.					
Dry Dens. = 35.0 pcf * Final dry weight used as mineral solids weight					

		End-Of-Load Summary					
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	${f c}_{lpha}$	Void Ratio	% Strain	
start	0.00010	0.00000			3.597		
0.06	0.00480	0.00470		0.001	3.568	0.6 Comprs.	
0.12	0.01200	0.01190	0.126	0.007	3.524	1.6 Comprs.	
0.25	0.02460	0.02450	0.074	0.007	3.448	3.3 Comprs.	
0.50	0.04900	0.04890	0.045	0.022	3.299	6.5 Comprs.	
1.00	0.13100	0.13090	0.004	0.024	2.798	17.4 Comprs.	
2.00	0.22430	0.22420	0.008	0.069	2.228	29.8 Comprs.	
8.00	0.37980	0.37970	0.001	0.000	1.279	50.4 Comprs.	
4.00	0.37820	0.37810	0.007	0.000	1.289	50.2 Comprs.	
1.00	0.35210	0.35200	0.002		1.448	46.7 Comprs.	
0.25	0.30530	0.30520			1.734	40.5 Comprs.	
0.06	0.24290	0.24280	0.000		2.115	32.2 Comprs.	

Compression index (C<sub>c</sub>), tsf = 1.95 Preconsolidation pressure (P<sub>p</sub>), tsf = 0.5 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 3.270



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-12

**Depth:** 14.0 Sample Number: 5C

Material Description: M DGR & GR CL6 W/ O

Liquid Limit: 42 Plasticity Index: 26

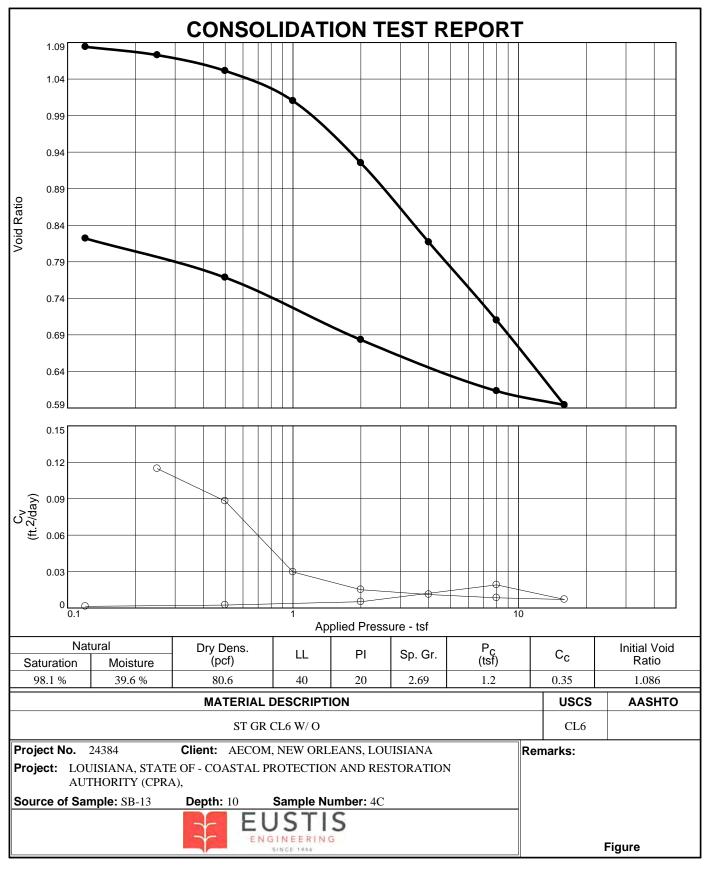
**USCS:** CL6 Tested by: RH

Checked by: RR

rested by: BH	Cnecked by: KK	
	Test Specimen Data	
NATURAL MOISTURE	VOID RATIO	AFTER TEST
Wet w+t = 293.76 g.	<b>Spec. Gr.</b> = $2.65$	Wet w+t = $597.94$ g.
<b>Dry w+t</b> = $225.18 \text{ g}$ .	<b>Est. Ht. Solids =</b> $0.413 \text{ in.}$	<b>Dry w+t</b> = $538.66$ g.
<b>Tare Wt.</b> = $0.00  \text{g}$ .	Init. V.R. = $0.814$	<b>Tare Wt.</b> = $313.48 \text{ g}$ .
<b>Moisture =</b> 30.5 %	Init. Sat. = 99.2 %	<b>Moisture =</b> 26.3 %
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $225.18^*$ g.
Height = $0.750$ in.	Height = $0.750$ in.	
<b>Diameter =</b> 3.996 in.	<b>Diameter</b> = 3.996 in.	
<b>Weight</b> = 293.76 g.		
<b>Dry Dens. =</b> 91.2 pcf	* Final dry weight used as mineral solid	ds weight

			End	-Of-Load	Summar	У	
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	${f c}_{lpha}$	Void Ratio	% Strain	
start	0.00030	0.00000			0.814		
0.06	0.00800	0.00770	0.107	0.001	0.795	1.0 Comprs.	
0.12	0.01300	0.01270	0.058	0.002	0.783	1.7 Comprs.	
0.25	0.02260	0.02230	0.147	0.003	0.760	3.0 Comprs.	
0.50	0.03860	0.03830	0.108	0.004	0.721	5.1 Comprs.	
1.00	0.06380	0.06350	0.070	0.008	0.660	8.5 Comprs.	
2.00	0.09750	0.09720	0.073	0.008	0.579	13.0 Comprs.	
4.00	0.12930	0.12900	0.095	0.008	0.502	17.2 Comprs.	
8.00	0.15920	0.15890	0.104	0.006	0.430	21.2 Comprs.	
4.00	0.15680	0.15650	6.702		0.435	20.9 Comprs.	
1.00	0.14940	0.14910	0.240		0.453	19.9 Comprs.	
0.25	0.13600	0.13570	0.018		0.486	18.1 Comprs.	
0.06	0.12160	0.12130			0.521	16.2 Comprs.	

Compression index (C<sub>C</sub>), tsf = 0.28 Preconsolidation pressure (P<sub>p</sub>), tsf = 0.5 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 0.722



Project: LOUISIANA, STATE OF - COASTAL PROTECTION AND RESTORATION AUTHORITY (CPRA), 15% DESIGN PHASE, MAUREPAS DIVERSION AND WEST SHORE OF LAKE PONTCHARTRAIN,

ST. JOHN THE BAPTIST PARISH, LOUISIANA. S.P. PO-0029

**Project Number: 24384** 

Location: SB-13

**Depth:** 10 Sample Number: 4C

Material Description: ST GR CL6 W/O

Liquid Limit: 40 Plasticity Index: 20

USCS: CL6

Tested by: RH Chacked by: RR

Tested by: BH	Checked by	y: RR			
	Test Specimen Data				
NATURAL MOISTURE	VOID RATIO	AFTER TEST			
Wet w+t = $370.04$ g.	<b>Spec. Gr.</b> = $2.69$	Wet w+t = $830.44$ g.			
<b>Dry w+t</b> = $265.08$ g.	<b>Est. Ht. Solids =</b> $0.479 \text{ in.}$	<b>Dry w+t</b> = $742.88 \text{ g}$ .			
<b>Tare Wt.</b> = $0.00  \text{g}$ .	Init. V.R. = $1.086$	<b>Tare Wt.</b> = $477.80 \text{ g}$ .			
<b>Moisture =</b> 39.6 %	Init. Sat. = 98.1 %	<b>Moisture =</b> 33.0 %			
UNIT WEIGHT	TEST START	<b>Dry Wt.</b> = $265.08^*$ g.			
<b>Height</b> = 0.997 in.	Height $= 0.998$ in.				
Diameter = $4.000$ in.	Diameter = $4.000$ in.				
Weight = 370.04 g.					
<b>Dry Dens. =</b> 80.6 pcf	* Final dry weight used as mineral solids weight				

			End-Of-Load Summary				
Pressure (tsf)	Final Dial (in.)	Deformation (in.)	C <sub>V</sub> (ft. <sup>2</sup> /day)	${f c}_{lpha}$	Void Ratio	% Strain	
start	1.82482	0.00000			1.086		
0.12	1.82552	0.00070		0.004	1.084	0.1 Comprs.	
0.25	1.83096	0.00614	0.115	0.001	1.073	0.6 Comprs.	
0.50	1.84126	0.01644	0.088	0.005	1.051	1.6 Comprs.	
1.00	1.86093	0.03611	0.030	0.004	1.010	3.6 Comprs.	
2.00	1.90159	0.07677	0.015	0.017	0.925	7.7 Comprs.	
4.00	1.95338	0.12856	0.011	0.020	0.817	12.9 Comprs.	
8.00	2.00453	0.17971	0.008	0.022	0.710	18.0 Comprs.	
16.00	2.06002	0.23520	0.007	0.009	0.594	23.6 Comprs.	
8.00	2.05083	0.22601	0.019		0.613	22.6 Comprs.	
2.00	2.01734	0.19252	0.005		0.683	19.3 Comprs.	
0.50	1.97657	0.15175	0.002		0.768	15.2 Comprs.	
0.12	1.95098	0.12616	0.001		0.822	12.6 Comprs.	

Compression index (C<sub>C</sub>), tsf = 0.35 Preconsolidation pressure (P<sub>p</sub>), tsf = 1.2 Void ratio at P<sub>p</sub> (e<sub>m</sub>) = 0.990